

Site Planning Civil Engineering Landscape Architecture Land Surveying Transportation Engineering Environmental Studies Entitlements Construction Services 3D Visualization Laser Scanning

January 25, 2021

Mr. Christopher Carthy, Chairman and Members of the Planning Board Town of North Castle 17 Bedford Road Armonk, NY 10504

RE: JMC Project 19124 #100-Building 2 100 Business Park Drive Town of North Castle, NY

## Response to Town Comments Submission

Dear Chairman Carthy and Members of the Planning Board:

On behalf of the owner and applicant, A & R Real Estate Holdings LLC, we are pleased to submit the following documents for your continued review of the Amended Site Plan Application of a new warehouse building at 100 Business Park Drive:

I. JMC Drawings:

<u>Title</u>	<u>Rev. #</u>	/Date
"Cover Sheet"	Ι	01/25/2021
"Overall Existing Conditions Map"	I	01/25/2021
"Existing Conditions Map"	I I	01/25/2021
"Site Demolition & Tree Removal Plan"	I I	01/25/2021
"Site Layout Plan"	I	01/25/2021
"Site Grading Plan"	I I	01/25/2021
"Site Utilities Plan"	I	01/25/2021
"Site Erosion & Sediment Control Plan"	I	01/25/2021
"Site Landscaping & Wetland Mitigation Plan"	2	01/25/2021
"Site Lighting Plan"	I I	01/25/2021
"Existing Flood Storage Volume Analysis Plan"	I I	01/25/2021
"Proposed Flood Storage Volume Analysis Plan"	I	01/25/2021
"Construction Details"	I I	01/25/2021
"Construction Details"	I	01/25/2021
"Construction Details"	I	01/25/2021
"Construction Details"	I	01/25/2021
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JMC Planning Engineering Landscape Architecture & Land Surveying, PLLC | JMC Site Development Consultants, LLC

- 2. "Stormwater Pollution Prevention Plan," prepared by JMC, PLLC, revised 01/25/2021.
- 3. "Email Correspondence from NYSDEC regarding their jurisdictional review", dated 04/28/2020.
- 4. "No Permit Required Determination Letter", prepared by the U.S. Army Corps of Engineers, dated 08/10/2020.
- 5. "Hydrologic & Hydraulic Report", prepared by Leonard Jackson Associates, dated 10/12/2020.
- 6. "Email Correspondence from Town Clerk regarding Approval of Compensatory Floodplain Storage variance by Town Board", dated 12/10/2020.
- 7. Draft Stormwater Control Facility Maintenance Agreement, prepared by A&R Real Estate Holdings, dated 01/25/2021.
- 8. Draft New York State Department of Environmental Conservation Notice of Intent, prepared by JMC, dated 01/25/2021.
- 9. Draft WCDPW Stream Control Permit, prepared by JMC, dated 01/25/2021.

Since our last appearance before the Planning Board, we have been before the Conservation Board (on 05/19/2020 and 06/16/2020) for review of the project's proposed disturbance to the Town-regulated 100-foot wetland adjacent area (buffer). After revising our site plans to provide the minimum 2:1 mitigation ratio associated with the proposed 38,840 sf of disturbance to the wetland buffer the Conservation Board was satisfied and prepared a positive recommendation back to the Planning Board.

In addition to receiving a recommendation letter from the Conservation Board, the applicant processed a separate application with the Town Board requesting a variance from the compensatory floodplain storage requirement of the Town Code. The applicant retained a Floodplain Consultant to study the need for compensatory floodplain storage and concluded that constructing compensatory floodplain storage as previously designed yields no measurable benefit to the Byram River and adjacent properties. After appearing before the Town Board (on 11/18/2020 and 12/09/2020) and a completed technical review by the Town Consulting Engineer, the applicant obtained the requested variance.

The revisions depicted on the above noted plans reflect responses to comments from the Conservation Board, Town Board (for compensatory floodplain variance) and responses to comments outlined in the Town of North Castle Planning Department memorandum, dated April 10, 2020, and the Kellard Sessions Consulting, P.C. memorandum, dated April 9, 2020. For ease of review, we have repeated and enumerated the comments in italic print, followed by our responses:

## Town of North Castle Planning Department Memorandum, dated April 10, 2020

## General Comments

## Comment No. 1

The proposed design of the warehouse building provides for loading at the front of the building. The site plan should be revised to depict how trucks would access the loading area without having to back up onto Business Park Drive. Additionally, as proposed, the loading in the front of the property is not permitted pursuant to Section 355-40.D(3) of the Town Code. It is strongly recommended that the loading area be relocated to the rear of the building in an effort to limit visual impacts of loading trucks fronting on Business Park Drive.

## Response No. I

The site plans have been revised to depict truck (WB-67) turning movements accessing the loading area without having to back up onto Business Park Drive. As discussed with the Planning Board, the proposed building has been designed to mirror the loading of the existing building to better sync daily operations of the facility. In an effort to address comments received from the Board and demonstrate compliance with Section 355-30.D(3), we have shifted the proposed loading entrance drive to the south approximately 30' to create an off-set with the loading dock to limit visual impacts.

#### Comment No. 2

The site plan depicts 250 feet of sight distance at the proposed loading dock curb cut; however, it appears that existing vegetation would need to be removed in order to achieve the depicted sight distance. The site plan should be revised to depict the removal of the vegetation.

#### Response No. 2

The site plans have been revised to depict the removal of two trees located in the Town right-ofway to provide an unobstructed 250 feet of sight distance from both proposed driveways.

#### Comment No. 3

The proposed warehouse building does not meet the 100 foot minimum front yard setback. The Applicant will need to secure a 43 foot front yard variance from the Zoning Board of Appeals.

#### Response No. 3

Comment is so noted. The applicant intends to request an area variance from the North Castle Zoning Board of Appeals (ZBA) for the proposed building's location within the required front yard. As discussed with the Planning Board, the applicant can only be referred to the ZBA after the Planning Board has completed SEQRA and issued their environmental determination.

#### Comment No. 4

The Zoning Conformance Table should be revised and updated to utilize net lot area when calculating development density pursuant to Section 355-30.H of the Town Code.

#### Response No. 4

The Zoning Conformance Table (Chart) has been revised to depict net lot area when calculating development density.

#### Comment No. 5

The off-street parking chart should be revised to identify the proposed 44 proposed land banked parking spaces. In addition, the Applicant will need to submit written guaranties, satisfactory to the Town Attorney, for the eventual improvement of any such spaces which may have been waived. Such spaces must be constructed within six months of the date of written notice to the property owner by the Planning Board that such spaces have been determined as necessary.

#### Response No. 5

The off-street parking summary has been revised to include the proposed land banked parking spaces. A note has been added to the plans indicating that 'In all cases, it shall be expressly demonstrated on the site plan that sufficient space remains for the provision of the total amount of off-street parking required, and the site plan shall bear such designation. All such undeveloped parking space shall be used and maintained as additional landscaped grounds until required for parking. In the event that construction of the land banked spaces is deemed necessary by the Town, the applicant shall guarantee the eventual improvement of any such spaces which may have been waived. Such spaces must be constructed within six months of the date of written notice to the property owner by the Planning Board that such spaces have been determined as necessary."

#### Comment No. 6

The Byram River is located at the rear of the property. The site plan should be revised to note that 34,270 square feet of Town-regulated wetland buffer disturbance is proposed. In addition, the Applicant should prepare a mitigation plan encompassing a minimum of 68,540 square feet.

#### Response No. 6

The site plans have been revised to depict the amount of wetland buffer disturbance and the proposed mitigation area. As a result of coordination between the Project Floodplain Consultant and Town Consulting Engineer, proposed disturbance within the regulated Floodway (for future potential floodplain compensatory storage basins) has been eliminated. Furthermore, the proposed wetland buffer disturbances have been summarized below as a result of receiving the variance to eliminate the compensatory floodplain storage:

1. The previous plan, which included compensatory storage basins and work within the regulated floodway proposed 38, 840 sf of Wetland buffer disturbance. The proposed

mitigation area was 78,354 sf which yields has been a 2:1 mitigation ratio. As previously discussed, this plan was reviewed and approved by the Conservation Board.

- The current plan (without the future potential compensatory floodplain storage basins) reduces the wetland buffer disturbance to 5,000 sf. The proposed mitigation area is 36,570 sf which yields a 7.3:1 mitigation ratio.
- 3. Should the future potential compensatory floodplain storage basins be built, a wetland buffer disturbance of 27,000 sf would be required. The proposed mitigation area for the option including the compensatory storage basins is 62,893 sf which yields a 2.3:1 mitigation ratio.

#### Comment No. 7

The site plan depicts the removal of 259 Town-regulated trees.

#### Response No. 7

Comment is so noted. The site plans have been revised to depict the updated proposed amount of Town-regulated tree removal (235 trees).

#### Comment No. 8

The site plan should be revised to depict screening along the southern property line between the subject site and 130 Business Park Drive.

#### Response No. 8

The site plans have been revised to depict screening, in the form of evergreen tree plantings, between the subject property and the 130 Business Park Drive property.

#### Comment No. 9

Pursuant to Section 355-30.D(1) of the Town Code, a ten-foot-deep landscaped foundation planting shall be provided along all building walls, except at access points, in interior courts, or where waived by the Planning Board. A sidewalk not exceeding four feet in width may be located in such required foundation parking area.

#### Response No. 9

Comment is so noted.

#### Comment No. 10

Pursuant to Section 355-56.H(2) of the Town Code, the site plan should demonstrate that at least 10% of the interior of the parking area shall be curbed and landscaped with trees, shrubs and other plant material.

The zoning compliance chart has been revised to depict the bulk requirement that demonstrates at least 10% of the interior of the parking area is curbed and landscaped with trees, shrubs and other plant material.

## Comment No. 11

Pursuant to Section 355-58.C(3) of the Town Code, the site plan should be revised to depict one 40'x 14' loading space for each establishment, and one additional space for each 10,000 square feet of gross floor area or major portion thereof in excess of 4,000 square feet of gross floor area.

## Response No. 11

The site plans have been revised to depict the required number of off-street loading spaces. Sixteen total loading spaces are required for the 137,632 sf of building area (62,782 sf existing building and 74,850 sf proposed building).

#### Comment No. 12

Pursuant to Section 355-15.0 of the Town Code, the site plan should be revised to provide adequate facilities for disposal of refuse. No incinerators shall be permitted. In multifamily and nonresidential districts, all refuse disposal units or locations for deposit must also be screened from view and designed in such fashion as to be fireproof and to prevent access by rodents and blowing away of refuse.

#### Response No. 12

The site plans have been revised to propose a masonry trash enclosure at the rear of the property which limits any visual impacts from Business Park Drive and adjacent properties.

#### Comment No. 13

The Applicant should confirm that the proposed new site plan for the property would supersede the previous outdoor storage site plan and that all outdoor storage on the site would be eliminated and transferred to the new warehouse.

#### Response No. 13

As discussed with the Planning Board, the majority of the existing outdoor storage would be relocated within the proposed building, however, there will still be the need to stage materials outside from deliveries. There is a precise system in place to make sure delivered materials for each project stay together. The extents of the proposed outdoor storage areas located at the rear and side of the existing building are depicted on the site plans which limit views from Business Park Drive.

## Kellard Sessions Consulting, P.C. Memorandum, dated April 9, 2020

#### **General Comments**

## Comment No. 1

As illustrated on the plan, the project site is located partially within the FEMA regulated floodway of the Byram River and the associated 100-year floodplain with a base flood elevation (BFE) of Elevation 370.0. Development is proposed within both the floodway and floodplain. As such, the applicant will be required to obtain a Floodplain Development Permit demonstrating compliance with Chapter 177 - Flood Damage Prevention of the Town Code. The Existing Condition Site Plan shall include references to the FEMA Effective FIRM Maps. The applicant has prepared a Flood Storage Volume Analysis Plan for review; however, the required no-risk analysis has not been provided and shall be prepared for consideration.

## <u>Response No. I</u>

Comment is so noted. A reference to the effective FEMA FIRM maps has been added to the Existing Conditions Map. The applicant has submitted a "Hydrologic & Hydraulic Report", prepared by Leonard Jackson Associates, which demonstrates compliance with FEMA regulations for floodplain development. This report was previously submitted for review by the Town Consulting Engineer and is provided herein for the Planning Board's reference.

#### Comment No. 2

The plan proposes earthwork at the rear of the property to construct basins for compensatory flood water storage, as required Section 177-14 B (3) of the Town Code. The applicant has prepared an Existing and Proposed Flood Storage Volume Analysis Plan to demonstrate that adequate compensatory storage has been provided by the development. In review of the plan, we offer the following comments for consideration:

- After review of the Existing Condition Plan, it appears that the terrain model requires additional refinement and should include existing spot grades. As currently shown, areas of existing available storage within the site have not been accounted for and will be eliminated by the development, requiring additional storage to offset the loss. A copy of the TIN model should be provided.
- The Proposed Flood Storage Plan should be revised to discount the volume accounted for in the proposed stormwater management basin below Elevation 370.0. This volume must be reserved for stormwater mitigation for runoff from the building.
- In addition to the compensatory flood storage evaluation noted above and, as required by Section 177-14 B (2)(a) of the Town Code, on streams with a regulatory floodway, such as the Byram River, no new construction or development in the floodway shall be permitted unless a technical evaluation is prepared demonstrating that the development will not result in any increase in flood levels during the 100-year storm. The required study shall be prepared and submitted for review.

As noted above, the applicant has retained a floodplain consultant, Leonard Jackson Associates, to analyze the effect of the project on the Byram River. Leonard Jackson Associates has concluded that constructing compensatory floodplain storage as previously designed yields no measurable benefit to the Byram River and adjacent properties. A copy of this "Hydrologic & Hydraulic Report" is provided herein for the Planning Board's review. Furthermore, if we were to provide the compensatory storage an additional land disturbance of approximately 35,000 square feet within the 100' Town-regulated wetland buffer adjacent to the Byram River would be required. The Town Consulting Engineer has reviewed the report and in December 2020 the applicant obtained a variance from this requirement from the Town Board as permitted within Town Code Section 177-19. As additional floodplain mitigation, the applicant has agreed to develop restrictions on a property that they own upstream of the site, which is also along the Byram River.

## Comment No. 3

The Byram River flows generally north to south along the eastern property boundary. This watercourse is a locally-regulated wetland, as well as a New York State Department of Environmental Conservation (NYSDEC) Class C(T) stream. The 100-foot regulated buffer and adjacent area extends onto the property and within the proposed development area. A local Wetland Permit will be required.

#### Response No. 3

Comment is so noted.

#### Comment No. 4

The applicant has prepared a Wetland Evaluation and Impact Report, which indicates that the NYSDEC and the US Army Corps of Engineers (US ACOE) will not require permitting for the proposed development. The property is located within the check-zone of two (2) adjacent NYSDEC Freshwater Wetlands, Wetlands No. G-1 and G-2. Written confirmation from these agencies, indicating that no permits are required, should be provided to the Town for their information.

#### Response No. 4

The applicant has filed a joint permit application with the NYSDEC and US Army Corps of Engineers and has subsequently received a No Permit Required determination. All received correspondence from the NYSDEC and US Army Corps of Engineers has been included in this submission.

#### Comment No. 5

The applicant shall confirm whether the wetland boundary illustrated on the plan has been established in the field with fluorescent, sequentially-numbered ribbons. Once confirmed, please notify this office for field verification of the boundary by the Town Wetland Consultant.

The wetlands have been delineated in the field (with fluorescent, sequentially numbered ribbons) by the project Wetland Consultant (Ecological Solutions, LLC) on 10/17/2019 and we request the Town Wetland Consultant conduct a field verification of the boundary.

## Comment No. 6

The Byram River is a Westchester County controlled stream and development is proposed within 100 feet of its banks. The applicant shall provide confirmation from the Westchester County Department of Public Works (WC DPW) whether a Stream Control Permit is required.

## Response No. 6

Comment is so noted. We have confirmed with Jeffrey A. Dean, PE of the Westchester County Department of Public Works (WCDPW) that a Stream Control Permit is required for any work being proposed within 100' of the channel lines of the Byram River. A draft of this application is included herein and as noted on the application, a signature is required from the municipality prior to submission. Once we receive the signed application, the application and plans will be provided to the Westchester County Department of Public Works for their review.

## Comment No. 7

As previously indicated, the plan proposes disturbances within the locally regulated 100-foot buffer of the Byram River and associated fringe wetland area. The applicant will be required to provide mitigation at a ratio of 2:1 for unavoidable disturbances within the wetland/wetland buffer, as required by Chapter 340, Wetlands and Watercourse Protection of the Town Code. We recommend that the Planning Board refer the plan to the Conservation Board for review and consideration.

#### Response No. 7

The site plans have been revised to depict the amount of wetland buffer disturbance and the proposed mitigation area which meets the required 2:1 ratio. We have been before the Conservation Board for two consecutive meetings (05/19/2020 & 06/16/2020) and have obtained a positive recommendation back to the Planning Board for the proposed wetland buffer disturbance and associated mitigation. Furthermore, as a result of the compensatory floodplain storage variance that was obtained from the Town Board and eliminating disturbance within the regulated floodway, the proposed disturbance within the 100-foot wetland buffer has been reduced. Furthermore, the disturbance associated with this construction is not required at this time. As additional floodplain mitigation, the applicant has agreed to develop restrictions on a property that they own upstream of the site, which is also along the Byram River. The proposed wetland buffer disturbances and provided mitigation have been depicted on the site plans and have been separately quantified in a summary table.

#### Comment No. 8

As required by Town Code, the applicant will be required to provide a long-term monitoring and

maintenance plan for the proposed wetland mitigation for a period of at least five (5) years. This office will provide standard conditions for this plan for inclusion on the Site Landscaping and Wetland Mitigation Plan.

## Response No. 8

A long-term monitoring and maintenance plan for the proposed wetland mitigation for a period of at least five (5) years has been prepared and approved by the Conservation Board.

#### Comment No. 9

The wetland mitigation plan shall include a summary table illustrating and quantifying the total area of disturbance for the project, the disturbance area within the wetland and wetland buffer, existing and proposed pervious and impervious surface areas, as well as the total area of wetland mitigation proposed.

#### Response No. 9

A summary table has been prepared that depicts and quantifies the total area of the site, total area of disturbance, total area of disturbance within the wetland and wetland buffer, existing and proposed coverage calculations (impervious/pervious) and total area of proposed wetland mitigation.

## Comment No. 10

As indicated in the Wetland Evaluation and Impact Report, a total of approximately 34,270 s.f. of disturbance is proposed within the wetland buffer. This same area is proposed for wetland mitigation. As such, it appears that the 2:1 ratio of mitigation to disturbed areas has not been met by this plan. However, there seems to be other on-site areas which could be used for additional mitigation. We will defer further comment until the Conservation Board has had an opportunity to review the plan. We note that the plantings proposed for the stormwater infiltration basin are typically not credited towards wetland mitigation, as they are required for compliance with the NYSDEC Stormwater Management Design Manual (SMDM) for stormwater mitigation. Additionally, the application rate of the stormwater basin seeding should be confirmed by the applicant, as the 4 lbs./acre appears to be insufficient for adequate stabilization.

#### Response No. 10

The site plans have been revised to depict the amount of wetland buffer disturbance and the proposed mitigation area which meets the required 2:1 ratio. Furthermore, the application rate of the future potential compensatory flood storage basins, should they be constructed, has been increased to 20-40 lbs./acre or 1 lb. per 1,000 sf.

#### Comment No. 11

The Wetland Evaluation and Impact Report makes reference to debris removal along the Byram River and its banks. This work is not shown on the plans and may require permitting by the NYSDEC and/or Westchester County Department of Public Works. Please clarify this on the plans and provide confirmation from both agencies regarding the need for any permits.

All proposed trash and debris removal along the Byram River corridor will be conducted on the 100 Business Park Drive property.

#### Comment No. 12

The plan proposes the removal of 259 Town-regulated trees, six (6) of which are Significant Trees, as defined by Town Code. The Planning Board should discuss whether the amount of proposed tree removal is appropriate for the development and if any tree preservation or replacement, in addition to what is shown on the proposed Site Landscaping and Wetland Mitigation Plan, is required.

## Response No. 12

Comment is so noted. The site plans have been revised to depict the updated proposed amount of Town-regulated tree removal (235 trees).

#### Comment No. 13

The proposed southern driveway access is opposite an existing well potentially to be used as a public water supply for the Town. A portion of this driveway is located within 100 feet of the well and the access drive, a portion of the proposed land-banked parking spaces and stormwater infiltration basin lie within the 200 foot control radius. The applicant should communicate with the Town and Westchester County Department of Health (WCHD) regarding any restrictions that may result should this well be put into service. Specifically, Part 5, Appendix 5-D, Table I of the NYCRR, Public Water Systems, provides required minimum separation distances to public water supply wells. We note that the proposed stormwater infiltration basin is a potential contamination source and is located within the 200 foot control area. The basin may require relocation or be subject to monitoring.

#### Response No. 13

Based on the above comment and from additional coordination efforts with the Westchester County Department of Health (WCDH) and the Town Water Supply Consultant (WSP), the proposed stormwater basin(s) (for roof runoff only) have been relocated outside of the 200-foot control area from the future Town public water supply well.

#### Comment No. 14

The plan proposes to land-bank 44 of the 63 required off-street parking spaces at the south side of the property. The Planning Board should discuss whether this is appropriate for the project.

#### Response No. 14

Comment is so noted.

#### Comment No. 15

The plan proposes a circulation drive around the south and east perimeter of the proposed building to connect to the existing parking area and driveway access. We note that a significant portion of this proposed access drive and adjacent parking will be submerged by as much as three (3) feet of water during the 100 year storm event; presumably to help off-set the impacts to the flood plain area and lost compensatory storage volume. This office recommends that the plan be revised to eliminate this condition. The Planning Board should discuss whether this is appropriate and if any flooding of the driveway or associated parking should be permitted. At a minimum, we would recommend that the plan be referred to the Armonk Fire Department and emergency services for their review for adequate emergency access for both emergency and aerial apparatus. The applicant should provide the Board with copies of any communication from the Fire Department in this regard and modify the plan, as may be required.

## Response No. 15

Following the approval of the compensatory floodplain storage variance from the Town Board, the site grading has been revised in order to improve this condition. The lowest point of the access drive now lies I foot below the base flood elevation of the Byram River. The lowest point of the parking area lies approximately 8 inches below the base flood elevation, which was driven by providing an appropriate slope for drainage purposes.

The Armonk Fire Department had reviewed and accepted the previous proposed grading around the building, which depicted up to 4 feet of the driveway potentially being flooded during the 100-year flood event, as the configuration meets the fire code as well as the Fire Department's needs.

## Comment No. 16

The applicant should illustrate turning movements for fire apparatus vehicles around the proposed building and exiting the site. We note that, as part of the prior approval for the existing facility, the Fire Department designated certain areas for fire access. These areas should be illustrated on the plan and reviewed with the Fire Department.

#### Response No. 16

The site plans have been revised to depict fire apparatus vehicle turning analyses around the proposed building and exiting the site. The plan has been reviewed and approved by the Armonk Fire Department.

#### Comment No. 17

The plan proposes a total of eight (8) loading docks at the front of the proposed warehouse building, four (4) of which are shown as tandem spaces. The applicant should provide vehicle turning movements for the size trailer anticipated to access the site to demonstrate adequate maneuverability into the site and accessing these loading areas.

The site plans have been revised to depict delivery truck (WB-67) vehicle turning analyses demonstrating adequate maneuverability into the site and accessing the loading areas.

#### Comment No. 18

The site plan illustrates available sight line distances for vehicles exiting both proposed driveway locations. The plan should also include sight line profiles for these locations to demonstrate adequate visibility in both directions from both locations. The plan should identify whether any additional existing vegetation along the right-of-way of Business Park Drive will require removal to maintain adequate sight lines.

#### Response No. 18

The site plans have been revised to depict the removal of two trees located in the Town right-ofway to provide an unobstructed 250 feet of sight distance from both proposed driveways.

#### Comment No. 19

The plan proposes a reduction in the depth of the parking stalls to 16 feet, as permitted by Town Code Section 355-56 D. The plan should illustrate the area of the required two (2) foot bumper overhang to demonstrate that no conflicts with proposed site lighting, landscaping or otherwise will result.

#### Response No. 19

The proposed parking spaces has been revised to provide a depth of 18 feet within the primary parking stalls. The landbanked parking stalls propose a depth of 16 feet and provide an unobstructed depth of 18 feet when considering the 2 foot bumper overhang.

#### Comment No. 20

The plan proposes an accessible parking space in the front of the building, as required by Town Code. The plan should illustrate the required directional signage.

#### Response No. 20

The plans have been revised to identify the required signage for the accessible parking spaces.

#### Comment No. 21

We note that, as required by Section 177-17 of the Town Code, the proposed building floor elevation has been set two (2) feet above the Base Flood Elevation (Elevation 372.0). The plan should also illustrate the location of any electrical transformer or similar utility and note that it shall be set at an elevation no lower than the BFE (Elev. 370.0).

The proposed transformer has been located on the site plan in an area that lies above the base flood elevation and a note has been added stating such.

#### Comment No. 22

The applicant has provided a Landscape Plan for consideration by the Planning Board. We note that the proposed off-street loading spaces in the front of the building are immediately opposite the entry drive. The Board should consider weather adequate screening of this loading area has been provided by the current plan.

## Response No. 22

The proposed access drive in the front of the building has been shifted approximately 30 feet to the south to create an off-set with the loading dock to limit visual impacts.

#### Comment No. 23

The plan shall include proposed driveway profiles to demonstrate compliance with Section 355-59, Driveways of the Town Code.

#### Response No. 23

Profiles of the proposed driveways will be prepared and submitted under separate cover.

#### Comment No. 24

The plan proposes approximately 4.8 acres of disturbance, which will require the owner to obtain coverage under the NYSDEC General Permit (GP-0-20-001) for Stormwater Discharge from Construction Activities. The applicant has prepared a Stormwater Pollution Prevention Plan (SWPPP) and Erosion and Sediment Control Plan for review. We will defer a detailed review of the SWPPP until the plan is developed further. However, we offer the following preliminary comments for consideration:

- a) The proposed Water Quality Unit, located in the front of the building, is partially within the limits of an existing drainage easement in favor of the Town. The structure should be shifted to the south beyond the limits of this easement area.
- b) The plan proposes the removal and relocation of existing storm conveyances in the central portion of the site, with the exception of a final section of 24-inch corrugated metal pipe that ultimately discharges to the Byram River. The condition of this section of pipe should be inspected with this office and the Superintendent of Highways to determine whether this last section shall also be replaced.
- c) The SWPPP should include pipe capacity calculations and storm drain profiles demonstrating adequate capacity for the increased flows.
- d) The two (2) proposed compensatory storage areas are piped to permit flows to surcharge the basins

during a flood event and then recede once the storm passes. It appears, however, that the bottom elevation of the northerly basin (Elevation 386) should be lowered to Elevation 384 to allow proper drainage based on the proposed pipe layout.

- e) The plan proposes a stormwater infiltration basin, water quality treatment unit and porous pavement as stormwater management practices. A Long-Term Maintenance Agreement will be required to be put in place by the owner. A draft agreement should be provided for review by the Town Attorney.
- f) The SWPPP should include a draft copy of the Notice of Intent (NOI) for review.
- g) The plans should include a detail of the infiltration basin providing elevations for the channel protection volume, and peak water surface elevations for the 10-year and 100-year storm events.
- h) Provide confirmation as to whether the proposed stormwater basin is permitted to be located as proposed within 200 feet of the public water supply well.
- i) This office witnessed deep and percolation soil testing at various locations throughout the site. Based on the soil testing and observed groundwater elevations (approximate Elevation 363.5), it appears that the minimum three (3) foot separation from the bottom of the practice to the ground water surface, as required by the NYS SMDM, has not been provided for either the stormwater infiltration basin or the porous pavement. The plan shall be revised, as required, to maintain this separation.
- *j)* The drainage area Maps provided in the SWPPP should be supplemented with soil types and land cover areas to support the calculations in the hydrologic model.
- k) The infiltration basin calculation should be reviewed for consistency with the plan and revised, as needed. The available depth of the basin appears to only be  $2\frac{1}{2}$  feet deep, as opposed to the  $3\frac{1}{2}$  feet used in the calculation.
- I) The SWPPP shall clearly indicate the bypass rate provided by the water quality unit to ensure safe passage of the 100-year design flow.
- m) The plans and SWPPP shall provide details and documentation to support design compliance with the minimum requirements of the NYS SMDM for infiltration basins (Practice I-2). Specifically, provisions for pre-treatment of the water quality volume shall be provided with a capacity based on the infiltration rate of the underlying soil.
- n) The location of the temporary construction fence illustrated on the Erosion and Sediment Control Plan is within the boundaries of the FEMA floodway. The plan shall be revised to avoid this.
- o) The plan shall illustrate the area of the stormwater infiltration basin to be cordoned-off during construction.
- p) The sequence of construction on the plan and within the SWPPP shall be expanded to include the following, at a minimum:
  - Steps to construct the proposed stormwater infiltration basin and temporary measures to

prevent flow into the basin until stabilized and timing as to when it should be put online;

- Construction of the compensatory storage basins and the associated interconnection to the existing storm system;
- Construction of the porous pavement system and means of protection during construction;
- Wetland mitigation, landscaping and site restoration.

## Response No. 24

- a) The proposed water quality structure has been relocated so that it lies entirely out of the easement associated with the Town drainage line.
- b) The comment is so noted. It should be noted, the pipe in question is a Town owned storm pipe that has an easement across the applicant's property and conveys drainage from Business Park Drive and a portion of the existing project site. We are not aware of any issues with the performance of this pipe, however, we are awaiting the results of the Town Consulting Engineer and the Superintendent of Highways inspection of the existing pipe to remain that extends into New York State Department of Transportation (NYSDOT) property and discharges into the Byram River.
- c) Hydraulic capacity calculations will be prepared and submitted under separate cover.
- d) This comment is no longer applicable as the compensatory storage basins are no longer required. Refer to Response No. 2 from Town Planner memorandum above.
- e) A draft maintenance agreement has been prepared and included in this submission for review.
- f) A draft Notice of Intent has been prepared and included in this submission for review.
- g) Cross sections of the stormwater basins have been prepared and are provided on JMC Drawing C-904. These cross sections depict the infiltration basins and include the water surface elevations during the 1, 10, and 100 year storm events.
- h) The infiltration basin has been redesigned and split into two separate basins. The two infiltration basins are now located outside of the 200' radius (well buffer) from the public water supply well.
- i) The bottom elevations of the infiltration basins have been revised accordingly, and cross sections of the basins have been prepared. The cross sections graphically depict the groundwater elevations that were encountered in the test pits performed in each basin. In addition, a test pit summary table has been added to the Grading Plan describing the groundwater elevations encountered within each test pit.
- j) The Drainage Area Maps have been revised to graphically depict soil type boundaries and coverage types to support the calculations in the hydrologic model.
- k) The hydrologic model has been reviewed to ensure that the correct elevations and

information have been input for the proposed infiltration basins. Please see the Hydrologic Calculations within the Stormwater Pollution Prevention Plan which contain elevation-area tables for the proposed basins.

- I) The NYSDEC Sizing Calculations within the appendices of the Stormwater Pollution Prevention Plan have been revised to include the peak bypass rates for each separator. In addition, these bypass rates have been confirmed with Hydro International, the manufacturer of the First Defense units.
- m) The plans and SWPPP have been revised to provide pre-treatment of the roof runoff through a First Defense hydrodynamic separator. Please see Appendix B within the SWPPP for the associated sizing calculations.
- n) All work previously shown within the FEMA floodway has been removed including the previously proposed construction fencing.
- o) The Erosion & Sediment Control Plan has been revised to show the infiltration basins being cordoned off during construction.
- p) The sequence of construction on the Erosion & Sediment Control Plan and within the SWPPP has been revised to include the above requested details.

## Comment No. 25

The site plan illustrates utility connections for domestic water and sanitary sewer services. The applicant should clarify whether a water service for Fire Protection is required and whether any fire hydrants are proposed or required by the Armonk Fire Department in the vicinity of the proposed building. Any alternative means of Fire Protection should be noted on the plan.

#### Response No. 25

Based on discussion with the Project Architect, we understand the proposed building does not require fire protection per the New York State Building Code. Existing Fire Hydrants are located and available for use by the Fire Department along Business Park Drive. One is located perpendicularly across Business Park Drive from the existing building and the another is located perpendicularly across Business Park Drive from proposed warehouse building (±140 linear feet from the face of the proposed building).

#### Comment No. 26

The applicant has provided a Site Lighting and Photometric Plan for consideration by the Planning Board.

#### Response No. 26

This comment is so noted.

#### Comment No. 27

The applicant should indicate what fuel source is proposed to heat the space. We note that in March 2019, ConEdison imposed a moratorium on new or expansions to existing gas services.

#### Response No. 27

According to the Project Architect, it is anticipated the proposed building will be heated with electric heating due to the natural gas moratorium by Con Edison.

Comment No. 28

The plan shall include details of the storm water basin and outlet structure.

Response No. 28

The site plan has been revised to provide details for the proposed stormwater basin and construction details for the proposed outlet control structures. <u>Comment No. 29</u>

The plans include a detail for steel bollards. Please indicate their location on the site plan.

Response No. 29

The plans have been revised to provide a construction detail for the proposed steel bollards, which are depicted and labeled on the Site Layout Plan.

We trust the attached documents and above responses are sufficient for your review and look forward to being placed on the next available Planning Board agenda. Thank you for your consideration.

If you have any questions or require additional information, please do not hesitate to contact our office at (914) 273-5225.

Sincerely,

## JMC Planning Engineering Landscape Architecture & Land Surveying, PLLC

Paul Dysk

Paul R. Sysak, RLA Project Manager

Paul J. Dumont, PE Senior Designer

cc: Mr. Robert Troccoli Mr. Curt M. Johnson, R.A.

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**Applicant/Owner:** A & R REAL ESTATE HOLDINGS. LLC 100 BUSINESS PARK DRIVE **ARMONK, NY 10504** (718) 655-5450

# Architect:

J GROUP DESIGNS, LLC **63 EAST MAIN STREET** PAWLING, NY 12564 (845) 493-0235

# Wetland Consultant:

**Ecological Solutions, LLC** 1248 Southford Road Southbury, CT 06488 (203) 910-4716



Site Planner, Civil & Traffic Engineer, Surveyor and Landscape Architect: 120 BEDFORD ROAD **ARMONK, NY 10504** (914) 273-5225

# Attorney:

OCHS & GOLDBERG, LLP 60 EAST 42ND STREET, SUITE 4600 NEW YORK, NY 10165 (212) 983-1221



## GENERAL CONSTRUCTION NOTES APPLY TO ALL WORK HEREIN:

PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL CALL 811 "DIG SAFELY" (1-800-962-7962) TO HAVE UNDERGROUND UTILITIES LOCATED INCLUDING ARRANGING FOR A PRIVATE MARKOUT ON-SITE WHERE APPLICABLE. EXPLORATORY EXCAVATIONS SHALL COMPLY WITH CODE 753 REQUIREMENTS. NO WORK SHALL COMMENCE UNTIL ALL THE OPERATORS HAVE NOTIFIED THE CONTRACTOR THAT THEIR UTILITIES HAVE BEEN LOCATED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PRESERVATION OF ALL PUBLIC AND PRIVATE UNDERGROUND AND SURFACE UTILITIES AND STRUCTURES AT OR ADJACENT TO THE SITE OF CONSTRUCTION, INSOFAR AS THEY MAY BE ENDANGERED BY THE CONTRACTOR'S OPERATIONS. THIS SHALL HOLD TRUE WHETHER OR NOT THEY ARE SHOWN ON THE CONTRACT DRAWINGS. IF THEY ARE SHOWN ON THE DRAWINGS, THEIR LOCATIONS ARE NOT GUARANTEED EVEN THOUGH THE INFORMATION WAS OBTAINED FROM THE BEST AVAILABLE SOURCES, AND IN ANY EVENT, OTHER UTILITIES ON THESE PLANS MAY BE ENCOUNTERED IN THE FIELD. THE CONTRACTOR SHALL, AT HIS OWN EXPENSE, IMMEDIATELY REPAIR OR REPLACE ANY STRUCTURES OR UTILITIES THAT HE DAMAGES, AND SHALL CONSTANTLY PROCEED WITH CAUTION TO PREVENT UNDUE INTERRUPTION OF UTILITY SERVICE.

2. CONTRACTOR SHALL HAND DIG TEST PITS TO VERIFY THE LOCATION OF ALL EXISTING UNDERGROUND UTILITIES PRIOR TO THE START OF CONSTRUCTION. CONTRACTOR SHALL VERIFY EXISTING UTILITIES DEPTHS AND ADVISE OF ANY CONFLICTS WITH PROPOSED UTILITIES. IF CONFLICTS ARE PRESENT. THE OWNER'S FIELD REPRESENTATIVE, JMC, PLLC AND THE APPLICABLE MUNICIPALITY OR AGENCY SHALL BE NOTIFIED IN WRITING. THE EXISTING/PROPOSED UTILITIES RELOCATION SHALL BE DESIGNED BY JMC, PLLC.

3. CONTRACTOR IS RESPONSIBLE FOR OBTAINING ANY AND ALL LOCAL PERMITS REQUIRED.

4. ALL WORK SHALL BE DONE IN STRICT COMPLIANCE WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES, STANDARDS, ORDINANCES. RULES. AND REGULATIONS. ALL CONSTRUCTION WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL SAFETY CODES. APPLICABLE SAFETY CODES MEAN THE LATEST EDITION INCLUDING ANY AND ALL AMENDMENTS, REVISIONS, AND ADDITIONS THERETO, TO THE FEDERAL DEPARTMENT OF LABOR, OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION'S OCCUPATIONAL SAFETY AND HEALTH STANDARDS (OSHA): AND APPLICABLE SAFETY, HEALTH REGULATIONS AND BUILDING CODES FOR CONSTRUCTION IN THE STATE OF NEW YORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR GUARDING AND PROTECTING ALL OPEN EXCAVATIONS IN ACCORDANCE WITH THE PROVISION OF SECTION 107-05 (SAFETY AND HEALTH REQUIREMENTS) OF THE NYSDOT STANDARD SPECIFICATIONS. IF THE CONTRACTOR PERFORMS ANY HAZARDOUS CONSTRUCTION PRACTICES, ALL OPERATIONS IN THE AFFECTED AREA SHALL BE DISCONTINUED AND IMMEDIATE ACTION SHALL BE TAKEN TO CORRECT THE SITUATION TO THE SATISFACTION OF THE APPROVAL AUTHORITY HAVING JURISDICTION.

5. CONTRACTOR SHALL MAINTAIN ACCESS TO ALL PROPERTIES AFFECTED BY THE SCOPE OF WORK SHOWN HEREON AT ALL TIMES TO THE SATISFACTION OF THE OWNERS REPRESENTATIVE. RAMPING CONSTRUCTION TO PROVIDE ACCESS MAY BE CONSTRUCTED WITH SUBBASE MATERIAL EXCEPT THAT TEMPORARY ASPHALT CONCRETE SHALL BE PLACED AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SAFE PEDESTRIAN ACCESS AT ALL TIMES. 6. CONTRACTOR SHALL MAINTAIN THE INTEGRITY OF EXISTING PAVEMENT TO REMAIN.

. THE TOWN OF NORTH CASTLE IS PURSUING THE OWNERSHIP OF AN EXISTING WATER SUPPLY WELL LOCATED AT 125 BUSINESS PARK DRIVE IN THE EFFORT TO TRANSITION THE WELL TO SERVE THE PUBLIC WATER SUPPLY. WELLS SERVING PUBLIC WATER SYSTEMS SHALL BE LOCATED SUCH THAT THE OWNER OF THE WATER SYSTEM POSSESSES LEGAL TITLE TO LANDS WITHIN 100' OF THE WELL AND THE OWNER CONTROLS BY OWNERSHIP. LEASE. EASEMENT OR OTHER LEGALLY ENFORCEABLE ARRANGEMENT THE LAND USE ACTIVITIES WITHIN 200' OF THE WELL. HYDROGEOLOGIC EVALUATIONS AND SOURCE WATER ASSESSMENTS SHOULD BE USED TO DETERMINE APPROPRIATE SEPARATION FROM POTENTIAL CONTAMINANT SOURCES.

# SITE PLAN APPROVAL DRAWINGS **PROPOSED WAREHOUSE** TAX MAP SECTION 108.03 | BLOCK 1 | LOT 51 WESTCHESTER COUNTY **100 BUSINESS PARK DRIVE** TOWN OF NORTH CASTLE, NEW YORK





**PERSPECTIVE RENDERING** SCALE: N.T.S.

# JMC Drawing List:

<b>C-000</b>	COVER SHEET
C-010	<b>OVERALL SITE EXISTING CONDITIONS M</b>
C-011	SITE EXISTING CONDITIONS MAP
C-020	SITE DEMOLITION & TREE REMOVAL PLA
C-100	SITE LAYOUT PLAN
C-110	TRUCK TURNING ANALYSES
C-120	FIRE APPARATUS TURNING ANALYSIS
C-200	SITE GRADING PLAN
C-300	SITE UTILITIES PLAN
<b>C-400</b>	SITE EROSION & SEDIMENT CONTROL P
<b>C-500</b>	SITE LANDSCAPING & WETLAND MITIGA
<b>C-600</b>	SITE LIGHTING PLAN
<b>C-900</b>	CONSTRUCTION DETAILS
C-901	CONSTRUCTION DETAILS
C-902	CONSTRUCTION DETAILS
<b>C-903</b>	CONSTRUCTION DETAILS
<b>C-904</b>	CONSTRUCTION DETAILS
C-905	CONSTRUCTION DETAILS

# J GROUP DESIGNS, LLC Drawing List:

**SCHEMATIC PLAN & ELEVATIONS** Δ2 SCHEMATIC EXTERIOR VIEWS

ZONING COMPLIANCE CHART						
TAX PARCEL: 108.03–1–51 ZONE DISTRICT: PLI – PLANNED LIGHT INDUSTRY PROPOSED USE: WAREHOUSE						
DESCRIPTION		REQUIREMENT	EXISTING	PROPOSED		
MINIMUM LOT AREA	(ACRES)	4	11.26	11.26		
NET LOT AREA	(ACRES)	-	10.80	10.80		
MINIMUM LOT FRONTAGE	(FEET)	300	1,215	1,215		
MINIMUM LOT DEPTH	(FEET)	300	409	409		
MAXIMUM BUILDING HEIGHT	(STORIES/FEET)	3/35	2/-	1/25		
MAXIMUM BUILDING COVERAGE	(%)	30	10.29	26.20		
FLOOR AREA RATIO		0.30	0.10	0.26		
MINIMUM INTERIOR LANDSCAPED AREA	(%)	10	14	11		
MINIMUM BUILDING SETBACKS						
FRONT YARD	(FEET)	100	100.6	57 <sup>(1)</sup>		
SIDE YARD	(FEET)	50	305.3	63		
REAR YARD	(FEET)	100	118	117		
PARKING SPACES						
STANDARD PARKING SPACES	(SPACES)	(SEE TABLE)	46	212		
ACCESSIBLE PARKING SPACES	(SPACES)	(SEE TABLE)	6	8		
TOTAL PARKING SPACES	(SPACES)	(SEE TABLE)	152	220		

<u>NOTES:</u>

1. VARIANCE REQUIRED.

	PARKING	CALCULATION	SUMMARY
DESCRIPTION	AREA (SF)	REQUIREMENT	PARKING REQU
EXISTING OFFICE	14,555	1 SPACE / 250 SF	58
EXISTING WAREHOUSE	36,625	1 SPACE / 1,200 SF + 1 SPACE FOR EACH	31
PROPOSED WAREHOUSE	74,850	PARKED ON THE SITE	63
EXISTING RECREATION CENTER AREA: -DANCE - RECREATION CENTER -WRESTLING - RECREATION CENTER	11,602	1 SPACE / 200 SF + 3 SPACES FOR THE DANCE STUDIO EMPLOYEES AT THE LARGEST SHIFT + 2 SPACES FOR THE WRESTLING STUDIO EMPLOYEES AT THE LARGEST SHIFT	63
TOTAL	137,632	_	215

\*INCLUDING 8 ADA ACCESSIBLE PARKING SPACES



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LAN **ATION PLAN** 

PARKING PROVIDED IRED 220\*



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- - 1. ALL PLANT MATERIAL SPECIFIED ON THE DRAWINGS NURSERY GROWN STOCK, CERTIFIED TRUE TO THEIR VARIETY, ALL OF WHICH SHALL CONFORM TO THE " NURSERY STOCK PUBLISHED BY AMERICANHORTI (AN
  - 2. ALL AREAS OF THE SITE NOT OCCUPIED BY BUILDING
  - IMPERVIOUS SURFACE, AND NOT SPECIFIED AS BEING SHRUBS OR GROUND COVER, SHALL BE LAWN. 3. ALL MULCH TO BE PLACED IN PLANTING BEDS SHAL
  - TOXIC FREE, ORGANIC MATERIAL, CONSISTING OF SHE MULCH SHREDDED CEDAR, OR BARK CHIPS, AS APPR FIELD REPRESENTATIVE OR PROJECT LANDSCAPE ARC THICKNESS OF THE MULCH SHALL BE 3" AND/OR AS
  - 4. PLANT MATERIAL SUBSTITUTIONS SHALL NOT BE PER PROJECT LANDSCAPE ARCHITECTS WRITTEN APPROVA
- - 5. ALL LANDSCAPE PLANTINGS SHALL BE MAINTAINED I CONDITION THROUGHOUT THE DURATION OF THE PRO SO MAINTAINED SHALL BE REPLACED WITH NEW PLAI OF THE NEXT, IMMEDIATELY FOLLOWING PLANTING SE 6. ALL PLANT MATERIAL SHALL BE SUBJECT TO THE A LANDSCAPE ARCHITECT AND GOVERNMENTAL AUTHOR B & B B & B B & B B & B ROO B&B B & B B & E
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    - PERIODICALLY INSPECT PLANTING AREAS TO SELECTIVELY REMOVE, BY HAND,
    - 2. SELECTIVELY WEED-WACK REMAINING LAWN AREA WITHIN PLANTING MITIGATION AREA, AS NEEDED, SO AS TO REMOVE ANY UNWANTED WEED GROWTH (WEEDS) OVER PERIOD OF NEXT 2-3 YEARS. THIS SHOULD BE PERFORMED BIANNUALLY
    - 3. REMOVAL OF LEAF LITTER SHOULD BE LIMITED TO HAND CLEARING OF SEEDED
    - CONSULTANT, APPROVED BY THE TOWN OF NORTH CASTLE AND THE PROPERTY OWNER. TO CONDUCT TWO (2) FIELD INSPECTIONS AND PREPARE AN ANNUAL
    - . SEED MIXTURE: SUPPLEMENTAL WETLAND SEED MIX IS TO BE APPLIED TO FLOODPLAIN STORAGE BASINS AT 20-40 LBS/ACRE OR 1 LB PER 1,000 PER ACRE AND CONSIST OF FOLLOWING:

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										Drawing No:	-600

![](_page_30_Figure_0.jpeg)

![](_page_31_Figure_0.jpeg)

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![](_page_32_Figure_0.jpeg)

![](_page_33_Figure_0.jpeg)

Product De The Noctura <sup>TM</sup> performance a coordinated appearance a lexibility and Applications: applications. Performan	Escription LED Area luminaire and quality backed b style across area, flucture cross a building site ease of application f Ideal for both retrofience Stree Summary	e minimizes cost y a Cree Lighting bod and wall moi or campus. Vers or direct pole, te t and new install	of ownership with ea 5-year limited warra unt luminaires to prov atile mounting config non or vertical flat su ation: parking area, s	sy installation, high Inty. The Noctura Series vide a consistent daytim urations offer installatio rface mounting. security, or site pathway	has e on	11L Direct M NTA-DA	ount BZ			
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Backlight Shiel NTA-BLSF-11L - For use with 1 NTA-BLSF-19L - For use with 1 NTA-BLSF-35L	ld Bir - NT 11L only - F - NT 19L only - F	<b>d Spikes</b> A-BRDSPK-19L or use with 19L only A-BRDSPK-35L or use with 35L only	<b>Cable</b> NTX-QC-5 - 5' (1.5m) d	able replacement			¥	6.2"		
Mount (Lumin Direct Arm Mo NTA-DA-BZ	naire must be ordered su	aire: NTA-A-NM-T eparately)* Adjustable Arm I NTX-AA-BZ	3-11L-40K-UL-BZ	runnion Mounts				"C"		
- Direct Arm M NTX-3R-BZ - Extended Arm	ount n includes a NEMA®	- Adjustable Arm for Field-Installed	Mount W/knockout - N Button Photocell (by -	Irunnion Mount ITX-TR-3R-BZ Trunnion Mount w/NEMA® 3	-Pin	Lumen Package	Weight**	Dim "A"	Dim "B"	Dim "C"
- Must be used	an Occupancy Sensor with NTA-DA-BZ mount	• Adjustable Arm	Nount w/NEMA® -	Trunnion Mount w/knockout	for	11L*	12.5 lbs. (5.7kg)	10.6" (269mm)	12.9" (328mm)	18.7" (476mm)
		NTX-3R-BZ - Extended Arm in	ncludes a NEMA®	others) ITX-3R-BZ		19L*	16.0 lbs. (7.3kg)	15.4" (391mm)	13.8" (351mm)	19.6" (499mm)
		knockout for an - Must be used w	Occupancy Sensor th NTX-AA-BZ mount	3-Pin Photocell Receptacle knockout for an Occupancy	and a Sensor	35L*	21.1 lbs. (9.6kg)	15.4" (391mm)	19.8" (502mm)	25.6" (650mm)
Note: Photocell o	or shorting cap by others		-	Must be used with NTX-TR-I	3Z mount	* See page 3-5 ** Includes ho	for additional line using & mount (fo	e drawings or housing weight	only refer to pag	e 2)
NTA	Α	NM							BZ	
Product	Version	Mounting	Optic	Lumen Package**	ССТ		Voltage		Color Options	
NTA	A	NM No Mount	T3* Type III Medium T4* Type IV Medium T5 Type V Medium	<b>11L</b> 11,000 lumens <b>19L</b> 19,000 lumens <b>35L</b> 35,000 lumens	30K*** 3000K 40K 4000K 50K 5000K	6	UL Universal 120-277V UH Universal 347-480V		<b>BZ</b> Bronze	

LED Wall Mount Luminaires
Product Description
The Noctura <sup>™</sup> LED Wall Mount luminaire minimizes cost of ow performance and quality backed by a Cree Lighting 5-year lim a coordinated style across area, flood and wall mount luminair appearance across a building site or campus. The rugged yet l mounting box are designed for easy installation of a variety of octagonal, single gang (vertical or horizontal mounts). Lumina or conduit entry from the top, sides and rear. Luminaires (7L a conduit entry from the top, bottom, sides and rear. <b>Applications:</b> Ideal for both retrofit and new installation: build applications.
Performance Summary

![](_page_33_Picture_5.jpeg)

![](_page_34_Figure_0.jpeg)

	STRUCTURE CHART							
BASIN	STRUCTURE	PIPE/ORIFICE INVERT AND GRATE ELEVATIONS						
No.	No.	WEIR (A)	GRATE (B)	PIPE OUT (C)				
A	OCS-A	_	370.50	24" @ 366.90				
В	OCS-B	4'@ 370.00	370.50	15"@ 365.00				

	30	Drawing No:
		Scale:         NOT TO SCALE           Date:         01/25/2021           Project No:         19124           19124-DETAILS         DET-5
		Drawn: NC Approved: DI
350 5		ANY ALTERATION OF PLANS, SPECIFICATIONS, PLATS AND REPORTS BEARING THE SEAL OF A LICENSED PROFESSIONAL ENGINEER OR LICENSED LAND SURVEYOR IS A VIOLATION OF SECTION 7209 OF THE NEW YORK STATE EDUCATION LAW, EXCEPT AS PROVIDED FOR BY SECTION 7209, SUBSECTION 2.
400		CONSTRUCTION DETAILS PROPOSED WAREHOUSE 100 BUSINESS PARK DRIVE TOWN OF NORTH CASTLE, NEW YORK
	X	JMC Planning, Engineering, Landscape Architecture & Land Surveying, PLLC JMC Site Bevelopment Consultants, LLC John Meyer Consulting, Inc. 120 BEDF0RD R0AD • ARMONK, NY 10504 voice 914.273.5225 • fax 914.273.2102 www.jmcpllc.com
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# **STORMWATER POLLUTION PREVENTION PLAN**

# **PROPOSED WAREHOUSE** 100 BUSINESS PARK DRIVE TOWN OF NORTH CASTLE, NEW YORK

#### Applicant/Owner: **A&R Real Estate Holdings, LLC**

100 Business Park Drive Armonk, NY 10504 Contact: Mr. Robert Troccoli Phone: (718) 655-5450

Prepared by:



03/23/2020

JMC Planning Engineering Landscape Architecture & Land Surveying, PLLC 120 Bedford Road Armonk, NY 10504

**JMC Project 19124** 

Dated: Revised:

JMC Planning Engineering Landscape Architecture & Land Surveying, PLLC | JMC Site Development Consultants, LLC

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III.	STUDY METHODOLOGY	8
IV.	EXISTING CONDITIONS	11
V.	PROPOSED CONDITIONS	13
VI.	SOIL EROSION & SEDIMENT CONTROL	18
VII.	CONSTRUCTION PHASE AND POST-CONSTRUCTION MAINTENANC	E33
VIII.	CONCLUSION	35

#### APPENDICES

#### FIGURES DESCRIPTION

I. Site Location Map

## APPENDIX DESCRIPTION

- A. Hydrologic Calculations
- B. NYSDEC Stormwater Sizing Calculations
- C. Soil Testing Data
- D. Hydro International First Defense Operation and Maintenance Manual
- E. Temporary Erosion and Sediment Control Inspection and Maintenance Checklist

Permanent Stormwater Practice Operation, Maintenance and Management Inspection Checklists

- F. Contractor's Certification
- G. Drawings

DA-I "Existing Drainage Area Map"

DA-2 "Proposed Drainage Area Map"

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## **REFERENCED DRAWINGS FOR SWPPP DESIGN AND DETAILS**

# JMC SITE PLANS

<u>Dwg. No.</u>	<u>Title</u>	<u>Rev. No./Date</u>
C-000	"Cover Sheet"	I 01/25/202
C-010	"Overall Existing Conditions Map"	I 01/25/202
C-011	"Existing Conditions Map"	I 01/25/202
C-020	"Site Demolition and Tree Removal Plan"	I 01/25/202
C-100	"Site Layout Plan"	I 01/25/202
C-200	"Site Grading Plan"	I 01/25/202
C-300	"Site Utilities Plan"	I 01/25/202
C-400	"Site Erosion & Sediment Control Plan"	I 01/25/202
C-500	"Site Landscaping & Wetland Mitigation Plan"	2 01/25/202
C-900	"Construction Details"	I 01/25/202
C-901	"Construction Details"	I 01/25/202
C-902	"Construction Details"	I 01/25/202
C-903	"Construction Details"	I 01/25/202



#### I. INTRODUCTION

This Stormwater Pollution Prevention Plan has been prepared for the 11.26 acre site located at 100 Business Park Drive, in the Town of North Castle, Westchester County, New York (hereinafter referred to as the "Site"). The site is bordered by the La Quinta Hotel site to the north, the 130 Business Park Drive office building to the south, the Byram River and Interstate 684 to the east, and Business Park Drive to the west. The development has been designed in accordance with the following:

- Requirements of the New York State Department of Environmental Conservation (NYSDEC) SPDES General Permit No. GP-0-20-001, effective January 29, 2020.
- Chapter 267 "Stormwater Management" of the Town of North Castle Code

The project consists of the construction of a 74,850 SF warehouse on the currently undeveloped portion of the site. Associated improvements are proposed consisting of off-street parking, access driveways, loading areas, floodplain compensatory storage, stormwater management facilities, landscaping, and wetland mitigation.

#### II. STORMWATER MANAGEMENT PLANNING

In order to be eligible for coverage under the NYSDEC SPDES General Permit No. GP-0-20-001 for Stormwater Discharges from Construction Activities, the Stormwater Pollution Prevention Plan (SWPPP) includes stormwater management practices (SMP's) from the publication "New York State Stormwater Management Design Manual," last revised January 2015.

A Stormwater Pollution Prevention Plan has been prepared for this project because it is a construction activity that involves:

• Soil disturbances of one (1) or more acres of land.

The proposed stormwater facilities have been designed such that the quantity and quality of stormwater runoff during and after construction are not adversely altered or are enhanced when compared to pre-development conditions.

#### The Six Step Process for Stormwater Site Planning and Practice Selection

Stormwater management using green infrastructure is summarized in the six step process described below. The six step process was adhered to when developing this SWPPP. Information is provided in this SWPPP which documents compliance with the required process as follows:

#### Step I: Site Planning

Implement planning practices that protect natural resources and utilize the hydrology of the site. Strong consideration must be given to reducing impervious cover to aid in the preservation of natural resources including protecting natural areas, avoiding sensitive areas and minimizing grading and soil disturbance.

#### Step 2: Determine Water Quality Treatment Volume (WQv)

Determine the required WQv for the site based on the site layout, impervious areas and subcatchments. This initial calculation of WQv will have to be revised after green infrastructure techniques are applied. The following method has been used to calculate the WQv.

• <u>90% Rule</u> - According to the New York State Stormwater Design Manual, Section 4.1, the water quality volume is determined from the 90% rule. The method is based on 90% of the average annual stormwater runoff volume which must be provided due to impervious surfaces. The Water Quality Volume (denoted as the WQv) is designed to improve water quality sizing to capture and treat 90% of the average annual stormwater runoff volume. The WQv is directly related to the amount of impervious cover created at a site. The average rainfall storm depth for 90% of storms in New

York State in one year is used to calculate a volume of runoff. The rainfall depth depends on the location of the site within the state. From this depth of rainfall, the required water quality volume is calculated.

# <u>Step 3: Runoff Reduction Volumes (RRv) by Applying Green Infrastructure Techniques and</u> <u>Standard SMP's</u>

RRv is required for this project since it is a combination of both new development and redevelopment.

Green infrastructure techniques or standard SMP's with RRv capacity can potentially reduce the required WQv by incorporating combinations of green infrastructure techniques and standard SMP's within each drainage area on the site.

Green infrastructure techniques are grouped into two categories:

- Practices resulting in a reduction of contributing area such as preservation/restoration of conservation areas, vegetated channels, etc.
- Practices resulting in a reduction of contributing volume such as green roofs, stormwater planters, and rain gardens.

Apply a combination of green infrastructure techniques and standard SMPs with RRv capacity to provide 100% of the WQv calculated in Step 2. If the RRv calculated in this step is greater than or equal to the WQv in Step 2, the RRv requirement has been met and Step 4 can be skipped. If the RRv provided cannot meet or exceed 100% of the WQv, the project must, at a minimum, reduce a percentage of the runoff from impervious areas to be constructed on the site. The percent reduction is based on the Hydrologic Soil Group(s) (HSG) of the site and is defined as Specific Reduction Factor (S).

The following green infrastructure techniques and practices are provided in the Design Manual:

#### • Conservation of Natural Areas

• The entire site is developed and has been for decades. There are no undisturbed areas that could be planned to be included within a conservation easement. Therefore, there is no area to be subtracted from the contributing area for the WQv calculation.

### • Sheet flow to Riparian Buffers or Filter Strips

• There are no well vegetated areas on-site with acceptable slopes that lend an opportunity as a buffer and still meet the minimum contributing length of flow. This practice is not practical for this project since these items are typically used in a residential application.

#### • Vegetated Swales

• The use of sheet flow into vegetated swales is not practicable due to limited flow lengths, and a lack of sufficient head / elevation on the site.

#### • Tree Planting / Tree Pits

• The project includes extensive tree planting around its perimeter as part of the proposed landscaping plan. However, the new trees are not credited towards area reduction for the water quality volume.

## • Disconnection of Rooftop Runoff

• This practice is not practical for this project since these items are typically used in a residential application for small rooftop areas.

#### • Stream Daylighting

• This practice is not possible for this project since there are no existing streams on the property which are currently piped / covered.

#### • Rain Gardens

• This practice is not practical for this project since a contributing drainage area is limited to 1,000 square feet of rooftop. This practice is typically used in a residential application.

#### Green Roofs

• This practice is not practicable due to the design and size of the proposed warehouse building.

#### • Stormwater Planters

 Infiltration planters are typically proposed at various locations around proposed buildings to collect and infiltrate runoff from portions of the building rooftops. Small drainage areas, less than 15,000 square feet can be collected by roof drains and discharged into stormwater planters which infiltrate stormwater prior to entering the underground storm pipes. Stormwater planters are not practicable due to the number needed in addition to the site only having small landscaped areas around the building that would be impractical for stormwater planters.

#### • Rain Barrels and Cisterns

 Underground storage tanks installed to collect stormwater runoff to be used for irrigation purposes are impractical since the project will not have an irrigation system for the limited landscaped areas.

#### • Porous Paving

 This practice is being utilized within the new access driveway and associated parking areas. Porous pavement can be used to provide RRv because the soil on-site is classified as hydrologic soil group B. The other paved areas of the site are not acceptable for porous pavement because they will be high traffic areas, and separation to groundwater is not feasible.

#### • Standard Practices with RRv Capacity

- Biofilters and Bioretention Basins These practices cannot be proposed because the soil within the areas that have the ability to accommodate the practices has observed groundwater elevations that are too close to the surface which would not provide the required separation from the bottom of the practice to groundwater.
- Infiltration Practices Infiltration basins are proposed to treat and retain runoff from the proposed building, which comprises the majority of the new impervious area on-site.

The Minimum RRv capacity required must be provided by green infrastructure techniques to verify that the RRv requirement has been met. The RRv that is provided by the green

infrastructure techniques can then be subtracted from the Total Required WQv that must be provided by the SMP's.

#### Step 4: Determine the minimum RRv Required

The minimum RRv is calculated similar to the WQV. However, it is determined using only the new impervious cover and accounts for the hydrologic soil group present. In no case shall the runoff reduction achieved from the newly constructed impervious area be less than the minimum runoff reduction volume ( $RRv_{min}$ ).

## <u>Step 5: Apply Standard Stormwater Management Practices to Address Remaining Water Quality</u> <u>Volume</u>

Apply the standard SMP's to meet additional water quality volume requirements that cannot be addressed by applying the green infrastructure techniques. The standard SMP's with RRv capacity must be implemented to verify that the RRv requirement has been met.

 Infiltration Practices – Infiltration basins are proposed to treat and retain runoff from the proposed building. This practice is located in an area where the groundwater elevation is acceptable to provide the required separation. According to Section 3.6 of the Design Manual, 90% of the WQv provided by an Infiltration Practice can be applied towards meeting the RRv criteria.

<u>Step 6: Apply Volume and Peak Rate Control Practices to Meet Water Quantity Requirements</u> The Channel Protection Volume (CPv), Overbank Flood Control (Qp) and Extreme Flood Control (Qf) must be met for the plan to be completed. This is accomplished by using practices such as infiltration basins, dry detention basins, etc. to meet water quantity requirements. The following standards must be met:

#### I. Stream Channel Protection (CPv)

Stream Channel Protection Volume Requirements (CPv) are designed to protect stream channels from erosion. In New York State this goal is accomplished by providing 24-hour extended detention of the one-year, 24-hour storm event, remained from runoff reduction. Reduction of runoff for meeting stream channel protection objectives, where site conditions allow, is encouraged and the volume reduction achieved through green infrastructure can be deducted from CPv. Trout waters may be exempted from the 24-hour ED requirement, with only 12 hours of extended detention required to meet this criterion. Detention time may be calculated using either a center of mass method or plug flow calculation method.

#### 2. Overbank Flood (Qp) which is the 10 year storm.

Overbank control requires storage to attenuate the post development 10-year, 24-hour peak discharge rate (Qp) to predevelopment rates.

The overbank flood control requirement (Qp) does not apply in certain conditions, including:

- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams.
- A downstream analysis reveals that overbank control is not needed.

#### 3. Extreme Storm (Qf) which is the 100 year storm.

100 Year Control requires storage to attenuate the post development 100-year, 24hour peak discharge rate (Qf) to predevelopment rates. The 100-year storm control requirement can be waived if:

- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams.
- Development is prohibited within the ultimate 100-year floodplain
- A downstream analysis reveals that 100-year control is not needed.
- If redevelopment results in no increase in impervious area or changes to hydrology that increase the discharge rate from the site the hundred-year criteria does not apply.

Based on the foregoing, this project is eligible for coverage under NYSDEC SPDES General Permit No. GP-0-20-001.

#### III. STUDY METHODOLOGY

Runoff rates were calculated based upon the standards set forth by the United States Department of Agriculture Natural Resources Conservation Service Technical Release 55, <u>Urban</u> <u>Hydrology for Small Watersheds</u> (TR-55), dated June 1986. The methodology set forth in TR-55 considers a multitude of characteristics for watershed areas including soil types, soil permeability, vegetative cover, time of concentration, topography, rainfall intensity, ponding areas, etc.

The I, I0, and I00 year storm recurrence intervals were reviewed in the design of the stormwater management facilities (see Appendix A for the supporting Hydrologic Calculations).

Anticipated drainage conditions were analyzed taking into account the rate of runoff which will result from the construction of buildings, parking areas and other impervious surfaces associated with the site development.

#### Base Data and Design Criteria

For the stormwater management analysis, the following base information and methodology were used:

- The site drainage patterns and outfall facilities were reviewed by JMC personnel for the purpose of gathering background data and confirming existing mapping of the watershed areas.
- 2. An Existing Drainage Area Map was developed from the topographical survey. The drainage area map reflects the existing conditions within and around the project area.
- 3. A Proposed Drainage Area Map was developed from the proposed grading design superimposed over the topographical survey. The drainage area map reflects the proposed conditions within the project area and the existing conditions to remain in the surrounding area.
- 4. The United States Department of Agriculture (USDA) Web Soil Survey of the site available on its website at <a href="http://websoilsurvey.nrcd.usda.gov">http://websoilsurvey.nrcd.usda.gov</a>.
- 5. <u>Soil Survey of Putnam and Westchester Counties</u>, 1994.
- 6. The United States Department of Agriculture Natural Resources Conservation Service <u>National Engineering Handbook, Section 4 - Hydrology"</u>, dated March 1985.
- The United States Department of Agriculture Natural Resources Conservation Service Technical Report No. 55, <u>Urban Hydrology for Small Watersheds</u> (TR-55), dated June 1986.
- United States Department of Commerce Weather Bureau Technical Release No. 40 <u>Rainfall Frequency Atlas of the United States</u>.

The time of concentration was calculated using the methods described in Chapter 3 of TR-55, Second Edition, June 1986. Manning's kinematics wave equation was used to determine the travel time of sheet flow. The 2-year 24 hour precipitation amount of 3.4 inches was used in the equation for all storm events. The travel time for shallow concentrated flow was computed using Figure 3-1 and Table 3-1 of TR-55. Manning's Equation was used to determine the travel time for channel reaches.

- 9. All hydrologic calculations were performed with the Bentley PondPack software package version 10.0.
- 10. The New York State Stormwater Management Design Manual, revised January 2015.
- <u>New York Standards and Specifications for Erosion and Sediment Control</u>, November 2016.
- 12. The storm flows for the 1, 10, and 100 year recurrence interval storms were analyzed for the total watershed areas. The Type III distribution design storm for a 24 hour duration was used and the mass rainfall for each design storm was taken from the <u>Extreme</u> <u>Precipitation in New York & New England developed by the Natural Resource</u> <u>Conservation Service (NRCS) and the Northeast Regional Climate Center (NRCC)</u> as follows:

#### 24 Hour Rainfall Amounts

Design Storm Recurrence Interval	Inches of Rainfall
l Year	2.81
I0 Year	5.12
100 Year	9.15

#### IV. EXISTING CONDITIONS

The project site is 11.26 acres, with the 7.75 acre northern portion of the site being developed and the 3.51 acre southern portion of the site being undeveloped. The currently developed portion of the site consists of a 62,782 square foot office/light industrial building with associated off-street parking and driveways. The undeveloped portion of the site consists of woods, grassed areas, and floodplain areas. After stormwater runoff exits the project site, it flows to the Byram River directly to the east of the site. The undeveloped portions of the property flow overland to the Byram River and the developed portions of the site drain to existing conveyance systems which pipe stormwater runoff to the Byram River.

The following natural features, conservation areas, resource areas and drainage patterns of the project site have been identified and utilized to develop Drawing DA-I "Existing Drainage Area Map" which is included in Appendix G:

- Wetlands (jurisdictional, wetland of special concern)
- Waterways (major, perennial, intermittent, springs)
- Buffers (stream, wetland, forest, etc.)
- Floodplains
- Forest, vegetative cover
- Topography (contour lines, existing flow paths, steep slopes, etc.)
- Soil (hydrologic soil groups, highly erodible soils, etc.)

Based on the USDA Web soil survey, all on-site soils are moderately well drained / poorly drained and belong to hydrological groups B / D. The soil types, boundaries and drainage areas/designations are depicted on Drawing DA-1 within Appendix G.

One Design Line (analysis boundary) was identified for comparing peak rates of runoff in existing and proposed conditions. Similarly, two drainage areas were identified in existing conditions based on the existing drainage divides at the site. The following is a description of each of the drainage areas analyzed in the existing conditions analysis:

<u>Existing Drainage Area IA (EDA-IA)</u> is 4.96 acres in size and is located within the center of the site. This area consists of the existing building, southern parking / outdoor storage areas, existing loading area, and lawn / wooded areas in the rear of the property. This drainage area drains towards the existing conveyance system which pipes flows to the Byram River.

The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 88 and 10 minutes, respectively. Refer to Drawing DA-1 in Appendix G.

Existing Drainage Area IB (EDA-IB) is 3.29 acres in size and is located on the southern undeveloped portion of the site. This area consists of existing lawn and wooded areas. This drainage area drains towards the existing southern parking area, where it then enters the existing conveyance system.

The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 56 and 31.8 minutes, respectively. Refer to Drawing DA-1 in Appendix G.

The peak rates of runoff to the design points from the drainage areas for each storm are shown in the table below:

<u>Table I</u>		
Summary of Peak Rates of Runoff in Existing Condition		
(Cubic Feet per Second)		

Storm Recurrence Interval	DL-I
l year	7.71
10 year	17.98
100 year	38.16

#### V. PROPOSED CONDITIONS

The project consists of the construction of a 74,850 SF warehouse on the 3.51 acre southern portion of the site that is currently undeveloped. Associated improvements are proposed consisting of off-street parking, access driveways, loading areas, floodplain compensatory storage, stormwater management facilities, landscaping, and wetland mitigation.

The proposed drainage improvements include a variety of stormwater practices, such as an infiltration basin, areas of porous pavement, and a hydrodynamic separator to treat areas of redevelopment. After treatment for water quality and peak rate attenuation, stormwater discharges from the practices will drain to the existing conveyance system, which pipes flows to the existing discharge point into the Byram River. The proposed practices provide multiple opportunities for water quality enhancement and infiltration in addition to the proposed stormwater management basins.

This section describes the design and analysis of the proposed conditions used to demonstrate that the SWPPP meets the requirements of the General Permit.

#### The Six Step Process For Stormwater Site Planning and Practice Selection

#### Step I: Site Planning

The following practices and site features were incorporated in the site design:

- Preserving hydrology Maintaining drainage divides
- Wetlands and buffers The Byram River lies immediately to the east of the project site, and the site includes 1.73 acres of wetland buffer. The project requires the disturbance of 0.79 acres of wetland buffer.
- Floodplain considerations The site lies within the 100 year flood zone according to the National Flood Insurance Program Flood Insurance Rate Map (FIRM) No. 36119C0277F, effective date 09/28/2007.

- Forest, vegetative cover The maximum amount of forest and vegetative cover has been maintained and/or provided.
- Topography (contour lines, existing flow paths, steep slopes, etc.) has been maintained or disturbed to the minimum extent practicable.
- Soil (hydrologic soil groups, highly erodible soils, etc.)

## Step 2: Determine Water Quality Treatment Volume (WQv)

The 90% rule was used to calculate the required Water Quality Volume. Please refer to Appendix 'B' for the required Water Quality Volume calculations.

# <u>Step 3: Runoff Reduction Volumes (RRv) by Applying Green Infrastructure Techniques and</u> <u>Standard SMP's</u>

- Porous Paving
- Infiltration Basins

## Step 4: Determine the minimum RRv Required

 $RRv_{min}$  calculations can be found in Appendix 'B'.  $RRv_{min}$  was met through the proposed infiltration basin and areas of porous pavement.

## <u>Step 5: Apply Standard Stormwater Management Practices to Address Remaining Water Quality</u> <u>Volume</u>

## Infiltration Systems

Infiltration Basin (I-2)

## **Description**

An infiltration practice that stores the water quality volume in a shallow depression, before it is infiltrated it into the ground.

## <u>Alternative SMP's for Redevelopment Portion of Project</u>

#### Hydrodynamic Separator

#### **Description**

A hydrodynamic separator will be utilized to treat the impervious areas from the redeveloped portion of the site.

#### Step 6: Apply Volume and Peak Rate Control Practices to Meet Water Quantity Requirements

## • Infiltration Systems

Infiltration Basin (I-2)

#### **Description**

An infiltration practice that stores the water quality volume in a shallow depression, before it is infiltrated it into the ground.

All practices exceed the required elements of SMP criteria as outlined in Chapter 6 of the NYS Stormwater Management Design Manual. A summary of each category is provided below.

- 1. Feasibility Stormwater practices are designed based upon unique physical environmental considerations noted in the NYS Stormwater Management Design Manual (NYSSMDM).
- Conveyance The design conveys runoff to the designed stormwater practice in a manner that is safe, minimizes erosion and disruption to natural drainage channel and promotes filtering and infiltration.

- 3. Pretreatment All stormwater practices provide pretreatment as required in accordance with NYSSMDM design guidelines.
- 4. Treatment Geometry The plan provides water quality treatment in accordance with NYSSMDM guidelines.
- 5. Environmental/Landscaping –Extensive landscaping has been provided for each proposed stormwater practice to enhance pollutant removal and provide aesthetic enhancement to the property.
- 6. Maintenance Maintenance for the environment practices has been provided and is detain the SWPPP Report as required. Maintenance access is provided in the design plans.

In order to determine the post-development rates of runoff generated on-site, the following drainage areas were analyzed in the post-development conditions. These areas are graphically depicted on Drawing DA-2 "Proposed Drainage Area Map" located in Appendix G.

One Design Line (analysis boundary) was identified for comparing peak rates of runoff in existing and proposed conditions. Similarly, four separate drainage areas were identified in proposed conditions based on the proposed drainage divides at the site.

The following is a description of each of the drainage areas analyzed in the proposed conditions analysis:

<u>Proposed Drainage Area IA (PDA-IA)</u> is 2.11 acres in size and is located on the southern portion of the site where development is proposed. This area consists of the proposed warehouse building, areas of lawn, and the proposed infiltration basins. This drainage area drains towards the proposed infiltration basins. Stormwater runoff that exits the proposed infiltration basins will be directed to a series of pipes that will convey the flows to the existing outfall to the Byram River. The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 91 and 5.0 minutes, respectively.

<u>Proposed Drainage Area IB (PDA-IB)</u> is 0.60 acres in size and is located on the southern portion of the site where development is proposed. This area consists of the proposed access driveway which runs along the southern and eastern sides of the proposed warehouse. This area is comprised of areas of asphalt pavement, lawn, and porous pavement. This drainage area will be collected and treated by porous asphalt pavement, where flows will then enter the existing conveyance system that connects to the Byram River.

The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 94 and 5.0 minutes, respectively.

<u>Proposed Drainage Area IC (PDA-IC)</u> is 0.95 acres in size and is located on the western side of the proposed warehouse building. This area consists of the proposed loading area and areas of lawn in the front of the proposed building. This drainage area drains towards a proposed hydrodynamic separator which will treat the impervious areas within this drainage area. Flows will then be directed to the proposed conveyance system which leads to the existing outfall to the Byram River.

The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 80 and 5.0 minutes, respectively.

<u>Proposed Drainage Area ID (PDA-ID)</u> is 4.55 acres in size and is located on the center of the site. This area consists of the existing building, southern parking area, existing loading area, and compensatory storage areas, and lawn / wooded areas in the rear of the property. This drainage area drains towards the existing conveyance system which pipes flows to the Byram River.

The Curve Number (CN) and Time of Concentration (Tc) for this drainage area are 86 and 10.9 minutes, respectively.

Refer to Drawing DA-2 in Appendix G.

The peak rates of runoff to the design point of each of the analyzed drainage areas for each storm are shown on the table below:

# Table 3 Summary of Proposed Peak Rates of Runoff in Proposed Conditions (Cubic Feet per Second)

Storm Recurrence Interval	DL-I
l year	7.56
10 year	17.45
100 year	37.93

The reductions in peak rates of runoff from proposed to existing conditions are shown on the table below:

#### <u>Table 4</u> <u>Percent Reductions in Peak Rates of Runoff (Existing vs. Proposed Conditions)</u> (Cubic Feet per Second)

Design Line	Storm Recurrence Frequency (Years)	Existing Peak Runoff Rate (cfs)	Proposed Peak Runoff Rate (cfs)	Percent Reduction (%)
I	l year	7.71	7.56	1.9
	10 year	17.98	17.45	2.9
	100 year	38.16	37.93	0.6

As demonstrated in Table 4, the proposed stormwater improvements will result in significant reductions of peak rates of runoff for all storms and design points analyzed.

#### VI. SOIL EROSION & SEDIMENT CONTROL

A potential impact of the proposed development on any soils or slopes will be that of erosion and transport of sediment during construction. An Erosion and Sediment Control Management Program will be established for the proposed development, beginning at the start of construction and continuing throughout its course, as outlined in the "New York State Standards and Specifications for Erosion and Sediment Control," November 2016. A continuing maintenance program will be implemented for the control of sediment transport and erosion control after construction and throughout the useful life of the project.

The Operator shall have a qualified professional conduct an assessment of the site prior to the commencement of construction and certify that the appropriate erosion and sediment controls, as shown on the Sediment & Erosion Control Plans, have been adequately installed to ensure overall preparedness of the site for the commencement of construction. In addition, the Operator shall have a qualified professional conduct one site inspection at least every seven calendar days and at least two site inspections every seven calendar days when greater than five acres of soil is disturbed at any one time.

Prior to the commencement of construction activity, the owner or operator must identify the contractor(s) and subcontractor(s) that will be responsible for installing, constructing, repairing, replacing, inspecting and maintaining the erosion and sediment control practices included in the SWPPP; and the contractor(s) and subcontractor(s) that will be responsible for constructing the post-construction stormwater management practices included in the SWPPP. The owner or operator shall have each of the contractors and subcontractors identify at least one person from their company that will be responsible for implementation of the SWPPP. This person shall be known as the trained contractor. The owner or operator shall ensure that at least one trained contractor is on site on a daily basis when soil disturbance activities are being performed. The owner or operator shall have each of the contractors and subcontractors and subcontractors identified above sign a copy of the certification statement provided in Appendix F before they commence any construction activity.

#### Soil Description

As provided by the United States Department of Agriculture, Soil Conservation Service "Web Soil Survey," soil classifications which exist on the subject site are described below.

Soils are placed into four hydrologic groups: A, B, C, and D. In the definitions of the classes, infiltration rate is the rate at which water enters the soil at the surface and is controlled by the surface conditions. Transmission rate is the rate at which water moves in the soil and is controlled by soil properties. Definitions of the classes are as follows:

- A. (Low runoff potential). The soils have a high infiltration rate even when thoroughly wetted.
   They chiefly consist of deep, well drained to excessively drained sands or gravels. They have a high rate of water transmission.
- B. The soils have a moderate infiltration rate when thoroughly wetted. They chiefly are moderately deep to deep, moderately well drained to well drained soils that have moderately fine to moderately coarse textures. They have a moderate rate of water transmission.
- C. The soils have a slow infiltration rate when thoroughly wetted. They chiefly have a layer that impedes downward movement of water or have moderately fine to fine texture. They have a slow rate of water transmission.
- D. (High runoff potential). The soils have a very slow infiltration rate when thoroughly wetted. They chiefly consist of clay soils that have a high swelling potential, soils that have a permanent high water table, soils that have a claypan or clay layer at or near the surface, and shallow soils over nearly impervious material. They have a very slow rate of water transmission.

A soil's tendency to erode is also described in the USDA web soil survey. The ratings in this interpretation indicate the hazard of soil loss from unsurfaced areas. The ratings are based on soil erosion factor K, slope, and content of rock fragments. The hazard is described as "slight," "moderate," or "SEVERE." A rating of "slight" indicates that little or no erosion is likely; "moderate" indicates that some erosion is likely, that the temporarily unsurfaced / unstabilized during construction may require occasional maintenance, and that simple erosion-control measures are needed; and "SEVERE" indicates that significant erosion is expected, that the roads or trails require frequent maintenance, and that erosion-control measures are needed.

Per the Soil Survey, the following soils listed below are present at the site. Following this list is a detailed description of each soil type found on the property:

SYM.	H.S.G.	DESCRIPTION
Ff	D	Fluvaquents-Udifluvents complex, frequently flooded
Ub	В	Udorthents, smoothed
Uf	D	Urban land

#### Ff, Fluvaquents-Udifluvents Complex, Frequently Flooded

This soil is very deep, well drained to very poorly drained, nearly level soils that formed in recent alluvial deposits. The parent material consists of alluvium with highly variable texture. Depth to the top of a seasonal high water table is 1.5 to 3.0 feet below the surface from November through April. Available water capacity is moderate.

Hydrologic group: D Erosion Hazard Rating: Slight

#### Ub, Udorthents, Smoothed

This soil is very deep, excessively drained to moderately well drained soils that have been altered by cutting and filling. It is mainly in and adjacent to urban areas, highways, and borrow areas. It is made up of soil material in alternating layers ranging from sand to silt loam Depth to the top of a seasonal high water table is approximately 1.5 to 4 feet. Available water capacity is moderate.

Hydrologic group: B Erosion Hazard Rating: Not Rated

#### <u>Uf, Urban Land</u>

This soil consists of areas where at least 60 percent of the land surface is covered with buildings or other structures. The areas include parking lots, shopping centers, industrial parks, and institutional sites. Depth to the top of a seasonal high water table is greater than 6'. Available water capacity is moderate.

Hydrologic group: D Erosion Hazard Rating: Not Rated

#### **On-Site Pollution Prevention**

There are temporary pollution prevention measures used to control litter and construction debris on site, such as:

- Silt Fence
- Inlet Protection
- Stabilized Construction Access

There will be inlet protection provided for all storm drains and inlets with the use of curb gutter inlet protection structures and stone & block drop inlet protection, which keep silt, sediment and construction litter and debris out of the on-site stormwater drainage system.

#### Temporary Control Measures

Temporary control measures and facilities will include silt fences, construction ditches, stabilized construction access, temporary seeding, mulching and sediment traps with temporary riser and anti-vortex devices.

Throughout the construction of the proposed development, temporary control facilities will be implemented to control on-site erosion and sediment transfer. Construction ditches, if required, will be used to direct stormwater runoff to temporary sediment traps for settlement. The sediment traps will be constructed as part of this project will serve as temporary sediment basins to remove sediment and pollutants from the stormwater runoff produced during construction. Descriptions of the temporary sediment & erosion controls that will be used during the development of the site including silt fence, stabilized construction access, seeding, mulching and inlet protection are as follows:

- 1. <u>Silt Fence</u> is constructed using a geotextile fabric. The fence will be either 18 inches or 30 inches high. The height of the fence can be increased in the event of placing these devices on uncompacted fills or extremely loose undisturbed soils. The fences will not be placed in areas which receive concentrated flows such as ditches, swales and channels nor will the filter fabric material be placed across the entrance to pipes, culverts, spillway structures, sediment traps or basins.
- 2. <u>Stabilized Construction Access</u> consists of AASHTO No. I rock. The rock entrance will be a minimum of 50 feet in length by 24 feet in width by 8 inches in depth.
- Seeding will be used to create a vegetative surface to stabilize disturbed earth until at least 80% of the disturbed area has a perennial vegetative cover. This amount is required to adequately function as a sediment and erosion control facility. Grass lining will also be used to line temporary channels and the surrounding disturbed areas.
- 4. <u>Mulching</u> is used as an anchor for seeding and disturbed areas to reduce soil loss due to storm events. These areas will be mulched with straw at a rate of 3 tons per acre such that the mulch forms a continuous blanket. Mulch must be placed after seeding or within 48 hours after seeding is completed.
- 5. <u>Inlet Protection</u> will be provided for all stormwater basins and inlets with the use of curb & gutter inlet protection and stone & block inlet protection structures, which will keep silt, sediment and construction debris out of the storm system. Existing structures within existing paved areas will be protected using "Silt Sacks" inside the structures.

The contractor shall be responsible for maintaining the temporary sediment and erosion control measures throughout construction. This maintenance will include, but not be limited to, the following tasks:

- For dust control purposes, moisten all exposed graded areas with water at least twice a day in those areas where soil is exposed and cannot be planted with a temporary cover due to construction operations or the season (December through March).
- Inspection of erosion and sediment control measures shall be performed at the end of each construction day and immediately following each rainfall event. All required repairs shall be immediately executed by the contractor.
- 3. Sediment deposits shall be removed when they reach approximately <sup>1</sup>/<sub>3</sub> the height of the silt fence. All such sediment shall be properly disposed of in fill areas on the site, as directed by the Owner's Field Representative. Fill shall be protected following disposal with mulch, temporary and/or permanent vegetation and be completely circumscribed on the downhill side by silt fence.
- 4. Rake all exposed areas parallel to the slope during earthwork operations.
- 5. Following final grading, the disturbed area shall be stabilized with a permanent surface treatment (i.e. turf grass, pavement or sidewalk). During rough grading, areas which are not to be disturbed for fourteen or more days shall be stabilized with the temporary seed mixture, as defined on the plans. Seed all piles of dirt in exposed soil areas that will not receive a permanent surface treatment.

#### Concrete Material and Equipment Management

Concrete washouts shall be used to contain concrete and liquids when the chutes of concrete mixers and hoppers of concrete pumps are rinsed out after delivery. The washout facilities

consolidate solid for easier disposal and prevent runoff of liquids. The wash water is alkaline and contains high levels of chromium, which can leach into the ground and contaminate groundwater. It can also migrate to a storm drain, which can increase the pH of area waters and harm aquatic life. Solids that are improperly disposed of can clog storm drain pipes and cause flooding. Installing concrete washout facilities not only prevents pollution but also is a matter of good housekeeping at your construction site.

Prefabricated concrete washout containers can be delivered to the site to provide maintenance and disposal of materials. Regular pick-ups of solid and liquid waste materials will be necessary. To prevent leaks on the job site, ensure that prefabricated washout containers are watertight. A self installed concrete washout facility can be utilized although they are much less reliable than prefabricated containers and are prone to leaks. There are many design options for the washout, but they are preferably built below-grade to prevent breaches and reduce the likelihood of runoff. Above-grade structures can also be used if they are sized and constructed correctly and are diligently maintained. One of the most common problems with self-installed concrete washout facilities is that they can leak or be breached as a result of constant use, therefore the contractor shall be sure to use quality materials and inspect the facilities on a daily basis.

Washouts must be sized to handle solids, wash water, and rainfall to prevent overflow. Concrete Washout Systems, Inc. estimates that 7 gallons of wash water are used to wash one truck chute and 50 gallons are used to wash out the hopper of a concrete pump truck.

For larger sites, a below-grade washout should be at least 10 feet wide and sized to contain all liquid and solid waste expected to be generated in between cleanout periods. A minimum of 12-inches of freeboard must be provided. The pit must be lined with plastic sheeting of at least 10-mil thickness without holes or tears to prevent leaching of liquids into the ground. Concrete wash water should never be placed in a pit that is connected to the storm drain system or that drains to nearby waterways.

An above-grade washout can be constructed at least 10 feet wide by 10 feet long and sized to contain all liquid and solid waste expected to be generated in between cleanout periods. A minimum of 4-inches of freeboard must be provided. The washout structures can be constructed with staked straw bales or sandbags double-or triple lined with plastic sheeting of at least 10-mil thickness without holes or tears.

Concrete washout facilities shall not be located within 50 feet of storm drains, open ditches, or water bodies and should be placed in locations that allow for convenient access for concrete trucks. The contractor shall check all concrete washout facilities daily to determine if they have been filled to 75 percent capacity, which is when materials need to be removed. Both above-and below-ground self-installed washouts should be inspected daily to ensure that plastic linings are intact and sidewalls have not been damaged by construction activities. Prefabricated washout containers should be inspected daily as well as to ensure the container is not leaking or nearing 75 percent capacity. Inspectors should also note whether the facilities are being used regularly. Additional signage for washouts may be needed in more convenient locations if concrete truck operators are not utilizing them.

The washout structures must be drained or covered prior to predicted rainstorms to prevent overflows. Hardened solids either whole or broken must be removed and then they may be reused onsite or hauled away for recycling.

Once materials are removed from the concrete washout, a new structure must be built or excavated, or if the previous structure is still intact, inspect it for signs of weakening or damage and make any necessary repairs. Line the structure with new plastic that is free of holes or tears and replace signage if necessary. It is very important that new plastic be used after every cleaning because pumps and concrete removal equipment can damage the existing liner.

#### Construction Site Chemical Control

The purpose of this management measure is to prevent the generation of nonpoint source pollution from construction sites due to improper handling and usage of nutrients and toxic substances, and to prevent the movement of toxic substances from the construction site.

Many potential pollutants other than sediment are associated with construction activities. These pollutants include pesticides; fertilizers used for vegetative stabilization; petrochemicals; construction chemicals such as concrete products, sealers, and paints; wash water associated with these products; paper; wood; garbage; and sanitary waste.

Disposal of excess pesticides and pesticide-related wastes should conform to registered label directions for the disposal and storage of pesticides and pesticide containers set forth in applicable Federal, State and local regulations that govern their usage, handling, storage, and disposal.

Pesticides should be disposed of through either a licensed waste management firm or a treatment, storage and disposal (TSD) facility. Containers should be triple-rinsed before disposal, and rinse waters should be reused as product.

Other practices include setting aside a locked storage area, tightly closing lids, storing in a cool, dry place, checking containers periodically for leaks or deterioration, maintaining a list of products in storage, using plastic sheeting to line the storage areas, and notifying neighboring property owners prior to spraying.

When storing petroleum products, follow these guidelines:

- Create a shelter around the area with cover and wind protection;
- Line the storage area with a double layer of plastic sheeting or similar material;
- Create an impervious berm around the perimeter with a capacity of 110 percent greater than that of the largest container;
- Clearly label all products;

- Keep tanks off the ground; and
- Keep lids securely fastened.

Post spill procedure information and have persons trained in spill handling on site or on call at all times. Materials for cleaning up spills should be kept on site and easily available. Spills should be cleaned up immediately and the contaminated material properly disposed of. Maintain and wash equipment and machinery in confined areas specifically designed to control runoff.

Thinners or solvents should not be discharged into sanitary or storm systems when cleaning machinery. Use alternative methods for cleaning larger equipment parts, such as high-pressure, high-temperature water washes, or steam cleaning. Equipment-washing detergents can be used, and wash water may be discharged into sanitary sewers if solids are removed from the solution first. (This practice should be verified with the local sewer authority.) Small parts can be cleaned with degreasing solvents, which can then be reused or recycled.

#### Solid Waste Management and Portable Sanitary Management

The purpose of this management measure is to prevent the potential for solid waste such as construction debris, trash, etc. from construction sites due to improper handling and storage. Debris and litter should be removed periodically from the BMP's and surrounding areas to prevent clogging of pipes and structures. All construction material shall be stored in designated staging areas. Roll-off containers shall be placed on site and all empty containers, construction debris and litter shall be placed in the containers.

Portable sanitary units may be utilized on-site or bathrooms will be provided within construction trailers. A sanitation removal company will be hired to pump/remove any sanitary waste. In the event that portable sanitary units are used and then cleaned after being emptied, the rinse water may not be disposed of to the storm drain system. It shall be contained for later disposal if it can't be disposed of on-site. Remove paper and trash before cleaning the portable sanitary units. The portable sanitary units shall be located away from the storm drain system if possible. Provide over head cover for wash areas if possible. Maintain spill response material and equipment on site

to eliminate the potential for contaminants and wash water from entering the storm drain system.

#### Permanent Control Measures and Facilities for Long Term Protection

Towards the completion of construction, permanent sediment and erosion control measures will be developed for long term erosion protection. The following permanent control measures and facilities have been proposed to be implemented for the project:

- Infiltration Basins will be used to treat the runoff volume generated from the developed area and provide improvement to water quality control. The proposed basins will provide water quality for 90% of the average annual stormwater runoff volume. The water quality volume will be retained and higher storms will be released gradually. Refer to the water quality volume calculations, in Appendix B.
- 2. <u>A Hydrodynamic Separator</u> will be used to provide treatment of the water quality flow rate from the redeveloped areas to separate sediment, debris, floatables, etc. from the runoff prior to discharge.
- <u>Catch Basins</u> will be used to remove some of the coarse sand and grit sediment before entering the drainage system. Each catch basin will be constructed with an 18 inch deep sump.
- 4. <u>Seeding</u> of at least 70% perennial vegetative cover will be used to produce a permanent uniform erosion resistant surface. The seeded areas will be mulched with straw at a rate of 2 tons per acre such that the mulch forms a continuous blanket.

#### **Specifications for Soil Restoration**

Prior to the final stabilization of the disturbed areas, soil restoration will be required for all vegetated areas to recover the original properties and porosity of the soil. Soil Restoration Requirements are provided on Table 5 below:

#### Table 5

## **Soil Restoration Requirements**

Type of Soil Disturbance	Soil Restoration Requirement		Comments/Examples
No soil disturbance	Restoration not permitted		Preservation of Natural Features
Minimal soil disturbance	Restoration no	t required	Clearing and grubbing
Areas where topsoil is	HSG A&B	HSG C&D	Protect area from any
stripped only – no change in	apply 6 inches	Aerate* and	ongoing construction
grade	of topsoil	apply 6 inches of topsoil	
Areas of cut or fill	HSG A&B	HSG C&D	Clearing and grubbing
	Aerate and	Apply full Soil	
	apply 6 inches of topsoil	Restoration**	
Heavy traffic areas on site	Apply full Soil Restoration		
(especially) in a zone 5-25 feet	(decompaction and compost		
around buildings but not	enhancement)		
within a 5 foot perimeter			
around foundation walls)			
Areas where Runoff Reduction	Restoration not required, but		Keep construction
and/or Infiltration practices may be applied to enhance the		to enhance the	equipment from crossing
are applied	appropriate practices.		these areas. To protect
			newly installed practice from
			any ongoing construction
			phase operation fence area
Redevelopment projects	evelopment projects Soil Restoration is required on		
	redevelopment projects in areas		
	where existing impervious area		
	will be converted to pervious		
	area.		

\* Aeration includes the use of machines such as tractor-drawn implements with coulters making a narrow slit in the soil, a roller with many spikes making indentations in the soil, or prongs which function like a mini-subsoiler.

\*\* Per "Deep Ripping and De-compaction, DEC 2008."

During periods of relatively low to moderate subsoil moisture, the disturbed subsoils are returned to rough grade and the following full soil restoration steps applied:

- I. Apply 3 inches of compost over subsoil.
- 2. Till compost into subsoil to a depth of at least 12 inches using a cat-mounted ripper, tractor-mounted disc, or tiller, mixing, and circulating air and compost into subsoils.
- 3. Rock-pick until uplifted stone/rock materials of four inches and larger size are cleaned off the site.

#### **Specifications for Final Stabilization of Graded Areas**

Final stabilization of graded areas consists of the placement of topsoil and installation of landscaping (unless the area is to be paved, or a building is to be constructed in the location). Topsoil is to be spread as soon as grading operations are completed. Topsoil is to be placed to a minimum depth of six inches on all embankments, planting areas and seeding/sod areas. The subgrade is to be scarified to a depth of two inches to provide a bond of the topsoil with the subsoil. Topsoil is to be raked to an even surface and cleared of all debris, roots, stones and other unsatisfactory material.

Planting operations shall be conducted under favorable weather conditions as follows:

 Permanent Lawns - April 15 (provided soil is frost-free and not excessively moist) to May 15; August 15 to October 15.

• Temporary Lawn Seeding - if outside of the time periods noted above, the areas shall be seeded immediately on completion of topsoil operations with annual ryegrass (Italian rye) at a rate of six pounds per 1,000 square feet. Temporary lawn installation is permitted provided the soil is frost-free and not excessively moist. The permanent lawn is to be installed the next planting season.

On slopes with a grade of 3 horizontal to 1 vertical or greater, and in swales, a geotextile netting or mat shall be installed for stabilization purposes as shown on the Plans. Seeded areas are to be mulched with straw or hay at an application rate of 70-90 pounds per 1,000 s.f. Straw or hay mulch must be spread uniformly and anchored immediately after spreading to prevent wind blowing. Mulches must be inspected periodically and in particular after rainstorms to check for erosion. If erosion is observed, additional mulch must be applied. Netting shall be inspected after rainstorms for dislocation or failure; any damage shall be repaired immediately.

All denuded surfaces which will be exposed for a period of over two months or more shall be temporarily hydroseeded with (a) perennial ryegrass at a rate of 40 lbs per acre (1.0 lb per 1000 square feet ); (b) Certified "Aroostook" winter rye (cereal rye) @ 100 lb per acre (2.5 lb/1000 s.f.) to be used in the months of October and November.

Permanent turfgrass cover is to consist of a seed mixture as follows:

#### (a) <u>Sunny sites</u>

Kentucky Bluegrass	2.0-2.6 pounds/1000 square feet
Perennial Ryegrass	0.6-0.7 pounds/1000 square feet
Fine Fescue	0.4-0.6 pounds/1000 square feet

(b) <u>Shady sites</u>

Kentucky Bluegrass	0.8-1.0 pounds/1000 square feet
Perennial Ryegrass	0.6-0.7 pounds/1000 square feet
Fine Fescue	2.6-3.3 pounds/1000 square feet

All plant materials shall comply with the standards of the American Association Of Nurserymen with respect to height and caliper as described in its publication American Standard for Nursery Stock, latest edition.
# VII. CONSTRUCTION PHASE AND POST-CONSTRUCTION MAINTENANCE

During the construction phase and following construction of the project, a number of maintenance measures will be taken with respect to the site maintenance. Measures to be taken included the following:

# I. During Construction

A comprehensive sediment and erosion control plan will be in place during the construction period. Maintenance measures for sediment and erosion controls will include:

A qualified professional acceptable to the municipality will be hired by the owner or operator to monitor the installation and maintenance of the sediment and erosion control plans. The qualified professional shall report directly to the Engineering Consultant and shall be responsible for ensuring compliance with the design of the sediment and erosion control plans.

The qualified professional so hired will inspect all sediment and erosion control measures at least every seven calendar days. In the event that there has been a variance with the design of the sediment and erosion control measures so that the ability of the measures to adequately perform the intended function is lessened or compromised and/or the facilities are not adequately maintained, the qualified professional shall be required to report such variance to the Engineering Consultant within 48 hours and shall be empowered to order immediate repairs to the sediment and erosion control measures.

The qualified professional will also be responsible for observing the adequacy of the vegetation growth (trees, shrubs, groundcovers and turfgrasses) in newly graded areas and for ordering additional plantings in the event that the established plant materials do not adequately protect the ground surface from erosion.

# 2. Following Construction

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Site maintenance activities on the property will include:

- Grounds maintenance, including mowing of lawns;
- Planting of trees, shrubs and groundcovers; pruning of trees and shrubs;
- Application of fertilizer and herbicides;
- Maintenance of stormwater management area;

Grounds maintenance on the site will be performed by landscaping contractor.

Fertilizer is typically applied twice in the year - once in the spring and once in the fall. The application of fertilizer is usually necessary to maintain healthy lawn growth due to competition for nutrients with trees and shrubs and since the clippings are often removed. It is not recommended that fertilizer be applied during the summer. It is at this time that lawns are typically dormant.

Fertilizers come in three basic types: (1) Organic; (2) Soluble synthetic and (3) Slow release.

Organic fertilizers are derived from plant or animal waste. Since they are heavier and bulkier than other fertilizers, it is necessary to apply a much greater amount at one time. Soluble synthetic fertilizers are predictable with determining the exact impact on a lawn. However more applications are necessary since their effect is often short term. Slow release fertilizers have a high percentage of nitrogen so quantities that need be handled at one time are smaller. Slow release fertilizers will be utilized by the project.

A complete fertilizer contains all three of the primary nutrients - nitrogen (N), phosphorus (P) and potassium in the form of potash (K). Typically, a 3-1-2 ratio of nutrients (N-P-K) is used for lawn applications.

Fertilizer shall be applied by the landscape contractor in accordance with the manufacturer's instructions. The application of fertilizer does require some skill on the part of the operator.

Should there be a spill of fertilizer, the landscape contractor shall be required to scrape or vacuum it up. The area will then be watered in accordance with the manufacturer's instructions to ensure that the fertilizer becomes soluble and available to plants and does not run off.

A&R Real Estate Holdings, LLC will be responsible for the long-term operation and maintenance of the permanent stormwater management practices. The permanent stormwater management practices shall be maintained in accordance with the Maintenance Inspection Checklists provided in Appendix E.

## VIII. <u>CONCLUSION</u>

This Stormwater Pollution Prevention Plan has been prepared to describe the project's pre and post-development stormwater management improvements and its sediment and erosion control improvements to be utilized during construction. The proposed permanent improvements and the interim improvements to be utilized during construction have been designed in accordance with the requirements of the:

- New York State Department of Environmental Conservation (NYSDEC) SPDES General Permit No. GP-0-20-001, effective January 29, 2020.
- Chapter 267 "Stormwater Management" of the Town of North Castle Code

The project employs a variety of practices to enhance stormwater quality and reduce peak rates of runoff associated with the proposed improvements. These measures include an infiltration basin, extensive areas of porous pavement, and a hydrodynamic separator. These improvements will also mitigate runoff volumes from the proposed improvements as runoff volumes will be slightly reduced or maintained in all the analyzed storms.

Based on the foregoing, it is our professional opinion that the proposed improvements will provide water quantity and quality enhancements which exceed the above mentioned requirements and are not anticipated to have any adverse impacts to the site or any surrounding areas.

# **APPENDIX A**

# HYDROLOGIC CALCULATIONS



EDA-PDA.ppc 12/30/2020 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 1 of 1



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# **Catchments Summary**

Label	Label Scenario		Hydrograph Volume (ft³)	Time to Peak (hours)	Peak Flow (ft³/s)
EDA-1A	Existing Conditions 1 Year Storm	1	29,640.000	12.150	7.71
EDA-1A	Existing Conditions 10 Year Storm	10	67,958.000	12.150	17.11
EDA-1A	Existing Conditions 100 Year Storm	100	138,255.000	12.150	33.35
PDA-1D	Proposed Conditions 1 Year Storm	1	24,860.000	12.150	6.37
PDA-1D	Proposed Conditions 10 Year Storm	10	59,299.000	12.150	14.88
PDA-1D	Proposed Conditions 100 Year Storm	100	123,455.000	12.150	29.81
PDA-1A	Proposed Conditions 1 Year Storm	1	14,470.000	12.100	4.10
PDA-1A	Proposed Conditions 10 Year Storm	10	32,534.000	12.100	8.85
PDA-1A	Proposed Conditions 100 Year Storm	100	65,439.000	12.100	17.02
PDA-1B	Proposed Conditions 1 Year Storm	1	4,090.000	12.100	1.13
PDA-1B	Proposed Conditions 10 Year Storm	10	8,680.000	12.100	2.30
PDA-1B	Proposed Conditions 100 Year Storm	100	16,887.000	12.100	4.28
EDA-1B	Existing Conditions 1 Year Storm	1	1,983.000	12.750	0.12
EDA-1B	Existing Conditions 10 Year Storm	10	13,069.000	12.450	1.96
EDA-1B	Existing Conditions 100 Year Storm	100	44,148.000	12.400	7.62
PDA-1C	Proposed Conditions 1 Year Storm	1	5,002.000	12.100	1.42
PDA-1C	Proposed Conditions 10 Year Storm	10	11,248.000	12.100	3.06
PDA-1C	Proposed Conditions 100 Year Storm	100	22,623.000	12.100	5.88

# **Node Summary**

Label	Scenario	Return Event (years)	Hydrograph Volume (ft³)	Time to Peak (hours)	Peak Flow (ft <sup>3</sup> /s)	
DP	Proposed Conditions 1 Year Storm	1	29,862.000	12.150	7.56	
DP	Existing Conditions 1 Year Storm	1	31,623.000	12.150	7.71	
DP	Existing Conditions 10 Year Storm	10	81,027.000	12.150	17.98	
EDA-PDA.ppc 1/22/2021	Bentley 27 Water	PondPack CONNECT [10.02 Page 1	Edition 00.01] of 448			

Subsection: Master Network Summary

## **Node Summary**

Label	Scenario	Return Event (years)	Hydrograph Volume (ft³)	Time to Peak (hours)	Peak Flow (ft³/s)
DP	Proposed Conditions 10 Year Storm	10	70,547.000	12.100	17.45
DP	Existing Conditions 100 Year Storm	100	182,403.000	12.150	38.16
DP	Proposed Conditions 100 Year Storm	100	161,309.000	12.150	37.93

# **Pond Summary**

Label	Scenario	Return Event (years)	Hydrograph Volume (ft <sup>3</sup> )	Time to Peak (hours)	Peak Flow (ft <sup>3</sup> /s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft <sup>3</sup> )
INFILTRATI ON BASIN A (IN)	Proposed Conditions 1 Year Storm	1	14,470.000	12.100	4.10	(N/A)	(N/A)
INFILTRATI ON BASIN A (OUT)	Proposed Conditions 1 Year Storm	1	5,294.000	12.150	2.67	367.98	2,611.000
INFILTRATI ON BASIN A (IN)	Proposed Conditions 10 Year Storm	10	32,534.000	12.100	8.85	(N/A)	(N/A)
INFILTRATI ON BASIN A (OUT)	Proposed Conditions 10 Year Storm	10	13,251.000	12.100	4.27	369.41	7,460.000
INFILTRATI ON BASIN A (IN)	Proposed Conditions 100 Year Storm	100	65,439.000	12.100	17.02	(N/A)	(N/A)
INFILTRATI ON BASIN A	Proposed Conditions 100 Year Storm	100	36,630.000	12.050	7.78	370.73	12,791.000
Porous Pavement (IN)	Proposed Conditions 1 Year Storm	1	4,090.000	12.100	1.13	(N/A)	(N/A)
Porous Pavement (OUT)	Proposed Conditions 1 Year Storm	1	0.000	0.000	0.00	366.91	1,221.000
Porous Pavement (IN)	Proposed Conditions 10 Year Storm	10	8,680.000	12.100	2.30	(N/A)	(N/A)
Porous Pavement (OUT)	Proposed Conditions 10 Year Storm	10	0.000	0.000	0.00	367.13	2,538.000
Porous Pavement (IN)	Proposed Conditions 100 Year Storm	100	16,887.000	12.100	4.28	(N/A)	(N/A)

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# **Pond Summary**

Label	Scenario	Return Event (years)	Hydrograph Volume (ft <sup>3</sup> )	Time to Peak (hours)	Peak Flow (ft³/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft³)
Porous Pavement (OUT)	Proposed Conditions 100 Year Storm	100	0.000	0.000	0.00	367.61	5,447.000
INFILTRATI ON BASIN B (IN)	Proposed Conditions 1 Year Storm	1	5,294.000	12.150	2.67	(N/A)	(N/A)
INFILTRATI ON BASIN B (OUT)	Proposed Conditions 1 Year Storm	1	0.000	0.000	0.00	367.98	2,498.000
INFILTRATI ON BASIN B (IN)	Proposed Conditions 10 Year Storm	10	13,251.000	12.100	4.27	(N/A)	(N/A)
INFILTRATI ON BASIN B (OUT)	Proposed Conditions 10 Year Storm	10	0.000	0.000	0.00	369.40	6,413.000
INFILTRATI ON BASIN B (IN)	Proposed Conditions 100 Year Storm	100	36,630.000	12.050	7.78	(N/A)	(N/A)
INFILTRATI ON BASIN B (OUT)	Proposed Conditions 100 Year Storm	100	15,230.000	12.350	5.70	370.57	10,259.000

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Time-Depth Curve: 1	
Label	1
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	1 years

#### CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.0	0.0	0.0	0.0	0.0
1.500	0.0	0.0	0.0	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.000	0.1	0.1	0.1	0.1	0.1
3.500	0.1	0.1	0.1	0.1	0.1
4.000	0.1	0.1	0.1	0.1	0.1
4.500	0.1	0.1	0.1	0.2	0.2
5.000	0.2	0.2	0.2	0.2	0.2
5.500	0.2	0.2	0.2	0.2	0.2
6.000	0.2	0.2	0.2	0.2	0.2
6.500	0.2	0.2	0.2	0.2	0.2
7.000	0.3	0.3	0.3	0.3	0.3
7.500	0.3	0.3	0.3	0.3	0.3
8.000	0.3	0.3	0.3	0.3	0.4
8.500	0.4	0.4	0.4	0.4	0.4
9.000	0.4	0.4	0.4	0.4	0.5
9.500	0.5	0.5	0.5	0.5	0.5
10.000	0.5	0.5	0.6	0.6	0.6
10.500	0.6	0.6	0.6	0.7	0.7
11.000	0.7	0.7	0.7	0.8	0.8
11.500	0.8	0.9	1.0	1.0	1.2
12.000	1.4	1.6	1.8	1.9	1.9
12.500	2.0	2.0	2.0	2.1	2.1
13.000	2.1	2.1	2.1	2.2	2.2
13.500	2.2	2.2	2.2	2.3	2.3
14.000	2.3	2.3	2.3	2.3	2.3
14.500	2.3	2.4	2.4	2.4	2.4
15.000	2.4	2.4	2.4	2.4	2.4
15.500	2.4	2.5	2.5	2.5	2.5
16.000	2.5	2.5	2.5	2.5	2.5
16.500	2.5	2.5	2.5	2.5	2.5
17.000	2.6	2.6	2.6	2.6	2.6

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center

27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 4 of 448 Subsection: Time-Depth Curve Label: 1-10-100-90% Scenario: Existing Conditions 1 Year Storm Return Event: 1 years Storm Event: 1

## CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
17.500	2.6	2.6	2.6	2.6	2.6
18.000	2.6	2.6	2.6	2.6	2.6
18.500	2.6	2.6	2.6	2.6	2.6
19.000	2.7	2.7	2.7	2.7	2.7
19.500	2.7	2.7	2.7	2.7	2.7
20.000	2.7	2.7	2.7	2.7	2.7
20.500	2.7	2.7	2.7	2.7	2.7
21.000	2.7	2.7	2.7	2.7	2.7
21.500	2.7	2.7	2.7	2.7	2.8
22.000	2.8	2.8	2.8	2.8	2.8
22.500	2.8	2.8	2.8	2.8	2.8
23.000	2.8	2.8	2.8	2.8	2.8
23.500	2.8	2.8	2.8	2.8	2.8
24.000	2.8	(N/A)	(N/A)	(N/A)	(N/A)

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Time-Depth Curve: 1	
Label	1
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	1 years

## CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.0	0.0	0.0	0.0	0.0
1.500	0.0	0.0	0.0	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.000	0.1	0.1	0.1	0.1	0.1
3.500	0.1	0.1	0.1	0.1	0.1
4.000	0.1	0.1	0.1	0.1	0.1
4.500	0.1	0.1	0.1	0.2	0.2
5.000	0.2	0.2	0.2	0.2	0.2
5.500	0.2	0.2	0.2	0.2	0.2
6.000	0.2	0.2	0.2	0.2	0.2
6.500	0.2	0.2	0.2	0.2	0.2
7.000	0.3	0.3	0.3	0.3	0.3
7.500	0.3	0.3	0.3	0.3	0.3
8.000	0.3	0.3	0.3	0.3	0.4
8.500	0.4	0.4	0.4	0.4	0.4
9.000	0.4	0.4	0.4	0.4	0.5
9.500	0.5	0.5	0.5	0.5	0.5
10.000	0.5	0.5	0.6	0.6	0.6
10.500	0.6	0.6	0.6	0.7	0.7
11.000	0.7	0.7	0.7	0.8	0.8
11.500	0.8	0.9	1.0	1.0	1.2
12.000	1.4	1.6	1.8	1.9	1.9
12.500	2.0	2.0	2.0	2.1	2.1
13.000	2.1	2.1	2.1	2.2	2.2
13.500	2.2	2.2	2.2	2.3	2.3
14.000	2.3	2.3	2.3	2.3	2.3
14.500	2.3	2.4	2.4	2.4	2.4
15.000	2.4	2.4	2.4	2.4	2.4
15.500	2.4	2.5	2.5	2.5	2.5
16.000	2.5	2.5	2.5	2.5	2.5
16.500	2.5	2.5	2.5	2.5	2.5
17.000	2.6	2.6	2.6	2.6	2.6

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27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 6 of 448 Subsection: Time-Depth Curve Label: 1-10-100-90% Scenario: Proposed Conditions 1 Year Storm Return Event: 1 years Storm Event: 1

## CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
17.500	2.6	2.6	2.6	2.6	2.6
18.000	2.6	2.6	2.6	2.6	2.6
18.500	2.6	2.6	2.6	2.6	2.6
19.000	2.7	2.7	2.7	2.7	2.7
19.500	2.7	2.7	2.7	2.7	2.7
20.000	2.7	2.7	2.7	2.7	2.7
20.500	2.7	2.7	2.7	2.7	2.7
21.000	2.7	2.7	2.7	2.7	2.7
21.500	2.7	2.7	2.7	2.7	2.8
22.000	2.8	2.8	2.8	2.8	2.8
22.500	2.8	2.8	2.8	2.8	2.8
23.000	2.8	2.8	2.8	2.8	2.8
23.500	2.8	2.8	2.8	2.8	2.8
24.000	2.8	(N/A)	(N/A)	(N/A)	(N/A)

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Time-Depth Curve: 10	
Label	10
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	10 years

## CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.2
3.000	0.2	0.2	0.2	0.2	0.2
3.500	0.2	0.2	0.2	0.2	0.2
4.000	0.2	0.2	0.2	0.2	0.2
4.500	0.3	0.3	0.3	0.3	0.3
5.000	0.3	0.3	0.3	0.3	0.3
5.500	0.3	0.3	0.3	0.4	0.4
6.000	0.4	0.4	0.4	0.4	0.4
6.500	0.4	0.4	0.4	0.4	0.5
7.000	0.5	0.5	0.5	0.5	0.5
7.500	0.5	0.5	0.5	0.6	0.6
8.000	0.6	0.6	0.6	0.6	0.6
8.500	0.7	0.7	0.7	0.7	0.7
9.000	0.7	0.8	0.8	0.8	0.8
9.500	0.8	0.9	0.9	0.9	0.9
10.000	1.0	1.0	1.0	1.0	1.1
10.500	1.1	1.1	1.2	1.2	1.2
11.000	1.3	1.3	1.4	1.4	1.5
11.500	1.5	1.6	1.7	1.9	2.1
12.000	2.6	3.0	3.2	3.4	3.5
12.500	3.6	3.7	3.7	3.8	3.8
13.000	3.8	3.9	3.9	3.9	4.0
13.500	4.0	4.0	4.1	4.1	4.1
14.000	4.2	4.2	4.2	4.2	4.2
14.500	4.3	4.3	4.3	4.3	4.4
15.000	4.4	4.4	4.4	4.4	4.4
15.500	4.5	4.5	4.5	4.5	4.5
16.000	4.5	4.5	4.6	4.6	4.6
16.500	4.6	4.6	4.6	4.6	4.6
17.000	4.7	4.7	4.7	4.7	4.7

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27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 8 of 448 Subsection: Time-Depth Curve Label: 1-10-100-90% Scenario: Existing Conditions 10 Year Storm

#### CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
17.500	4.7	4.7	4.7	4.7	4.7
18.000	4.8	4.8	4.8	4.8	4.8
18.500	4.8	4.8	4.8	4.8	4.8
19.000	4.8	4.8	4.8	4.9	4.9
19.500	4.9	4.9	4.9	4.9	4.9
20.000	4.9	4.9	4.9	4.9	4.9
20.500	4.9	4.9	4.9	5.0	5.0
21.000	5.0	5.0	5.0	5.0	5.0
21.500	5.0	5.0	5.0	5.0	5.0
22.000	5.0	5.0	5.0	5.0	5.0
22.500	5.0	5.1	5.1	5.1	5.1
23.000	5.1	5.1	5.1	5.1	5.1
23.500	5.1	5.1	5.1	5.1	5.1
24.000	5.1	(N/A)	(N/A)	(N/A)	(N/A)

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 9 of 448 Subsection: Time-Depth Curve Label: 1-10-100-90% Scenario: Proposed Conditions 10 Year Storm

Time-Depth Curve: 10	
Label	10
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	10 years

#### CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(nouis)	(11)	(11)	(11)	(11)	(11)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.0	0.0	0.0	0.0
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.1	0.1	0.1
2.000	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.2
3.000	0.2	0.2	0.2	0.2	0.2
3.500	0.2	0.2	0.2	0.2	0.2
4.000	0.2	0.2	0.2	0.2	0.2
4.500	0.3	0.3	0.3	0.3	0.3
5.000	0.3	0.3	0.3	0.3	0.3
5.500	0.3	0.3	0.3	0.4	0.4
6.000	0.4	0.4	0.4	0.4	0.4
6.500	0.4	0.4	0.4	0.4	0.5
7.000	0.5	0.5	0.5	0.5	0.5
7.500	0.5	0.5	0.5	0.6	0.6
8.000	0.6	0.6	0.6	0.6	0.6
8.500	0.7	0.7	0.7	0.7	0.7
9.000	0.7	0.8	0.8	0.8	0.8
9.500	0.8	0.9	0.9	0.9	0.9
10.000	1.0	1.0	1.0	1.0	1.1
10.500	1.1	1.1	1.2	1.2	1.2
11.000	1.3	1.3	1.4	1.4	1.5
11.500	1.5	1.6	1.7	1.9	2.1
12.000	2.6	3.0	3.2	3.4	3.5
12.500	3.6	3.7	3.7	3.8	3.8
13.000	3.8	3.9	3.9	3.9	4.0
13.500	4.0	4.0	4.1	4.1	4.1
14.000	4.2	4.2	4.2	4.2	4.2
14.500	4.3	4.3	4.3	4.3	4.4
15.000	4.4	4.4	4.4	4.4	4.4
15.500	4.5	4.5	4.5	4.5	4.5
16.000	4.5	4.5	4.6	4.6	4.6
16.500	4.6	4.6	4.6	4.6	4.6
17.000	4.7	4.7	4.7	4.7	4.7

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27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 10 of 448 Subsection: Time-Depth Curve Label: 1-10-100-90% Scenario: Proposed Conditions 10 Year Storm

## CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
17.500	4.7	4.7	4.7	4.7	4.7
18.000	4.8	4.8	4.8	4.8	4.8
18.500	4.8	4.8	4.8	4.8	4.8
19.000	4.8	4.8	4.8	4.9	4.9
19.500	4.9	4.9	4.9	4.9	4.9
20.000	4.9	4.9	4.9	4.9	4.9
20.500	4.9	4.9	4.9	5.0	5.0
21.000	5.0	5.0	5.0	5.0	5.0
21.500	5.0	5.0	5.0	5.0	5.0
22.000	5.0	5.0	5.0	5.0	5.0
22.500	5.0	5.1	5.1	5.1	5.1
23.000	5.1	5.1	5.1	5.1	5.1
23.500	5.1	5.1	5.1	5.1	5.1
24.000	5.1	(N/A)	(N/A)	(N/A)	(N/A)

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 11 of 448 Subsection: Time-Depth Curve Label: 1-10-100-90% Scenario: Existing Conditions 100 Year Storm

Time-Depth Curve: 100	
Label	100
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	100 years

## CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.1	0.1	0.1	0.1
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.2	0.2	0.2
2.000	0.2	0.2	0.2	0.2	0.2
2.500	0.2	0.2	0.3	0.3	0.3
3.000	0.3	0.3	0.3	0.3	0.3
3.500	0.3	0.3	0.4	0.4	0.4
4.000	0.4	0.4	0.4	0.4	0.4
4.500	0.5	0.5	0.5	0.5	0.5
5.000	0.5	0.5	0.5	0.6	0.6
5.500	0.6	0.6	0.6	0.6	0.6
6.000	0.7	0.7	0.7	0.7	0.7
6.500	0.7	0.8	0.8	0.8	0.8
7.000	0.8	0.8	0.9	0.9	0.9
7.500	0.9	1.0	1.0	1.0	1.0
8.000	1.0	1.1	1.1	1.1	1.1
8.500	1.2	1.2	1.2	1.3	1.3
9.000	1.3	1.4	1.4	1.4	1.5
9.500	1.5	1.6	1.6	1.6	1.7
10.000	1.7	1.8	1.8	1.9	1.9
10.500	2.0	2.0	2.1	2.2	2.2
11.000	2.3	2.4	2.4	2.5	2.6
11.500	2.7	2.9	3.1	3.4	3.8
12.000	4.6	5.3	5.7	6.0	6.3
12.500	6.4	6.5	6.6	6.7	6.8
13.000	6.9	6.9	7.0	7.1	7.1
13.500	7.2	7.2	7.3	7.3	7.4
14.000	7.4	7.5	7.5	7.6	7.6
14.500	7.6	7.7	7.7	7.7	7.8
15.000	7.8	7.9	7.9	7.9	7.9
15.500	8.0	8.0	8.0	8.1	8.1
16.000	8.1	8.1	8.2	8.2	8.2
16.500	8.2	8.2	8.3	8.3	8.3
17.000	8.3	8.3	8.4	8.4	8.4

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## CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
17.500	8.4	8.4	8.4	8.5	8.5
18.000	8.5	8.5	8.5	8.5	8.5
18.500	8.6	8.6	8.6	8.6	8.6
19.000	8.6	8.6	8.7	8.7	8.7
19.500	8.7	8.7	8.7	8.7	8.7
20.000	8.8	8.8	8.8	8.8	8.8
20.500	8.8	8.8	8.8	8.8	8.9
21.000	8.9	8.9	8.9	8.9	8.9
21.500	8.9	8.9	8.9	9.0	9.0
22.000	9.0	9.0	9.0	9.0	9.0
22.500	9.0	9.0	9.0	9.0	9.1
23.000	9.1	9.1	9.1	9.1	9.1
23.500	9.1	9.1	9.1	9.1	9.1
24.000	9.2	(N/A)	(N/A)	(N/A)	(N/A)

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Time-Depth Curve: 100	
Label	100
Start Time	0.000 hours
Increment	0.100 hours
End Time	24.000 hours
Return Event	100 years

## CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time	Depth	Depth	Depth	Depth	Depth
(hours)	(in)	(in)	(in)	(in)	(in)
0.000	0.0	0.0	0.0	0.0	0.0
0.500	0.0	0.1	0.1	0.1	0.1
1.000	0.1	0.1	0.1	0.1	0.1
1.500	0.1	0.1	0.2	0.2	0.2
2.000	0.2	0.2	0.2	0.2	0.2
2.500	0.2	0.2	0.3	0.3	0.3
3.000	0.3	0.3	0.3	0.3	0.3
3.500	0.3	0.3	0.4	0.4	0.4
4.000	0.4	0.4	0.4	0.4	0.4
4.500	0.5	0.5	0.5	0.5	0.5
5.000	0.5	0.5	0.5	0.6	0.6
5.500	0.6	0.6	0.6	0.6	0.6
6.000	0.7	0.7	0.7	0.7	0.7
6.500	0.7	0.8	0.8	0.8	0.8
7.000	0.8	0.8	0.9	0.9	0.9
7.500	0.9	1.0	1.0	1.0	1.0
8.000	1.0	1.1	1.1	1.1	1.1
8.500	1.2	1.2	1.2	1.3	1.3
9.000	1.3	1.4	1.4	1.4	1.5
9.500	1.5	1.6	1.6	1.6	1.7
10.000	1.7	1.8	1.8	1.9	1.9
10.500	2.0	2.0	2.1	2.2	2.2
11.000	2.3	2.4	2.4	2.5	2.6
11.500	2.7	2.9	3.1	3.4	3.8
12.000	4.6	5.3	5.7	6.0	6.3
12.500	6.4	6.5	6.6	6.7	6.8
13.000	6.9	6.9	7.0	7.1	7.1
13.500	7.2	7.2	7.3	7.3	7.4
14.000	7.4	7.5	7.5	7.6	7.6
14.500	7.6	7.7	7.7	7.7	7.8
15.000	7.8	7.9	7.9	7.9	7.9
15.500	8.0	8.0	8.0	8.1	8.1
16.000	8.1	8.1	8.2	8.2	8.2
16.500	8.2	8.2	8.3	8.3	8.3
17.000	8.3	8.3	8.4	8.4	8.4

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## CUMULATIVE RAINFALL (in) Output Time Increment = 0.100 hours Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
17.500	8.4	8.4	8.4	8.5	8.5
18.000	8.5	8.5	8.5	8.5	8.5
18.500	8.6	8.6	8.6	8.6	8.6
19.000	8.6	8.6	8.7	8.7	8.7
19.500	8.7	8.7	8.7	8.7	8.7
20.000	8.8	8.8	8.8	8.8	8.8
20.500	8.8	8.8	8.8	8.8	8.9
21.000	8.9	8.9	8.9	8.9	8.9
21.500	8.9	8.9	8.9	9.0	9.0
22.000	9.0	9.0	9.0	9.0	9.0
22.500	9.0	9.0	9.0	9.0	9.1
23.000	9.1	9.1	9.1	9.1	9.1
23.500	9.1	9.1	9.1	9.1	9.1
24.000	9.2	(N/A)	(N/A)	(N/A)	(N/A)

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#### Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	110.00 ft
Manning's n	0.150
Slope	0.040 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.24 ft/s
Segment Time of	0 129 hours
Concentration	0.129 110013
Segment #2: TR-55 Shallow Concer	ntrated Flow
Hydraulic Length	62.00 ft
Is Paved?	True
Slope	0.030 ft/ft
Average Velocity	3.52 ft/s
Segment Time of	0.005 hours
Concentration	
Segment #3: TR-55 Channel Flow	
Flow Area	3.1 ft <sup>2</sup>
Hydraulic Length	507.00 ft
Manning's n	0.011
Slope	0.003 ft/ft
Wetted Perimeter	6.28 ft
Average Velocity	4.35 ft/s
Segment Time of	0.032 hours
Concentration	
Time of Concentration (Composite)	
Time of Concentration (Composite)	0.166 hours

Return Event: 1 years Storm Event: 1 Subsection: Time of Concentration Calculations Label: EDA-1A Scenario: Existing Conditions 1 Year Storm

#### ==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius
	Aq= Flow area, square feet
	Wp= Wetted perimeter, feet
	V= Velocity, ft/sec
	Sf= Slope, ft/ft
	n= Manning's n
	Tc= Time of concentration, hours
	Lf= Flow length, feet

## ==== SCS TR-55 Shallow Concentration Flow

Tc =

Unpaved surface: V = 16.1345 \* (Sf\*\*0.5)

Paved Surface: V = 20.3282 \* (Sf\*\*0.5)

Where:

(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet

#### ==== SCS TR-55 Sheet Flow

 $\begin{array}{ll} Tc = & \left(0.007 * \left((n * Lf) * 0.8\right)\right) / \left((P * * 0.5) * (Sf * * 0.4)\right) \\ \text{Where:} & Tc = Time of concentration, hours } \\ n = Manning's n \\ Lf = Flow length, feet \\ P = 2yr, 24hr Rain depth, inches \\ Sf = Slope, \% \end{array}$ 

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#### Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	110.00 ft
Manning's n	0.150
Slope	0.040 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.24 ft/s
Segment Time of Concentration	0.129 hours
Segment #2: TR-55 Shallow Concer	trated Flow
Hydraulic Length	62.00 ft
Is Paved?	True
Slope	0.030 ft/ft
Average Velocity	3.52 ft/s
Segment Time of Concentration	0.005 hours
Segment #3: TR-55 Channel Flow	
Flow Area	3.1 ft <sup>2</sup>
Hydraulic Length	507.00 ft
Manning's n	0.011
Slope	0.003 ft/ft
Wetted Perimeter	6.28 ft
Average Velocity	4.35 ft/s
Segment Time of	0.032 hours
Concentration	
Time of Concentration (Composite)	
Time of Concentration	

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Subsection: Time of Concentration Calculations Label: EDA-1A Scenario: Existing Conditions 10 Year Storm

#### ==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius
	Aq= Flow area, square feet
	Wp= Wetted perimeter, feet
	V= Velocity, ft/sec
	Sf= Slope, ft/ft
	n= Manning's n
	Tc= Time of concentration, hours
	Lf= Flow length, feet

## ==== SCS TR-55 Shallow Concentration Flow

Tc =

Unpaved surface: V = 16.1345 \* (Sf\*\*0.5)

Paved Surface: V = 20.3282 \* (Sf\*\*0.5)

Where:

(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet

#### ==== SCS TR-55 Sheet Flow

 $\begin{array}{ll} Tc = & \left(0.007 * \left((n * Lf) * 0.8\right)\right) / \left((P * * 0.5) * (Sf * * 0.4)\right) \\ \text{Where:} & Tc = Time of concentration, hours } \\ n = Manning's n \\ Lf = Flow length, feet \\ P = 2yr, 24hr Rain depth, inches \\ Sf = Slope, \% \end{array}$ 

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Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	110.00 ft
Manning's n	0.150
Slope	0.040 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.24 ft/s
Segment Time of	0 129 hours
Concentration	0.129 110015
Segment #2: TR-55 Shallow Concer	ntrated Flow
Hydraulic Length	62.00 ft
Is Paved?	True
Slope	0.030 ft/ft
Average Velocity	3.52 ft/s
Segment Time of	0 005 hours
Concentration	0.005 110015
Segment #3: TR-55 Channel Flow	
Flow Area	3.1 ft <sup>2</sup>
Hydraulic Length	507.00 ft
Manning's n	0.011
Slope	0.003 ft/ft
Wetted Perimeter	6.28 ft
Average Velocity	4.35 ft/s
Segment Time of	0.032 hours
Concentration	
Time of Concentration (Composite)	
Time of Concentration (Composite)	0.166 hours

Return Event: 100 years Storm Event: 100

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#### ==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius Aq= Flow area, square feet Wp= Wetted perimeter, feet V= Velocity, ft/sec Sf= Slope, ft/ft n= Manning's n Tc= Time of concentration, hours Lf= Flow length, feet

## ==== SCS TR-55 Shallow Concentration Flow

Tc =

Unpaved surface: V = 16.1345 \* (Sf\*\*0.5)

Paved Surface: V = 20.3282 \* (Sf\*\*0.5)

Where:

(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet

#### ==== SCS TR-55 Sheet Flow

 $\begin{array}{ll} Tc = & \left(0.007 * \left((n * Lf) * 0.8\right)\right) / \left((P * * 0.5) * (Sf * * 0.4)\right) \\ \text{Where:} & Tc = Time of concentration, hours } \\ n = Manning's n \\ Lf = Flow length, feet \\ P = 2yr, 24hr Rain depth, inches \\ Sf = Slope, \% \end{array}$ 

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#### Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	92.00 ft
Manning's n	0.400
Slope	0.008 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.05 ft/s
Segment Time of Concentration	0.469 hours
Segment #2: TR-55 Shallow Conce	ntrated Flow
Hydraulic Length	320.00 ft
Is Paved?	False
Slope	0.008 ft/ft
Average Velocity	1.44 ft/s
Segment Time of Concentration	0.062 hours
Time of Concentration (Composite)	
Time of Concentration (Composite)	0.530 hours

Return Event: 1 years Storm Event: 1 Subsection: Time of Concentration Calculations Label: EDA-1B Scenario: Existing Conditions 1 Year Storm

#### ==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius
	Aq= Flow area, square feet
	Wp= Wetted perimeter, feet
	V= Velocity, ft/sec
	Sf= Slope, ft/ft
	n= Manning's n
	Tc= Time of concentration, hours
	Lf= Flow length, feet

## ==== SCS TR-55 Shallow Concentration Flow

Tc =

Unpaved surface: V = 16.1345 \* (Sf\*\*0.5)

Paved Surface: V = 20.3282 \* (Sf\*\*0.5)

Where:

(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet Return Event: 1 years Storm Event: 1 Subsection: Time of Concentration Calculations Label: EDA-1B Scenario: Existing Conditions 10 Year Storm

#### Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	92.00 ft
Manning's n	0.400
Slope	0.008 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.05 ft/s
Segment Time of Concentration	0.469 hours
Segment #2: TR-55 Shallow Concentrated Flow	
Hydraulic Length	320.00 ft
Is Paved?	False
Slope	0.008 ft/ft
Average Velocity	1.44 ft/s
Segment Time of Concentration	0.062 hours
Time of Concentration (Composite)	
Time of Concentration (Composite)	0.530 hours

Return Event: 10 years Storm Event: 10 Subsection: Time of Concentration Calculations Label: EDA-1B Scenario: Existing Conditions 10 Year Storm

#### ==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius
	Aq= Flow area, square feet
	Wp= Wetted perimeter, feet
	V= Velocity, ft/sec
	Sf= Slope, ft/ft
	n= Manning's n
	Tc= Time of concentration, hours
	Lf= Flow length, feet

## ==== SCS TR-55 Shallow Concentration Flow

Tc =

Unpaved surface: V = 16.1345 \* (Sf\*\*0.5)

Paved Surface: V = 20.3282 \* (Sf\*\*0.5)

Where:

(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet

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Subsection: Time of Concentration Calculations Label: EDA-1B Scenario: Existing Conditions 100 Year Storm

Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	92.00 ft
Manning's n	0.400
Slope	0.008 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.05 ft/s
Segment Time of Concentration	0.469 hours
Segment #2: TR-55 Shallow Conce	ntrated Flow
Hydraulic Length	320.00 ft
Is Paved?	False
Slope	0.008 ft/ft
Average Velocity	1.44 ft/s
Segment Time of Concentration	0.062 hours
Time of Concentration (Composite)	
Time of Concentration (Composite)	0.530 hours

Subsection: Time of Concentration Calculations Label: EDA-1B Scenario: Existing Conditions 100 Year Storm

### ==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius
	Aq= Flow area, square feet
	Wp= Wetted perimeter, feet
	V= Velocity, ft/sec
	Sf= Slope, ft/ft
	n= Manning's n
	Tc= Time of concentration, hours
	Lf= Flow length, feet

# ==== SCS TR-55 Shallow Concentration Flow

Т	с	=

Unpaved surface: V = 16.1345 \* (Sf\*\*0.5)

Paved Surface: V = 20.3282 \* (Sf\*\*0.5)

Where:

(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet

Subsection: Time of Concentration Calculations Label: PDA-1A Scenario: Proposed Conditions 1 Year Storm Time of Concentration Results

Segment #1: User Defined Tc

Time of Concentration

0.083 hours

 Time of Concentration (Composite)

 Time of Concentration (Composite)

 0.083 hours

Return Event: 1 years Storm Event: 1

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### ==== User Defined

Tc =	Value entered by user
Where:	Tc= Time of concentration, hours

Subsection: Time of Concentration Calculations Label: PDA-1A Scenario: Proposed Conditions 10 Year Storm Time of Concentration Results

Segment #1: User Defined Tc

Time of Concentration

0.083 hours

Time of Concentration (Composite) Time of Concentration 0.083 hours (Composite)

Subsection: Time of Concentration Calculations Label: PDA-1A Scenario: Proposed Conditions 10 Year Storm

### ==== User Defined

Tc =	Value entered by user
Where:	Tc= Time of concentration, hours

Return Event: 10 years Storm Event: 10

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Subsection: Time of Concentration Calculations Label: PDA-1A Scenario: Proposed Conditions 100 Year Storm Time of Concentration Results

Segment #1: User Defined Tc

Time of Concentration

0.083 hours

Time of Concentration (Composite) Time of Concentration 0.083 hours (Composite)

Subsection: Time of Concentration Calculations Label: PDA-1A Scenario: Proposed Conditions 100 Year Storm

### ==== User Defined

Tc =	Value entered by user
Where:	Tc= Time of concentration, hours

Subsection: Time of Concentration Calculations Label: PDA-1B Scenario: Proposed Conditions 1 Year Storm Time of Concentration Results

Segment #1: User Defined Tc

Time of Concentration

0.083 hours

 Time of Concentration (Composite)

 Time of Concentration (Composite)

 0.083 hours

Return Event: 1 years Storm Event: 1

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### ==== User Defined

Tc =	Value entered by user
Where:	Tc= Time of concentration, hours

Subsection: Time of Concentration Calculations Label: PDA-1B Scenario: Proposed Conditions 10 Year Storm Time of Concentration Results

Segment #1: User Defined Tc

Time of Concentration

0.083 hours

 Time of Concentration (Composite)

 Time of Concentration (Composite)

 0.083 hours

Subsection: Time of Concentration Calculations Label: PDA-1B Scenario: Proposed Conditions 10 Year Storm

### ==== User Defined

Tc =	Value entered by user
Where:	Tc= Time of concentration, hours

Return Event: 10 years Storm Event: 10

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 37 of 448 Subsection: Time of Concentration Calculations Label: PDA-1B Scenario: Proposed Conditions 100 Year Storm Time of Concentration Results

Segment #1: User Defined Tc

Time of Concentration

0.083 hours

 Time of Concentration (Composite)

 Time of Concentration (Composite)

 0.083 hours

Subsection: Time of Concentration Calculations Label: PDA-1B Scenario: Proposed Conditions 100 Year Storm

### ==== User Defined

Tc =	Value entered by user
Where:	Tc= Time of concentration, hours

Subsection: Time of Concentration Calculations Label: PDA-1C Scenario: Proposed Conditions 1 Year Storm Time of Concentration Results

Segment #1: User Defined Tc

Time of Concentration

0.083 hours

 Time of Concentration (Composite)

 Time of Concentration (Composite)

 0.083 hours

Return Event: 1 years Storm Event: 1

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### ==== User Defined

Tc =	Value entered by user
Where:	Tc= Time of concentration, hours

Subsection: Time of Concentration Calculations Label: PDA-1C Scenario: Proposed Conditions 10 Year Storm Time of Concentration Results

Segment #1: User Defined Tc

Time of Concentration

0.083 hours

 Time of Concentration (Composite)

 Time of Concentration (Composite)

 0.083 hours

Subsection: Time of Concentration Calculations Label: PDA-1C Scenario: Proposed Conditions 10 Year Storm

### ==== User Defined

Tc =	Value entered by user
Where:	Tc= Time of concentration, hours

Return Event: 10 years Storm Event: 10

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Segment #1: User Defined Tc

Time of Concentration

0.083 hours

 Time of Concentration (Composite)

 Time of Concentration (Composite)

 0.083 hours

Subsection: Time of Concentration Calculations Label: PDA-1C Scenario: Proposed Conditions 100 Year Storm

### ==== User Defined

Tc =	Value entered by user
Where:	Tc= Time of concentration, hours

Subsection: Time of Concentration Calculations Label: PDA-1D Scenario: Proposed Conditions 1 Year Storm

# Time of Concentration Results

Segment #1: TR-55 Sheet Flow					
Hydraulic Length	110.00 ft				
Manning's n	0.150				
Slope	0.040 ft/ft				
2 Year 24 Hour Depth	3.4 in				
Average Velocity	0.24 ft/s				
Segment Time of	0.130 hours				
Concentration					
Segment #2: TR-55 Shallow Concer	ntrated Flow				
Hydraulic Length	62.00 ft				
Is Paved?	True				
Slope	0.030 ft/ft				
Average Velocity	3.52 ft/s				
Segment Time of Concentration	0.005 hours				
Segment #3: TR-55 Channel Flow					
Flow Area	3.1 ft <sup>2</sup>				
Hydraulic Length	503.00 ft				
Manning's n	0.013				
Slope	0.002 ft/ft				
Wetted Perimeter	6.28 ft				
Average Velocity	2.98 ft/s				
Segment Time of	0.047 hours				
Concentration					
Time of Concentration (Composite)					
Time of Concentration (Composite)	0.181 hours				

Subsection: Time of Concentration Calculations Label: PDA-1D Scenario: Proposed Conditions 1 Year Storm

#### ==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius Aq= Flow area, square feet Wp= Wetted perimeter, feet V= Velocity, ft/sec Sf= Slope, ft/ft n= Manning's n Tc= Time of concentration, hours Lf= Flow length, feet

# ==== SCS TR-55 Shallow Concentration Flow

Tc =

Unpaved surface: V = 16.1345 \* (Sf\*\*0.5)

Paved Surface: V = 20.3282 \* (Sf\*\*0.5)

Where:

(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet

## ==== SCS TR-55 Sheet Flow

 $\begin{array}{ll} Tc = & \left(0.007 * \left((n * Lf) * 0.8\right)\right) / \left((P * * 0.5) * (Sf * * 0.4)\right) \\ \text{Where:} & Tc = Time of concentration, hours } \\ n = Manning's n \\ Lf = Flow length, feet \\ P = 2yr, 24hr Rain depth, inches \\ Sf = Slope, \% \end{array}$ 

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[10.02.00.01] Page 47 of 448 Subsection: Time of Concentration Calculations Label: PDA-1D

Scenario: Proposed Conditions 10 Year Storm

# Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	110.00 ft
Manning's n	0.150
Slope	0.040 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.24 ft/s
Segment Time of Concentration	0.130 hours
Segment #2: TR-55 Shallow Conce	ntrated Flow
Hydraulic Length	62.00 ft
Is Paved?	True
Slope	0.030 ft/ft
Average Velocity	3.52 ft/s
Segment Time of	0.005 hours
Concentration	
Segment #3: TR-55 Channel Flow	
Flow Area	3.1 ft <sup>2</sup>
Hydraulic Length	503.00 ft
Manning's n	0.013
Slope	0.002 ft/ft
Slope Wetted Perimeter	0.002 ft/ft 6.28 ft
Slope Wetted Perimeter Average Velocity	0.002 ft/ft 6.28 ft 2.98 ft/s
Slope Wetted Perimeter Average Velocity Segment Time of	0.002 ft/ft 6.28 ft 2.98 ft/s 0.047 hours
Slope Wetted Perimeter Average Velocity Segment Time of Concentration	0.002 ft/ft 6.28 ft 2.98 ft/s 0.047 hours
Slope Wetted Perimeter Average Velocity Segment Time of Concentration Time of Concentration (Composite)	0.002 ft/ft 6.28 ft 2.98 ft/s 0.047 hours

Subsection: Time of Concentration Calculations Label: PDA-1D Scenario: Proposed Conditions 10 Year Storm

#### ==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius
	Aq= Flow area, square feet
	Wp= Wetted perimeter, feet
	V= Velocity, ft/sec
	Sf= Slope, ft/ft
	n= Manning's n
	Tc= Time of concentration, hours
	Lf= Flow length, feet

# ==== SCS TR-55 Shallow Concentration Flow

Tc =

Unpaved surface: V = 16.1345 \* (Sf\*\*0.5)

Paved Surface: V = 20.3282 \* (Sf\*\*0.5)

Where:

(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet

## ==== SCS TR-55 Sheet Flow

 $\begin{array}{ll} Tc = & \left(0.007 * \left((n * Lf) * 0.8\right)\right) / \left((P * * 0.5) * (Sf * * 0.4)\right) \\ \text{Where:} & Tc = Time of concentration, hours } \\ n = Manning's n \\ Lf = Flow length, feet \\ P = 2yr, 24hr Rain depth, inches \\ Sf = Slope, \% \end{array}$ 

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[10.02.00.01] Page 49 of 448 Subsection: Time of Concentration Calculations Label: PDA-1D

Scenario: Proposed Conditions 100 Year Storm

# Time of Concentration Results

Segment #1: TR-55 Sheet Flow	
Hydraulic Length	110.00 ft
Manning's n	0.150
Slope	0.040 ft/ft
2 Year 24 Hour Depth	3.4 in
Average Velocity	0.24 ft/s
Segment Time of Concentration	0.130 hours
Segment #2: TR-55 Shallow Conce	ntrated Flow
Hydraulic Length	62.00 ft
Is Paved?	True
Slope	0.030 ft/ft
Average Velocity	3.52 ft/s
Segment Time of	0.005 hours
Concentration	
Segment #3: TR-55 Channel Flow	
Flow Area	3.1 ft <sup>2</sup>
Hydraulic Length	
, 5	503.00 ft
Manning's n	503.00 ft 0.013
Manning's n Slope	0.013 0.002 ft/ft
Manning's n Slope Wetted Perimeter	503.00 ft 0.013 0.002 ft/ft 6.28 ft
Manning's n Slope Wetted Perimeter Average Velocity	503.00 ft 0.013 0.002 ft/ft 6.28 ft 2.98 ft/s
Manning's n Slope Wetted Perimeter Average Velocity Segment Time of	503.00 ft 0.013 0.002 ft/ft 6.28 ft 2.98 ft/s 0.047 hours
Manning's n Slope Wetted Perimeter Average Velocity Segment Time of Concentration	503.00 ft 0.013 0.002 ft/ft 6.28 ft 2.98 ft/s 0.047 hours
Manning's n Slope Wetted Perimeter Average Velocity Segment Time of Concentration	503.00 ft 0.013 0.002 ft/ft 6.28 ft 2.98 ft/s 0.047 hours

Subsection: Time of Concentration Calculations Label: PDA-1D Scenario: Proposed Conditions 100 Year Storm

#### ==== SCS Channel Flow

Tc =	R = Qa / Wp V = (1.49 * (R**(2/3)) * (Sf**-0.5)) / n
	(Lf / V) / 3600
Where:	R= Hydraulic radius
	Aq= Flow area, square feet
	Wp= Wetted perimeter, feet
	V= Velocity, ft/sec
	Sf= Slope, ft/ft
	n= Manning's n
	Tc= Time of concentration, hours
	Lf= Flow length, feet

# ==== SCS TR-55 Shallow Concentration Flow

Tc =

Unpaved surface: V = 16.1345 \* (Sf\*\*0.5)

Paved Surface: V = 20.3282 \* (Sf\*\*0.5)

Where:

(Lf / V) / 3600 V= Velocity, ft/sec Sf= Slope, ft/ft Tc= Time of concentration, hours Lf= Flow length, feet

## ==== SCS TR-55 Sheet Flow

 $\begin{array}{ll} Tc = & \left(0.007 * \left((n * Lf) * 0.8\right)\right) / \left((P * * 0.5) * (Sf * * 0.4)\right) \\ \text{Where:} & Tc = Time of concentration, hours } \\ n = Manning's n \\ Lf = Flow length, feet \\ P = 2yr, 24hr Rain depth, inches \\ Sf = Slope, \% \end{array}$ 

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[10.02.00.01] Page 51 of 448 Subsection: Runoff CN-Area Label: EDA-1A Scenario: Existing Conditions 1 Year Storm

# **Runoff Curve Number Data**

Soil/Surface Description	CN	Area (ft²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil A	98.000	138,487.966	0.0	0.0	98.000
Pasture, grassland, or range - good - Soil B	61.000	24,708.545	0.0	0.0	61.000
Pasture, grassland, or range - good - Soil D	80.000	41,513.947	0.0	0.0	80.000
Woods - good - Soil B	55.000	7,825.884	0.0	0.0	55.000
Woods - good - Soil D	77.000	3,348.405	0.0	0.0	77.000
COMPOSITE AREA & WEIGHTED CN>	(N/A)	215,884.747	(N/A)	(N/A)	88.419

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# **Runoff Curve Number Data**

Soil/Surface Description	CN	Area (ft²)	C (%)	UC (%)	Adjusted CN
Pasture, grassland, or range - good - Soil B	61.000	19,262.670	0.0	0.0	61.000
Pasture, grassland, or range - good - Soil D	80.000	1,637.698	0.0	0.0	80.000
Woods - good - Soil B	55.000	119,963.994	0.0	0.0	55.000
Woods - good - Soil D	77.000	2,477.730	0.0	0.0	77.000
COMPOSITE AREA & WEIGHTED CN>	(N/A)	143,342.092	(N/A)	(N/A)	56.472

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## **Runoff Curve Number Data**

Soil/Surface Description	CN	Area (ft²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil A	98.000	74,850.000	0.0	0.0	98.000
Pasture, grassland, or range - good - Soil B	61.000	25,667.148	0.0	0.0	61.000
COMPOSITE AREA & WEIGHTED CN>	(N/A)	100,517.148	(N/A)	(N/A)	88.552

Return Event: 1 years Storm Event: 1

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# **Runoff Curve Number Data**

Soil/Surface Description	CN	Area	С	UC	Adjusted CN
		(ft²)	(%)	(%)	
Impervious Areas - Paved parking lots,	98.000	20,544.127	0.0	0.0	98.000
roofs, driveways, Streets and roads - Soil					
В					
Pasture, grassland, or range - good - Soil	61.000	3,953.153	0.0	0.0	61.000
В					
Pasture, grassland, or range - good - Soil	80.000	281.913	0.0	0.0	80.000
D					
COMPOSITE AREA & WEIGHTED CN>	(N/A)	24,779.193	(N/A)	(N/A)	91.892

Return Event: 1 years Storm Event: 1

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# **Runoff Curve Number Data**

Soil/Surface Description	CN	Area	С	UC	Adjusted CN
		(ft²)	(%)	(%)	
Impervious Areas - Paved parking lots,	98.000	23,394.493	0.0	0.0	98.000
roofs, driveways, Streets and roads - Soil					
A					
Pasture, grassland, or range - good - Soil	61.000	5,432.232	0.0	0.0	61.000
В					
Pasture, grassland, or range - good - Soil	80.000	5,923.610	0.0	0.0	80.000
D					
COMPOSITE AREA & WEIGHTED CN>	(N/A)	34,750.336	(N/A)	(N/A)	89.148

Return Event: 1 years Storm Event: 1

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# **Runoff Curve Number Data**

Soil/Surface Description	CN	Area (ft²)	C (%)	UC (%)	Adjusted CN
Impervious Areas - Paved parking lots, roofs, driveways, Streets and roads - Soil A	98.000	117,926.625	0.0	0.0	98.000
Pasture, grassland, or range - good - Soil B	61.000	32,571.565	0.0	0.0	61.000
Pasture, grassland, or range - good - Soil D	80.000	37,138.274	0.0	0.0	80.000
Woods - good - Soil B	55.000	10,442.771	0.0	0.0	55.000
Woods - good - Soil D	77.000	1,100.928	0.0	0.0	77.000
COMPOSITE AREA & WEIGHTED CN>	(N/A)	199,180.163	(N/A)	(N/A)	86.223

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Scenario: Existing Conditions 1 Year Storm

-

Storm Event	1
Return Event	1 years
Duration	24.000 hours
Depth	2.8 in
Time of Concentration (Composite)	0.166 hours
Area (User Defined)	215,884.747 ft <sup>2</sup>

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
7,400	0.00	0.00	0.00	0.00	0.01
7.650	0.01	0.01	0.01	0.01	0.01
7.900	0.02	0.02	0.02	0.02	0.02
8.150	0.03	0.03	0.03	0.03	0.04
8,400	0.04	0.04	0.04	0.05	0.05
8.650	0.05	0.06	0.06	0.06	0.07
8.900	0.07	0.08	0.08	0.08	0.09
9.150	0.09	0.10	0.10	0.11	0.11
9.400	0.12	0.12	0.13	0.13	0.14
9.650	0.14	0.15	0.16	0.16	0.17
9.900	0.17	0.18	0.19	0.19	0.20
10.150	0.21	0.22	0.22	0.23	0.24
10.400	0.25	0.26	0.28	0.29	0.30
10.650	0.31	0.32	0.33	0.35	0.36
10.900	0.37	0.38	0.40	0.41	0.43
11.150	0.46	0.49	0.52	0.56	0.59
11.400	0.63	0.68	0.72	0.79	0.92
11.650	1.10	1.38	1.71	2.09	2.49
11.900	2.94	3.66	5.06	6.50	7.47
12.150	7.71	6.84	5.69	4.81	4.18
12.400	3.63	3.12	2.62	2.18	1.81
12.650	1.54	1.38	1.27	1.20	1.13
12.900	1.08	1.03	0.97	0.93	0.89
13.150	0.85	0.83	0.81	0.80	0.78
13.400	0.77	0.76	0.74	0.73	0.72
13.650	0.70	0.69	0.68	0.66	0.65
13.900	0.64	0.62	0.61	0.60	0.59
14.150	0.58	0.57	0.56	0.55	0.55
14.400	0.54	0.54	0.53	0.52	0.52
14.650	0.51	0.50	0.50	0.49	0.48
14.900	0.48	0.47	0.46	0.46	0.45
15.150	0.44	0.44	0.43	0.42	0.42
15.400	0.41	0.40	0.40	0.39	0.38
15.650	0.38	0.37	0.36	0.36	0.35

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Return Event: 1 years Storm Event: 1

Scenario: Existing Conditions 1 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(10015)	(1075)	(119/5)	(113/5)	(113/5)	(112/5)
15.900	0.34	0.34	0.33	0.32	0.52
16.150	0.31	0.31	0.31	0.30	0.30
16.400	0.30	0.29	0.29	0.29	0.20
16.000	0.20	0.20	0.20	0.27	0.27
10.900	0.27	0.26	0.26	0.20	0.20
17.150	0.25	0.25	0.25	0.24	0.24
17.400	0.24	0.23	0.23	0.23	0.23
17.050	0.22	0.22	0.22	0.21	0.21
17.900	0.21	0.21	0.20	0.20	0.20
18.150	0.19	0.19	0.19	0.19	0.19
18.400	0.19	0.19	0.19	0.19	0.19
18.050	0.19	0.18	0.18	0.18	0.18
18.900	0.18	0.18	0.18	0.18	0.18
19.150	0.18	0.18	0.17	0.17	0.17
19.400	0.17	0.17	0.17	0.17	0.17
19.050	0.17	0.17	0.17	0.16	0.16
19.900	0.16	0.16	0.16	0.16	0.16
20.150	0.16	0.16	0.16	0.16	0.16
20.400	0.16	0.15	0.15	0.15	0.15
20.650	0.15	0.15	0.15	0.15	0.15
20.900	0.15	0.15	0.15	0.15	0.15
21.150	0.15	0.14	0.14	0.14	0.14
21.400	0.14	0.14	0.14	0.14	0.14
21.650	0.14	0.14	0.14	0.14	0.14
21.900	0.14	0.13	0.13	0.13	0.13
22.150	0.13	0.13	0.13	0.13	0.13
22.400	0.13	0.13	0.13	0.13	0.13
22.650	0.13	0.12	0.12	0.12	0.12
22.900	0.12	0.12	0.12	0.12	0.12
23.150	0.12	0.12	0.12	0.12	0.12
23.400	0.11	0.11	0.11	0.11	0.11
23.650	0.11	0.11	0.11	0.11	0.11
23.900	0.11	0.11	0.11	(N/A)	(N/A)

Scenario: Existing Conditions 10 Year Storm

-

Storm Event	10
Return Event	10 years
Duration	24.000 hours
Depth	5.1 in
Time of Concentration (Composite)	0.166 hours
Area (User Defined)	215,884.747 ft <sup>2</sup>

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
	(11-75)	(11-75)	(11-75)	(11-75)	(11-75)
4.050	0.00	0.00	0.00	0.00	0.01
5.100	0.01	0.01	0.01	0.01	0.02
5.350	0.02	0.02	0.02	0.02	0.03
5.000	0.03	0.03	0.03	0.04	0.04
5.850	0.04	0.04	0.04	0.05	0.05
6.100	0.05	0.05	0.06	0.06	0.06
6.350	0.07	0.07	0.07	0.07	0.08
6.600	0.08	0.09	0.09	0.09	0.10
6.850	0.10	0.10	0.11	0.11	0.12
/.100	0.12	0.13	0.13	0.13	0.14
7.350	0.14	0.15	0.15	0.16	0.16
7.600	0.17	0.17	0.18	0.18	0.19
7.850	0.19	0.20	0.20	0.21	0.22
8.100	0.22	0.23	0.24	0.25	0.25
8.350	0.26	0.27	0.28	0.29	0.30
8.600	0.31	0.32	0.33	0.34	0.35
8.850	0.37	0.38	0.39	0.40	0.41
9.100	0.42	0.44	0.45	0.46	0.47
9.350	0.49	0.50	0.51	0.52	0.54
9.600	0.55	0.57	0.58	0.59	0.61
9.850	0.62	0.64	0.65	0.66	0.68
10.100	0.70	0.72	0.74	0.76	0.79
10.350	0.81	0.84	0.86	0.89	0.92
10.600	0.94	0.97	1.00	1.03	1.06
10.850	1.08	1.11	1.14	1.17	1.21
11.100	1.25	1.31	1.39	1.47	1.56
11.350	1.65	1.75	1.85	1.95	2.11
11.600	2.44	2.91	3.59	4.37	5.27
11.850	6.19	7.20	8.78	11.88	14.95
12.100	16.84	17.11	14.99	12.34	10.34
12.350	8.91	7.68	6.58	5.51	4.57
12.600	3.79	3.21	2.86	2.64	2.48
12.850	2.35	2.23	2.12	2.01	1.91
13.100	1.82	1.76	1.71	1.67	1.64

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Return Event: 10 years Storm Event: 10

Scenario: Existing Conditions 10 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) **Output Time Increment = 0.050 hours** Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
13.350	1.61	1.58	1.55	1.52	1.50
13.600	1.47	1.44	1.41	1.38	1.36
13.850	1.33	1.30	1.27	1.24	1.22
14.100	1.19	1.17	1.15	1.14	1.12
14.350	1.11	1.10	1.08	1.07	1.06
14.600	1.04	1.03	1.02	1.00	0.99
14.850	0.98	0.96	0.95	0.94	0.92
15.100	0.91	0.89	0.88	0.87	0.85
15.350	0.84	0.83	0.81	0.80	0.79
15.600	0.77	0.76	0.74	0.73	0.72
15.850	0.70	0.69	0.68	0.66	0.65
16.100	0.64	0.63	0.62	0.61	0.61
16.350	0.60	0.59	0.59	0.58	0.58
16.600	0.57	0.56	0.56	0.55	0.55
16.850	0.54	0.53	0.53	0.52	0.52
17.100	0.51	0.51	0.50	0.49	0.49
17.350	0.48	0.47	0.47	0.46	0.46
17.600	0.45	0.44	0.44	0.43	0.43
17.850	0.42	0.41	0.41	0.40	0.40
18.100	0.39	0.39	0.39	0.38	0.38
18.350	0.38	0.38	0.38	0.37	0.37
18.600	0.37	0.37	0.37	0.36	0.36
18.850	0.36	0.36	0.36	0.36	0.35
19.100	0.35	0.35	0.35	0.35	0.35
19.350	0.34	0.34	0.34	0.34	0.34
19.600	0.33	0.33	0.33	0.33	0.33
19.850	0.33	0.32	0.32	0.32	0.32
20.100	0.32	0.31	0.31	0.31	0.31
20.350	0.31	0.31	0.31	0.31	0.30
20.600	0.30	0.30	0.30	0.30	0.30
20.850	0.30	0.29	0.29	0.29	0.29
21.100	0.29	0.29	0.29	0.29	0.28
21.350	0.28	0.28	0.28	0.28	0.28
21.600	0.28	0.27	0.27	0.27	0.27
21.850	0.27	0.27	0.27	0.26	0.26
22.100	0.26	0.26	0.26	0.26	0.26
22.350	0.25	0.25	0.25	0.25	0.25
22.600	0.25	0.25	0.25	0.24	0.24
22.850	0.24	0.24	0.24	0.24	0.24
23.100	0.23	0.23	0.23	0.23	0.23
23.350	0.23	0.23	0.23	0.22	0.22
23.600	0.22	0.22	0.22	0.22	0.22
23.850	0.21	0.21	0.21	0.21	(N/A)

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Scenario: Existing Conditions 100 Year Storm

Storm Event	100
Return Event	100 years
Duration	24.000 hours
Depth	9.2 in
Time of Concentration (Composite)	0.166 hours
Area (User Defined)	215,884.747 ft <sup>2</sup>

### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) **Output Time Increment = 0.050 hours** Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
3.000	0.00	0.00	0.01	0.01	0.01
3.250	0.02	0.02	0.02	0.03	0.03
3.500	0.04	0.04	0.05	0.05	0.05
3.750	0.06	0.06	0.07	0.07	0.08
4.000	0.08	0.09	0.09	0.09	0.10
4.250	0.10	0.11	0.11	0.12	0.12
4.500	0.13	0.13	0.14	0.14	0.15
4.750	0.15	0.16	0.16	0.16	0.17
5.000	0.17	0.18	0.18	0.19	0.19
5.250	0.20	0.20	0.21	0.21	0.22
5.500	0.22	0.23	0.23	0.24	0.24
5.750	0.25	0.25	0.26	0.26	0.27
6.000	0.27	0.28	0.29	0.29	0.30
6.250	0.31	0.32	0.32	0.33	0.34
6.500	0.35	0.36	0.37	0.38	0.39
6.750	0.40	0.41	0.42	0.43	0.44
7.000	0.45	0.46	0.47	0.48	0.49
7.250	0.50	0.51	0.52	0.53	0.54
7.500	0.56	0.57	0.58	0.59	0.60
7.750	0.61	0.62	0.64	0.65	0.66
8.000	0.67	0.68	0.70	0.72	0.73
8.250	0.75	0.78	0.80	0.82	0.84
8.500	0.87	0.89	0.91	0.93	0.96
8.750	0.98	1.01	1.03	1.06	1.08
9.000	1.11	1.13	1.16	1.18	1.21
9.250	1.23	1.26	1.29	1.31	1.34
9.500	1.37	1.39	1.42	1.45	1.48
9.750	1.50	1.53	1.56	1.59	1.61
10.000	1.64	1.67	1.71	1.75	1.79
10.250	1.84	1.89	1.94	1.99	2.05
10.500	2.10	2.15	2.21	2.26	2.31
10.750	2.37	2.42	2.48	2.53	2.59
11.000	2.65	2.71	2.81	2.93	3.08
11.250	3.25	3.44	3.63	3.82	4.02

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Return Event: 100 years Storm Event: 100

Scenario: Existing Conditions 100 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(nours)	(113/5)	(113/5)	(113/5)	(113/5)	(113/5)
11.500	4.22	4.55	5.22	6.19 14.77	7.60
11./50	9.19	10.99	12.81	14.//	17.84
12.000	23.84	29.09	33.12	33.35	29.05
12.250	23.78	19.83	7.03	14.03	12.51
12.500	10.45	8.05	7.10	0.07	5.40
12.750	4.90	4.07	4.42	4.19	3.99
12.000	3.70	2.07	2.01	2.50	3.20
13.230	2.13	2.07	2.01	2.90	2.91
13,500	2.03	2.00	2.75	2.03	2.04
14 000	2.33	2.55	2.70	2.45	2.30
14.000	2.52	2.27	2.25	2.15	2.13
14.200	2.15	1 97	1 95	1 92	1.89
14 750	1.87	1.57	1.55	1.52	1.05
15,000	1.07	1.01	1.62	1.75	1.77
15.000	1.61	1.59	1.56	1.54	1.51
15.500	1.49	1.46	1.44	1.41	1.38
15.750	1.36	1.33	1.31	1.28	1.26
16.000	1.23	1.21	1.19	1.17	1.15
16.250	1.14	1.13	1.12	1.10	1.09
16.500	1.08	1.07	1.06	1.05	1.04
16.750	1.03	1.01	1.00	0.99	0.98
17.000	0.97	0.96	0.95	0.94	0.92
17.250	0.91	0.90	0.89	0.88	0.87
17.500	0.86	0.85	0.84	0.82	0.81
17.750	0.80	0.79	0.78	0.77	0.76
18.000	0.75	0.74	0.73	0.72	0.71
18.250	0.71	0.71	0.70	0.70	0.70
18.500	0.69	0.69	0.69	0.68	0.68
18.750	0.68	0.67	0.67	0.67	0.66
19.000	0.66	0.66	0.65	0.65	0.65
19.250	0.64	0.64	0.64	0.63	0.63
19.500	0.63	0.62	0.62	0.62	0.61
19.750	0.61	0.61	0.60	0.60	0.59
20.000	0.59	0.59	0.59	0.58	0.58
20.250	0.58	0.58	0.57	0.57	0.57
20.500	0.56	0.56	0.56	0.56	0.55
20.750	0.55	0.55	0.55	0.54	0.54
21.000	0.54	0.54	0.54	0.53	0.53
21.250	0.53	0.52	0.52	0.52	0.52
21.500	0.51	0.51	0.51	0.51	0.50
21.750	0.50	0.50	0.50	0.50	0.49
22.000	0.49	0.49	0.48	0.48	0.48

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Return Event: 100 years Storm Event: 100

Scenario: Existing Conditions 100 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
22.250	0.48	0.47	0.47	0.47	0.47
22.500	0.46	0.46	0.46	0.46	0.45
22.750	0.45	0.45	0.45	0.44	0.44
23.000	0.44	0.44	0.43	0.43	0.43
23.250	0.43	0.42	0.42	0.42	0.42
23.500	0.41	0.41	0.41	0.41	0.40
23.750	0.40	0.40	0.40	0.39	0.39
24.000	0.39	(N/A)	(N/A)	(N/A)	(N/A)

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Scenario: Existing Conditions 1 Year Storm

Storm Event	1
Return Event	1 years
Duration	24.000 hours
Depth	2.8 in
Time of Concentration (Composite)	0.530 hours
Area (User Defined)	143,342.092 ft <sup>2</sup>

### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/S)	(ft³/s)
12.100	0.00	0.00	0.01	0.01	0.02
12.350	0.04	0.05	0.07	0.09	0.10
12.600	0.11	0.12	0.12	0.12	0.12
12.850	0.12	0.12	0.11	0.11	0.11
13.100	0.10	0.10	0.10	0.10	0.09
13.350	0.09	0.09	0.09	0.09	0.09
13.600	0.09	0.09	0.08	0.08	0.08
13.850	0.08	0.08	0.08	0.08	0.08
14.100	0.08	0.08	0.08	0.08	0.08
14.350	0.07	0.07	0.07	0.07	0.07
14.600	0.07	0.07	0.07	0.07	0.07
14.850	0.07	0.07	0.07	0.07	0.07
15.100	0.07	0.07	0.07	0.07	0.06
15.350	0.06	0.06	0.06	0.06	0.06
15.600	0.06	0.06	0.06	0.06	0.06
15.850	0.06	0.06	0.06	0.06	0.05
16.100	0.05	0.05	0.05	0.05	0.05
16.350	0.05	0.05	0.05	0.05	0.05
16.600	0.05	0.05	0.05	0.05	0.05
16.850	0.05	0.05	0.04	0.04	0.04
17.100	0.04	0.04	0.04	0.04	0.04
17.350	0.04	0.04	0.04	0.04	0.04
17.600	0.04	0.04	0.04	0.04	0.04
17.850	0.04	0.04	0.04	0.04	0.04
18.100	0.04	0.04	0.03	0.03	0.03
18.350	0.03	0.03	0.03	0.03	0.03
18.600	0.03	0.03	0.03	0.03	0.03
18.850	0.03	0.03	0.03	0.03	0.03
19.100	0.03	0.03	0.03	0.03	0.03
19.350	0.03	0.03	0.03	0.03	0.03
19.600	0.03	0.03	0.03	0.03	0.03
19.850	0.03	0.03	0.03	0.03	0.03
20.100	0.03	0.03	0.03	0.03	0.03
20.350	0.03	0.03	0.03	0.03	0.03

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Return Event: 1 years Storm Event: 1

Scenario: Existing Conditions 1 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
20.600	0.03	0.03	0.03	0.03	0.03
20.850	0.03	0.03	0.03	0.03	0.03
21.100	0.03	0.03	0.03	0.03	0.03
21.350	0.03	0.03	0.03	0.03	0.03
21.600	0.03	0.03	0.03	0.03	0.03
21.850	0.03	0.03	0.03	0.03	0.03
22.100	0.03	0.03	0.02	0.02	0.02
22.350	0.02	0.02	0.02	0.02	0.02
22.600	0.02	0.02	0.02	0.02	0.02
22.850	0.02	0.02	0.02	0.02	0.02
23.100	0.02	0.02	0.02	0.02	0.02
23.350	0.02	0.02	0.02	0.02	0.02
23.600	0.02	0.02	0.02	0.02	0.02
23.850	0.02	0.02	0.02	0.02	(N/A)

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Scenario: Existing Conditions 10 Year Storm

Storm Event	10
Return Event	10 years
Duration	24.000 hours
Depth	5.1 in
Time of Concentration (Composite)	0.530 hours
Area (User Defined)	143,342.092 ft <sup>2</sup>

#### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
11.600	0.00	0.00	0.00	0.01	0.02
11.850	0.05	0.09	0.15	0.26	0.41
12.100	0.62	0.87	1.15	1.42	1.66
12.350	1.81	1.91	1.96	1.94	1.87
12.600	1.77	1.65	1.53	1.40	1.28
12.850	1.17	1.07	0.98	0.90	0.84
13.100	0.78	0.73	0.69	0.65	0.62
13.350	0.59	0.57	0.55	0.53	0.52
13.600	0.50	0.49	0.48	0.47	0.46
13.850	0.45	0.44	0.43	0.42	0.42
14.100	0.41	0.40	0.39	0.39	0.38
14.350	0.38	0.37	0.37	0.36	0.36
14.600	0.35	0.35	0.34	0.34	0.34
14.850	0.33	0.33	0.33	0.32	0.32
15.100	0.31	0.31	0.31	0.30	0.30
15.350	0.30	0.29	0.29	0.28	0.28
15.600	0.28	0.27	0.27	0.26	0.26
15.850	0.26	0.25	0.25	0.24	0.24
16.100	0.24	0.23	0.23	0.22	0.22
16.350	0.22	0.21	0.21	0.21	0.21
16.600	0.20	0.20	0.20	0.20	0.20
16.850	0.19	0.19	0.19	0.19	0.19
17.100	0.18	0.18	0.18	0.18	0.18
17.350	0.18	0.17	0.17	0.17	0.17
17.600	0.17	0.16	0.16	0.16	0.16
17.850	0.16	0.15	0.15	0.15	0.15
18.100	0.15	0.14	0.14	0.14	0.14
18.350	0.14	0.14	0.14	0.13	0.13
18.600	0.13	0.13	0.13	0.13	0.13
18.850	0.13	0.13	0.13	0.13	0.13
19.100	0.13	0.13	0.13	0.13	0.12
19.350	0.12	0.12	0.12	0.12	0.12
19.600	0.12	0.12	0.12	0.12	0.12
19.850	0.12	0.12	0.12	0.12	0.12

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Return Event: 10 years Storm Event: 10

Scenario: Existing Conditions 10 Year Storm

## HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
20.100	0.12	0.12	0.11	0.11	0.11
20.350	0.11	0.11	0.11	0.11	0.11
20.600	0.11	0.11	0.11	0.11	0.11
20.850	0.11	0.11	0.11	0.11	0.11
21.100	0.11	0.11	0.11	0.10	0.10
21.350	0.10	0.10	0.10	0.10	0.10
21.600	0.10	0.10	0.10	0.10	0.10
21.850	0.10	0.10	0.10	0.10	0.10
22.100	0.10	0.10	0.10	0.10	0.10
22.350	0.10	0.09	0.09	0.09	0.09
22.600	0.09	0.09	0.09	0.09	0.09
22.850	0.09	0.09	0.09	0.09	0.09
23.100	0.09	0.09	0.09	0.09	0.09
23.350	0.09	0.09	0.08	0.08	0.08
23.600	0.08	0.08	0.08	0.08	0.08
23.850	0.08	0.08	0.08	0.08	(N/A)

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Scenario: Existing Conditions 100 Year Storm

-

Storm Event	100
Return Event	100 years
Duration	24.000 hours
Depth	9.2 in
Time of Concentration (Composite)	0.530 hours
Area (User Defined)	143,342.092 ft <sup>2</sup>

#### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft <sup>3</sup> /s)
9,800	0.00	0.00	0.00	0.01	0.01
10.050	0.01	0.02	0.02	0.03	0.04
10.300	0.04	0.05	0.06	0.07	0.08
10.550	0.09	0.10	0.11	0.13	0.14
10.800	0.15	0.16	0.18	0.19	0.21
11.050	0.23	0.24	0.26	0.28	0.30
11.300	0.33	0.36	0.39	0.42	0.46
11.550	0.50	0.55	0.62	0.72	0.84
11.800	1.00	1.22	1.51	1.87	2.39
12.050	3.07	3.87	4.81	5.73	6.55
12.300	7.21	7.51	7.62	7.53	7.21
12.550	6.77	6.27	5.74	5.21	4.71
12.800	4.23	3.82	3.45	3.13	2.85
13.050	2.62	2.42	2.25	2.10	1.97
13.300	1.86	1.76	1.68	1.61	1.55
13.550	1.49	1.45	1.40	1.37	1.33
13.800	1.30	1.27	1.24	1.22	1.19
14.050	1.17	1.15	1.12	1.10	1.08
14.300	1.06	1.04	1.03	1.01	1.00
14.550	0.98	0.97	0.96	0.95	0.93
14.800	0.92	0.91	0.90	0.89	0.88
15.050	0.87	0.86	0.85	0.83	0.82
15.300	0.81	0.80	0.79	0.78	0.77
15.550	0.76	0.74	0.73	0.72	0.71
15.800	0.70	0.69	0.68	0.66	0.65
16.050	0.64	0.63	0.62	0.61	0.60
16.300	0.59	0.58	0.57	0.56	0.55
16.550	0.55	0.54	0.54	0.53	0.52
16.800	0.52	0.51	0.51	0.50	0.50
17.050	0.49	0.49	0.48	0.48	0.47
17.300	0.47	0.46	0.46	0.45	0.44
17.550	0.44	0.43	0.43	0.42	0.42
17.800	0.41	0.41	0.40	0.40	0.39
18.050	0.39	0.38	0.38	0.37	0.37

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Return Event: 100 years Storm Event: 100

Scenario: Existing Conditions 100 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (bours)	Flow	Flow	Flow	Flow	Flow
(10013)	(1073)	(11/3)	(11/3)	(1173)	0.25
18.300	0.36	0.36	0.36	0.35	0.35
18.550	0.35	0.35	0.34	0.34	0.34
18.800	0.34	0.34	0.33	0.33	0.33
19.050	0.33	0.33	0.33	0.32	0.32
19.300	0.32	0.32	0.32	0.32	0.32
19.550	0.31	0.31	0.31	0.31	0.31
19.800	0.31	0.30	0.30	0.30	0.30
20.050	0.30	0.30	0.30	0.29	0.29
20.300	0.29	0.29	0.29	0.29	0.29
20.550	0.28	0.28	0.28	0.28	0.28
20.800	0.28	0.28	0.28	0.27	0.27
21.050	0.27	0.27	0.27	0.27	0.27
21.300	0.27	0.27	0.26	0.26	0.26
21.550	0.26	0.26	0.26	0.26	0.26
21.800	0.25	0.25	0.25	0.25	0.25
22.050	0.25	0.25	0.25	0.25	0.24
22.300	0.24	0.24	0.24	0.24	0.24
22.550	0.24	0.24	0.23	0.23	0.23
22.800	0.23	0.23	0.23	0.23	0.23
23.050	0.22	0.22	0.22	0.22	0.22
23.300	0.22	0.22	0.22	0.21	0.21
23.550	0.21	0.21	0.21	0.21	0.21
23.800	0.21	0.21	0.20	0.20	0.20

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

Scenario: Proposed Conditions 1 Year Storm

-

Storm Event	1
Return Event	1 years
Duration	24.000 hours
Depth	2.8 in
Time of Concentration (Composite)	0.083 hours
Area (User Defined)	100,517.148 ft <sup>2</sup>

## HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
7.000	0.00	0.00	0.00	0.00	0.00
7.250	0.00	0.00	0.01	0.01	0.01
7.500	0.01	0.01	0.01	0.01	0.01
7.750	0.01	0.01	0.01	0.02	0.02
8.000	0.02	0.02	0.02	0.02	0.02
8.250	0.02	0.02	0.03	0.03	0.03
8.500	0.03	0.03	0.03	0.04	0.04
8.750	0.04	0.04	0.04	0.05	0.05
9.000	0.05	0.05	0.06	0.06	0.06
9.250	0.06	0.06	0.07	0.07	0.07
9.500	0.08	0.08	0.08	0.08	0.09
9.750	0.09	0.09	0.10	0.10	0.10
10.000	0.10	0.11	0.11	0.12	0.12
10.250	0.13	0.13	0.14	0.14	0.15
10.500	0.15	0.16	0.16	0.17	0.18
10.750	0.18	0.19	0.19	0.20	0.21
11.000	0.21	0.22	0.24	0.25	0.27
11.250	0.29	0.31	0.33	0.35	0.37
11.500	0.39	0.47	0.56	0.72	0.89
11.750	1.09	1.30	1.52	1.75	2.54
12.000	3.55	3.91	4.10	3.44	2.50
12.250	2.12	1.86	1.63	1.41	1.18
12.500	0.95	0.80	0.66	0.61	0.58
12.750	0.56	0.53	0.51	0.48	0.46
13.000	0.43	0.42	0.40	0.39	0.38
13.250	0.38	0.37	0.37	0.36	0.35
13.500	0.35	0.34	0.33	0.33	0.32
13.750	0.32	0.31	0.30	0.30	0.29
14.000	0.28	0.28	0.27	0.27	0.27
14.250	0.26	0.26	0.26	0.25	0.25
14.500	0.25	0.24	0.24	0.24	0.24
14.750	0.23	0.23	0.23	0.22	0.22
15.000	0.22	0.21	0.21	0.21	0.20
15.250	0.20	0.20	0.19	0.19	0.19

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Return Event: 1 years Storm Event: 1

Scenario: Proposed Conditions 1 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
15.500	0.18	0.18	0.18	0.18	0.17
15.750	0.17	0.17	0.16	0.16	0.16
16.000	0.15	0.15	0.15	0.15	0.14
16.250	0.14	0.14	0.14	0.14	0.14
16.500	0.14	0.14	0.13	0.13	0.13
16.750	0.13	0.13	0.13	0.13	0.12
17.000	0.12	0.12	0.12	0.12	0.12
17.250	0.12	0.11	0.11	0.11	0.11
17.500	0.11	0.11	0.11	0.10	0.10
17.750	0.10	0.10	0.10	0.10	0.10
18.000	0.09	0.09	0.09	0.09	0.09
18.250	0.09	0.09	0.09	0.09	0.09
18.500	0.09	0.09	0.09	0.09	0.09
18.750	0.09	0.09	0.09	0.09	0.08
19.000	0.08	0.08	0.08	0.08	0.08
19.250	0.08	0.08	0.08	0.08	0.08
19.500	0.08	0.08	0.08	0.08	0.08
19.750	0.08	0.08	0.08	0.08	0.08
20.000	0.08	0.08	0.08	0.07	0.07
20.250	0.07	0.07	0.07	0.07	0.07
20.500	0.07	0.07	0.07	0.07	0.07
20.750	0.07	0.07	0.07	0.07	0.07
21.000	0.07	0.07	0.07	0.07	0.07
21.250	0.07	0.07	0.07	0.07	0.07
21.500	0.07	0.07	0.07	0.07	0.06
21.750	0.06	0.06	0.06	0.06	0.06
22.000	0.06	0.06	0.06	0.06	0.06
22.250	0.06	0.06	0.06	0.06	0.06
22.500	0.06	0.06	0.06	0.06	0.06
22.750	0.06	0.06	0.06	0.06	0.06
23.000	0.06	0.06	0.06	0.06	0.06
23.250	0.06	0.05	0.05	0.05	0.05
23.500	0.05	0.05	0.05	0.05	0.05
23.750	0.05	0.05	0.05	0.05	0.05
24.000	0.05	(N/A)	(N/A)	(N/A)	(N/A)

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Scenario: Proposed Conditions 10 Year Storm

-

Storm Event	10
Return Event	10 years
Duration	24.000 hours
Depth	5.1 in
Time of Concentration (Composite)	0.083 hours
Area (User Defined)	100,517.148 ft <sup>2</sup>

## HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft <sup>3</sup> /s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft <sup>3</sup> /s)
4.500	0.00	0.00	0.00	0.00	0.00
4.750	0.01	0.01	0.01	0.01	0.01
5.000	0.01	0.01	0.01	0.01	0.01
5.250	0.02	0.02	0.02	0.02	0.02
5.500	0.02	0.02	0.02	0.02	0.02
5.750	0.03	0.03	0.03	0.03	0.03
6.000	0.03	0.03	0.03	0.04	0.04
6.250	0.04	0.04	0.04	0.04	0.04
6.500	0.05	0.05	0.05	0.05	0.05
6.750	0.06	0.06	0.06	0.06	0.06
7.000	0.07	0.07	0.07	0.07	0.07
7.250	0.08	0.08	0.08	0.08	0.09
7.500	0.09	0.09	0.09	0.10	0.10
7.750	0.10	0.10	0.11	0.11	0.11
8.000	0.11	0.12	0.12	0.13	0.13
8.250	0.13	0.14	0.14	0.15	0.15
8.500	0.16	0.16	0.17	0.17	0.18
8.750	0.18	0.19	0.20	0.20	0.21
9.000	0.21	0.22	0.22	0.23	0.24
9.250	0.24	0.25	0.25	0.26	0.27
9.500	0.27	0.28	0.29	0.29	0.30
9.750	0.31	0.31	0.32	0.33	0.33
10.000	0.34	0.35	0.36	0.37	0.38
10.250	0.39	0.41	0.42	0.43	0.44
10.500	0.46	0.47	0.48	0.50	0.51
10.750	0.52	0.54	0.55	0.56	0.58
11.000	0.59	0.62	0.65	0.69	0.73
11.250	0.77	0.82	0.87	0.91	0.96
11.500	1.01	1.19	1.42	1.79	2.21
11.750	2.65	3.11	3.59	4.07	5.81
12.000	7.95	8.60	8.85	7.32	5.28
12.250	4.43	3.87	3.38	2.91	2.43
12.500	1.95	1.63	1.35	1.25	1.18
12.750	1.13	1.08	1.03	0.98	0.93

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Return Event: 10 years Storm Event: 10

Scenario: Proposed Conditions 10 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(nours)	(π <sup>3</sup> /S)	(π <sup>3</sup> /S)	(π <sup>3</sup> /S)	(π <sup>3</sup> /S)	(π³/s)
13.000	0.88	0.84	0.81	0.79	0.78
13.250	0.76	0.75	0.74	0.73	0.71
13.500	0.70	0.69	0.67	0.66	0.65
13./50	0.63	0.62	0.61	0.59	0.58
14.000	0.57	0.56	0.55	0.54	0.53
14.250	0.53	0.52	0.51	0.51	0.50
14.500	0.49	0.49	0.48	0.48	0.4/
14./50	0.46	0.46	0.45	0.44	0.44
15.000	0.43	0.43	0.42	0.41	0.41
15.250	0.40	0.39	0.39	0.38	0.3/
15.500	0.3/	0.36	0.35	0.35	0.34
15.750	0.34	0.33	0.32	0.32	0.31
16.000	0.30	0.30	0.29	0.29	0.29
16.250	0.28	0.28	0.28	0.28	0.27
16.500	0.27	0.27	0.26	0.26	0.26
16./50	0.26	0.25	0.25	0.25	0.24
17.000	0.24	0.24	0.24	0.23	0.23
17.250	0.23	0.23	0.22	0.22	0.22
17.500	0.21	0.21	0.21	0.21	0.20
17.750	0.20	0.20	0.19	0.19	0.19
18.000	0.19	0.18	0.18	0.18	0.18
18.250	0.18	0.18	0.18	0.18	0.18
18.500	0.1/	0.1/	0.1/	0.17	0.1/
18./50	0.1/	0.1/	0.1/	0.1/	0.1/
19.000	0.17	0.17	0.16	0.16	0.16
19.250	0.16	0.16	0.16	0.16	0.16
19.500	0.16	0.16	0.16	0.16	0.15
19./50	0.15	0.15	0.15	0.15	0.15
20.000	0.15	0.15	0.15	0.15	0.15
20.250	0.15	0.15	0.14	0.14	0.14
20.500	0.14	0.14	0.14	0.14	0.14
20.750	0.14	0.14	0.14	0.14	0.14
21.000	0.14	0.14	0.13	0.13	0.13
21.250	0.13	0.13	0.13	0.13	0.13
21.500	0.13	0.13	0.13	0.13	0.13
21./50	0.13	0.13	0.13	0.12	0.12
22.000	0.12	0.12	0.12	0.12	0.12
22.250	0.12	0.12	0.12	0.12	0.12
22.500	0.12	0.12	0.12	0.12	0.11
22./50	0.11	0.11	0.11	0.11	0.11
23.000	0.11	0.11	0.11	0.11	0.11
23.250	0.11	0.11	0.11	0.11	0.11
23.500	0.10	0.10	0.10	0.10	0.10

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Return Event: 10 years Storm Event: 10

Scenario: Proposed Conditions 10 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
23.750	0.10	0.10	0.10	0.10	0.10
24.000	0.10	(N/A)	(N/A)	(N/A)	(N/A)

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Scenario: Proposed Conditions 100 Year Storm

-

Storm Event	100
Return Event	100 years
Duration	24.000 hours
Depth	9.2 in
Time of Concentration (Composite)	0.083 hours
Area (User Defined)	100,517.148 ft <sup>2</sup>

### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(nours)	(113/5)	(113/5)	(113/5)	(113/5)	(113/5)
2./50	0.00	0.00	0.00	0.01	0.01
3.000	0.01	0.01	0.01	0.02	0.02
3.250	0.02	0.02	0.02	0.03	0.03
3.500	0.03	0.03	0.04	0.04	0.04
3.750	0.04	0.04	0.05	0.05	0.05
4.000	0.05	0.05	0.06	0.06	0.06
4.250	0.06	0.07	0.07	0.07	0.07
4.500	0.08	0.08	0.08	0.08	0.08
4.750	0.09	0.09	0.09	0.09	0.10
5.000	0.10	0.10	0.10	0.11	0.11
5.250	0.11	0.11	0.11	0.12	0.12
5.500	0.12	0.12	0.13	0.13	0.13
5.750	0.13	0.14	0.14	0.14	0.14
6.000	0.15	0.15	0.15	0.16	0.16
6.250	0.16	0.17	0.17	0.18	0.18
6.500	0.19	0.19	0.19	0.20	0.20
6.750	0.21	0.21	0.22	0.22	0.23
7.000	0.23	0.24	0.24	0.25	0.25
7.250	0.26	0.26	0.27	0.27	0.28
7.500	0.29	0.29	0.30	0.30	0.31
7.750	0.31	0.32	0.32	0.33	0.34
8.000	0.34	0.35	0.36	0.37	0.38
8.250	0.39	0.40	0.41	0.42	0.43
8.500	0.44	0.45	0.46	0.47	0.49
8.750	0.50	0.51	0.52	0.53	0.54
9.000	0.56	0.57	0.58	0.59	0.60
9.250	0.62	0.63	0.64	0.65	0.67
9.500	0.68	0.69	0.71	0.72	0.73
9.750	0.74	0.76	0.77	0.78	0.80
10.000	0.81	0.83	0.85	0.87	0.89
10.250	0.92	0.94	0.97	0.99	1.02
10.500	1.04	1.07	1.09	1.12	1.14
10.750	1.17	1.19	1.22	1.25	1.27
11.000	1.30	1.35	1.40	1.49	1.57

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Return Event: 100 years Storm Event: 100

Scenario: Proposed Conditions 100 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
11.250	1.66	1./6	1.85	1.94	2.03
11.500	2.13	2.50	2.96	3./1	4.56
11./50	5.43	6.32	7.23	8.14	11.50
12.000	15.5/	16.6/	17.02	13.99	10.03
12.250	8.39	/.31	6.38	5.4/	4.57
12.500	3.66	3.06	2.53	2.33	2.22
12.750	2.12	2.02	1.93	1.83	1.74
13.000	1.64	1.57	1.51	1.48	1.45
13.250	1.43	1.40	1.38	1.35	1.33
13.500	1.30	1.28	1.25	1.23	1.20
13.750	1.18	1.15	1.13	1.10	1.08
14.000	1.05	1.03	1.01	1.00	0.99
14.250	0.98	0.96	0.95	0.94	0.93
14.500	0.92	0.91	0.89	0.88	0.87
14.750	0.86	0.85	0.83	0.82	0.81
15.000	0.80	0.79	0.77	0.76	0.75
15.250	0.74	0.73	0.71	0.70	0.69
15.500	0.68	0.67	0.66	0.64	0.63
15.750	0.62	0.61	0.60	0.58	0.57
16.000	0.56	0.55	0.54	0.54	0.53
16.250	0.52	0.52	0.51	0.51	0.50
16.500	0.50	0.49	0.49	0.48	0.48
16.750	0.47	0.47	0.46	0.46	0.45
17.000	0.45	0.44	0.44	0.43	0.42
17.250	0.42	0.42	0.41	0.40	0.40
17.500	0.39	0.39	0.38	0.38	0.37
17.750	0.37	0.36	0.36	0.35	0.35
18.000	0.34	0.34	0.33	0.33	0.33
18.250	0.33	0.33	0.33	0.32	0.32
18.500	0.32	0.32	0.32	0.32	0.31
18.750	0.31	0.31	0.31	0.31	0.31
19.000	0.31	0.30	0.30	0.30	0.30
19.250	0.30	0.30	0.29	0.29	0.29
19.500	0.29	0.29	0.29	0.29	0.28
19.750	0.28	0.28	0.28	0.28	0.28
20.000	0.27	0.27	0.27	0.27	0.27
20.250	0.27	0.27	0.27	0.26	0.26
20.500	0.26	0.26	0.26	0.26	0.26
20.750	0.26	0.26	0.25	0.25	0.25
21.000	0.25	0.25	0.25	0.25	0.25
21.250	0.24	0.24	0.24	0.24	0.24
21.500	0.24	0.24	0.24	0.24	0.23
21.750	0.23	0.23	0.23	0.23	0.23

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Return Event: 100 years Storm Event: 100

Scenario: Proposed Conditions 100 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
22.000	0.23	0.23	0.22	0.22	0.22
22.250	0.22	0.22	0.22	0.22	0.22
22.500	0.21	0.21	0.21	0.21	0.21
22.750	0.21	0.21	0.21	0.21	0.20
23.000	0.20	0.20	0.20	0.20	0.20
23.250	0.20	0.20	0.20	0.20	0.19
23.500	0.19	0.19	0.19	0.19	0.19
23.750	0.19	0.18	0.18	0.18	0.18
24.000	0.18	(N/A)	(N/A)	(N/A)	(N/A)

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Scenario: Proposed Conditions 1 Year Storm

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Storm Event	1
Return Event	1 years
Duration	24.000 hours
Depth	2.8 in
Time of Concentration (Composite)	0.083 hours
Area (User Defined)	24,779.193 ft <sup>2</sup>

### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft <sup>3</sup> /s)	Flow (ft³/s)	Flow (ft <sup>3</sup> /s)	Flow (ft <sup>3</sup> /s)	Flow (ft <sup>3</sup> /s)
5.800	0.00	0.00	0.00	0.00	0.00
6.050	0.00	0.00	0.00	0.00	0.00
6.300	0.00	0.00	0.00	0.00	0.00
6.550	0.00	0.00	0.00	0.00	0.00
6.800	0.00	0.00	0.00	0.00	0.01
7.050	0.01	0.01	0.01	0.01	0.01
7.300	0.01	0.01	0.01	0.01	0.01
7.550	0.01	0.01	0.01	0.01	0.01
7.800	0.01	0.01	0.01	0.01	0.01
8.050	0.01	0.01	0.01	0.01	0.01
8.300	0.01	0.01	0.01	0.02	0.02
8.550	0.02	0.02	0.02	0.02	0.02
8.800	0.02	0.02	0.02	0.02	0.02
9.050	0.02	0.02	0.02	0.02	0.03
9.300	0.03	0.03	0.03	0.03	0.03
9.550	0.03	0.03	0.03	0.03	0.03
9.800	0.03	0.04	0.04	0.04	0.04
10.050	0.04	0.04	0.04	0.04	0.04
10.300	0.05	0.05	0.05	0.05	0.05
10.550	0.05	0.06	0.06	0.06	0.06
10.800	0.06	0.06	0.07	0.07	0.07
11.050	0.07	0.08	0.08	0.09	0.09
11.300	0.10	0.10	0.11	0.12	0.12
11.550	0.14	0.17	0.22	0.27	0.33
11.800	0.38	0.45	0.51	0.73	1.01
12.050	1.09	1.13	0.94	0.68	0.57
12.300	0.50	0.44	0.38	0.32	0.25
12.550	0.21	0.18	0.16	0.15	0.15
12.800	0.14	0.13	0.13	0.12	0.12
13.050	0.11	0.11	0.10	0.10	0.10
13.300	0.10	0.10	0.10	0.09	0.09
13.550	0.09	0.09	0.09	0.08	0.08
13.800	0.08	0.08	0.08	0.08	0.07
14.050	0.07	0.07	0.07	0.07	0.07

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Return Event: 1 years Storm Event: 1

Scenario: Proposed Conditions 1 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
14.300	0.07	0.07	0.07	0.07	0.07
14.550	0.06	0.06	0.06	0.06	0.06
14.800	0.06	0.06	0.06	0.06	0.06
15.050	0.06	0.06	0.05	0.05	0.05
15.300	0.05	0.05	0.05	0.05	0.05
15.550	0.05	0.05	0.05	0.04	0.04
15.800	0.04	0.04	0.04	0.04	0.04
16.050	0.04	0.04	0.04	0.04	0.04
16.300	0.04	0.04	0.04	0.04	0.04
16.550	0.04	0.03	0.03	0.03	0.03
16.800	0.03	0.03	0.03	0.03	0.03
17.050	0.03	0.03	0.03	0.03	0.03
17.300	0.03	0.03	0.03	0.03	0.03
17.550	0.03	0.03	0.03	0.03	0.03
17.800	0.03	0.03	0.03	0.02	0.02
18.050	0.02	0.02	0.02	0.02	0.02
18.300	0.02	0.02	0.02	0.02	0.02
18.550	0.02	0.02	0.02	0.02	0.02
18.800	0.02	0.02	0.02	0.02	0.02
19.050	0.02	0.02	0.02	0.02	0.02
19.300	0.02	0.02	0.02	0.02	0.02
19.550	0.02	0.02	0.02	0.02	0.02
19.800	0.02	0.02	0.02	0.02	0.02
20.050	0.02	0.02	0.02	0.02	0.02
20.300	0.02	0.02	0.02	0.02	0.02
20.550	0.02	0.02	0.02	0.02	0.02
20.800	0.02	0.02	0.02	0.02	0.02
21.050	0.02	0.02	0.02	0.02	0.02
21.300	0.02	0.02	0.02	0.02	0.02
21.550	0.02	0.02	0.02	0.02	0.02
21.800	0.02	0.02	0.02	0.02	0.02
22.050	0.02	0.02	0.02	0.02	0.02
22.300	0.02	0.02	0.02	0.02	0.02
22.550	0.02	0.02	0.02	0.02	0.02
22.800	0.01	0.01	0.01	0.01	0.01
23.050	0.01	0.01	0.01	0.01	0.01
23.300	0.01	0.01	0.01	0.01	0.01
23.550	0.01	0.01	0.01	0.01	0.01
23.800	0.01	0.01	0.01	0.01	0.01

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Scenario: Proposed Conditions 10 Year Storm

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Storm Event	10
Return Event	10 years
Duration	24.000 hours
Depth	5.1 in
Time of Concentration (Composite)	0.083 hours
Area (User Defined)	24,779.193 ft <sup>2</sup>

#### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
3.550	0.00	0.00	0.00	0.00	0.00
3.800	0.00	0.00	0.00	0.00	0.00
4.050	0.00	0.00	0.00	0.00	0.00
4.300	0.01	0.01	0.01	0.01	0.01
4.550	0.01	0.01	0.01	0.01	0.01
4.800	0.01	0.01	0.01	0.01	0.01
5.050	0.01	0.01	0.01	0.01	0.01
5.300	0.01	0.01	0.01	0.01	0.01
5.550	0.01	0.01	0.01	0.01	0.01
5.800	0.01	0.01	0.01	0.01	0.02
6.050	0.02	0.02	0.02	0.02	0.02
6.300	0.02	0.02	0.02	0.02	0.02
6.550	0.02	0.02	0.02	0.02	0.02
6.800	0.02	0.02	0.02	0.03	0.03
7.050	0.03	0.03	0.03	0.03	0.03
7.300	0.03	0.03	0.03	0.03	0.03
7.550	0.03	0.03	0.03	0.04	0.04
7.800	0.04	0.04	0.04	0.04	0.04
8.050	0.04	0.04	0.04	0.04	0.05
8.300	0.05	0.05	0.05	0.05	0.05
8.550	0.05	0.06	0.06	0.06	0.06
8.800	0.06	0.06	0.06	0.07	0.07
9.050	0.07	0.07	0.07	0.07	0.08
9.300	0.08	0.08	0.08	0.08	0.08
9.550	0.09	0.09	0.09	0.09	0.09
9.800	0.09	0.10	0.10	0.10	0.10
10.050	0.10	0.11	0.11	0.11	0.12
10.300	0.12	0.12	0.13	0.13	0.13
10.550	0.14	0.14	0.14	0.15	0.15
10.800	0.15	0.16	0.16	0.16	0.17
11.050	0.17	0.18	0.19	0.20	0.22
11.300	0.23	0.24	0.25	0.27	0.28
11.550	0.33	0.39	0.49	0.60	0.72
11.800	0.84	0.96	1.09	1.54	2.09

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Return Event: 10 years Storm Event: 10

Scenario: Proposed Conditions 10 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(nours)	(π <sup>3</sup> /S)	(π <sup>3</sup> /S)	(π³/s)	(π <sup>3</sup> /S)	(π <sup>3</sup> /S)
12.050	2.24	2.30	1.89	1.36	1.14
12.300	0.99	0.87	0.74	0.62	0.50
12.550	0.42	0.34	0.32	0.30	0.29
12.800	0.28	0.26	0.25	0.24	0.22
13.050	0.21	0.21	0.20	0.20	0.19
13.300	0.19	0.19	0.18	0.18	0.18
13.550	0.17	0.17	0.17	0.16	0.16
13.800	0.16	0.15	0.15	0.15	0.14
14.050	0.14	0.14	0.14	0.14	0.13
14.300	0.13	0.13	0.13	0.13	0.13
14.550	0.12	0.12	0.12	0.12	0.12
14.800	0.12	0.11	0.11	0.11	0.11
15.050	0.11	0.11	0.10	0.10	0.10
15.300	0.10	0.10	0.10	0.09	0.09
15.550	0.09	0.09	0.09	0.09	0.08
15.800	0.08	0.08	0.08	0.08	0.08
16.050	0.08	0.07	0.07	0.07	0.07
16.300	0.07	0.07	0.07	0.07	0.07
16.550	0.07	0.07	0.07	0.07	0.06
16.800	0.06	0.06	0.06	0.06	0.06
17.050	0.06	0.06	0.06	0.06	0.06
17.300	0.06	0.06	0.06	0.05	0.05
17.550	0.05	0.05	0.05	0.05	0.05
17.800	0.05	0.05	0.05	0.05	0.05
18.050	0.05	0.05	0.05	0.05	0.05
18.300	0.04	0.04	0.04	0.04	0.04
18.550	0.04	0.04	0.04	0.04	0.04
18.800	0.04	0.04	0.04	0.04	0.04
19.050	0.04	0.04	0.04	0.04	0.04
19.300	0.04	0.04	0.04	0.04	0.04
19.550	0.04	0.04	0.04	0.04	0.04
19.800	0.04	0.04	0.04	0.04	0.04
20.050	0.04	0.04	0.04	0.04	0.04
20.300	0.04	0.04	0.04	0.04	0.04
20.550	0.04	0.04	0.04	0.04	0.03
20.800	0.03	0.03	0.03	0.03	0.03
21.050	0.03	0.03	0.03	0.03	0.03
21.300	0.03	0.03	0.03	0.03	0.03
21.550	0.03	0.03	0.03	0.03	0.03
21.800	0.03	0.03	0.03	0.03	0.03
22.050	0.03	0.03	0.03	0.03	0.03
22.300	0.03	0.03	0.03	0.03	0.03
22.550	0.03	0.03	0.03	0.03	0.03

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Return Event: 10 years Storm Event: 10

Scenario: Proposed Conditions 10 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

	Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft <sup>3</sup> /s)
I	22.800	0.03	0.03	0.03	0.03	0.03
	23.050	0.03	0.03	0.03	0.03	0.03
	23.300	0.03	0.03	0.03	0.03	0.03
	23.550	0.03	0.03	0.03	0.03	0.03
	23.800	0.03	0.03	0.02	0.02	0.02

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Scenario: Proposed Conditions 100 Year Storm

Storm Event	100
Return Event	100 years
Duration	24.000 hours
Depth	9.2 in
Time of Concentration (Composite)	0.083 hours
Area (User Defined)	24,779.193 ft <sup>2</sup>

### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
2.050	0.00	0.00	0.00	0.00	0.00
2.300	0.00	0.00	0.00	0.01	0.01
2.550	0.01	0.01	0.01	0.01	0.01
2.800	0.01	0.01	0.01	0.01	0.01
3.050	0.01	0.01	0.01	0.01	0.01
3.300	0.02	0.02	0.02	0.02	0.02
3.550	0.02	0.02	0.02	0.02	0.02
3.800	0.02	0.02	0.02	0.02	0.02
4.050	0.02	0.03	0.03	0.03	0.03
4.300	0.03	0.03	0.03	0.03	0.03
4.550	0.03	0.03	0.03	0.03	0.03
4.800	0.03	0.03	0.04	0.04	0.04
5.050	0.04	0.04	0.04	0.04	0.04
5.300	0.04	0.04	0.04	0.04	0.04
5.550	0.04	0.04	0.04	0.04	0.05
5.800	0.05	0.05	0.05	0.05	0.05
6.050	0.05	0.05	0.05	0.05	0.05
6.300	0.05	0.06	0.06	0.06	0.06
6.550	0.06	0.06	0.06	0.06	0.07
6.800	0.07	0.07	0.07	0.07	0.07
7.050	0.07	0.08	0.08	0.08	0.08
7.300	0.08	0.08	0.08	0.08	0.09
7.550	0.09	0.09	0.09	0.09	0.09
7.800	0.09	0.10	0.10	0.10	0.10
8.050	0.10	0.10	0.11	0.11	0.11
8.300	0.12	0.12	0.12	0.12	0.13
8.550	0.13	0.13	0.14	0.14	0.14
8.800	0.14	0.15	0.15	0.15	0.16
9.050	0.16	0.16	0.17	0.17	0.17
9.300	0.18	0.18	0.18	0.18	0.19
9.550	0.19	0.19	0.20	0.20	0.20
9.800	0.21	0.21	0.21	0.22	0.22
10.050	0.22	0.23	0.23	0.24	0.25
10.300	0.25	0.26	0.27	0.27	0.28

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Return Event: 100 years Storm Event: 100

Scenario: Proposed Conditions 100 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
10.550	0.28	0.29	0.30	0.30	0.31
10.800	0.32	0.32	0.33	0.34	0.34
11.050	0.35	0.37	0.39	0.41	0.44
11.300	0.46	0.48	0.51	0.53	0.55
11.550	0.65	0.77	0.96	1.18	1.40
11.800	1.62	1.85	2.08	2.92	3.94
12.050	4.21	4.28	3.51	2.52	2.10
12.300	1.83	1.60	1.37	1.14	0.91
12.550	0.76	0.63	0.58	0.55	0.53
12.800	0.50	0.48	0.46	0.43	0.41
13.050	0.39	0.38	0.37	0.36	0.36
13.300	0.35	0.34	0.34	0.33	0.32
13.550	0.32	0.31	0.31	0.30	0.29
13.800	0.29	0.28	0.27	0.27	0.26
14.050	0.26	0.25	0.25	0.25	0.24
14.300	0.24	0.24	0.23	0.23	0.23
14.550	0.23	0.22	0.22	0.22	0.21
14.800	0.21	0.21	0.20	0.20	0.20
15.050	0.20	0.19	0.19	0.19	0.18
15.300	0.18	0.18	0.17	0.17	0.17
15.550	0.17	0.16	0.16	0.16	0.15
15.800	0.15	0.15	0.14	0.14	0.14
16.050	0.14	0.13	0.13	0.13	0.13
16.300	0.13	0.13	0.13	0.13	0.12
16.550	0.12	0.12	0.12	0.12	0.12
16.800	0.12	0.11	0.11	0.11	0.11
17.050	0.11	0.11	0.11	0.11	0.10
17.300	0.10	0.10	0.10	0.10	0.10
17.550	0.10	0.10	0.09	0.09	0.09
17.800	0.09	0.09	0.09	0.09	0.08
18.050	0.08	0.08	0.08	0.08	0.08
18.300	0.08	0.08	0.08	0.08	0.08
18.550	0.08	0.08	0.08	0.08	0.08
18.800	0.08	0.08	0.08	0.08	0.08
19.050	0.08	0.08	0.07	0.07	0.07
19.300	0.07	0.07	0.07	0.07	0.07
19.550	0.07	0.07	0.07	0.07	0.07
19.800	0.07	0.07	0.07	0.07	0.07
20.050	0.07	0.07	0.07	0.07	0.07
20.300	0.07	0.07	0.07	0.07	0.07
20.550	0.06	0.06	0.06	0.06	0.06
20.800	0.06	0.06	0.06	0.06	0.06
21.050	0.06	0.06	0.06	0.06	0.06

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Return Event: 100 years Storm Event: 100

Scenario: Proposed Conditions 100 Year Storm

### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
21.300	0.06	0.06	0.06	0.06	0.06
21.550	0.06	0.06	0.06	0.06	0.06
21.800	0.06	0.06	0.06	0.06	0.06
22.050	0.06	0.06	0.06	0.06	0.05
22.300	0.05	0.05	0.05	0.05	0.05
22.550	0.05	0.05	0.05	0.05	0.05
22.800	0.05	0.05	0.05	0.05	0.05
23.050	0.05	0.05	0.05	0.05	0.05
23.300	0.05	0.05	0.05	0.05	0.05
23.550	0.05	0.05	0.05	0.05	0.05
23.800	0.05	0.05	0.04	0.04	0.04

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Scenario: Proposed Conditions 1 Year Storm

-

Storm Event	1
Return Event	1 years
Duration	24.000 hours
Depth	2.8 in
Time of Concentration (Composite)	0.083 hours
Area (User Defined)	34,750.336 ft <sup>2</sup>

## HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow (ft3/c)	Flow (ft3/c)	Flow (ft3/c)	Flow (ft3/c)	Flow
(110015)	(119/5)	(119/5)	(119/5)	(113/5)	(119/5)
7.150	0.00	0.00	0.00	0.00	0.00
7.400	0.00	0.00	0.00	0.00	0.00
7.050	0.00	0.00	0.00	0.00	0.00
7.900	0.01	0.01	0.01	0.01	0.01
8.130	0.01	0.01	0.01	0.01	0.01
8.400	0.01	0.01	0.01	0.01	0.01
8.030	0.01	0.01	0.01	0.01	0.02
0.500	0.02	0.02	0.02	0.02	0.02
9.130	0.02	0.02	0.02	0.02	0.02
9.400	0.02	0.03	0.03	0.03	0.03
9,000	0.03	0.05	0.05	0.05	0.05
10 150	0.03	0.04	0.04	0.01	0.05
10,400	0.05	0.05	0.05	0.05	0.06
10.650	0.06	0.06	0.06	0.06	0.07
10.900	0.07	0.07	0.07	0.08	0.08
11.150	0.09	0.09	0.10	0.11	0.11
11.400	0.12	0.13	0.14	0.16	0.19
11.650	0.25	0.31	0.38	0.45	0.52
11.900	0.60	0.88	1.23	1.35	1.42
12.150	1.19	0.87	0.73	0.64	0.57
12.400	0.49	0.41	0.33	0.28	0.23
12.650	0.21	0.20	0.19	0.18	0.18
12.900	0.17	0.16	0.15	0.14	0.14
13.150	0.14	0.13	0.13	0.13	0.13
13.400	0.12	0.12	0.12	0.12	0.12
13.650	0.11	0.11	0.11	0.11	0.10
13.900	0.10	0.10	0.10	0.10	0.09
14.150	0.09	0.09	0.09	0.09	0.09
14.400	0.09	0.09	0.09	0.08	0.08
14.650	0.08	0.08	0.08	0.08	0.08
14.900	0.08	0.08	0.07	0.07	0.07
15.150	0.07	0.07	0.07	0.07	0.07
15.400	0.07	0.07	0.06	0.06	0.06

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Return Event: 1 years Storm Event: 1

Scenario: Proposed Conditions 1 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (bours)	Flow (ft3/c)	Flow	Flow	Flow	Flow
15 650	0.06	0.06	0.06	0.06	0.06
15,000	0.00	0.00	0.00	0.00	0.00
16 150	0.00	0.05	0.05	0.05	0.05
16 400	0.05	0.05	0.05	0.05	0.05
16.650	0.05	0.05	0.04	0.04	0.04
16.900	0.04	0.04	0.04	0.04	0.04
17.150	0.04	0.04	0.04	0.04	0.04
17.400	0.04	0.04	0.04	0.04	0.04
17.650	0.04	0.04	0.04	0.03	0.03
17.900	0.03	0.03	0.03	0.03	0.03
18.150	0.03	0.03	0.03	0.03	0.03
18.400	0.03	0.03	0.03	0.03	0.03
18.650	0.03	0.03	0.03	0.03	0.03
18.900	0.03	0.03	0.03	0.03	0.03
19.150	0.03	0.03	0.03	0.03	0.03
19.400	0.03	0.03	0.03	0.03	0.03
19.650	0.03	0.03	0.03	0.03	0.03
19.900	0.03	0.03	0.03	0.03	0.03
20.150	0.03	0.03	0.03	0.03	0.03
20.400	0.03	0.03	0.03	0.03	0.02
20.650	0.02	0.02	0.02	0.02	0.02
20.900	0.02	0.02	0.02	0.02	0.02
21.150	0.02	0.02	0.02	0.02	0.02
21.400	0.02	0.02	0.02	0.02	0.02
21.650	0.02	0.02	0.02	0.02	0.02
21.900	0.02	0.02	0.02	0.02	0.02
22.150	0.02	0.02	0.02	0.02	0.02
22.400	0.02	0.02	0.02	0.02	0.02
22.650	0.02	0.02	0.02	0.02	0.02
22.900	0.02	0.02	0.02	0.02	0.02
23.150	0.02	0.02	0.02	0.02	0.02
23.400	0.02	0.02	0.02	0.02	0.02
23.650	0.02	0.02	0.02	0.02	0.02
23.900	0.02	0.02	0.02	(N/A)	(N/A)

Scenario: Proposed Conditions 10 Year Storm

-

Storm Event	10
Return Event	10 years
Duration	24.000 hours
Depth	5.1 in
Time of Concentration (Composite)	0.083 hours
Area (User Defined)	34,750.336 ft <sup>2</sup>

## HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft <sup>3</sup> /s)	Flow (ft <sup>3</sup> /s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
4.600	0.00	0.00	0.00	0.00	0.00
4.850	0.00	0.00	0.00	0.00	0.00
5.100	0.00	0.00	0.00	0.01	0.01
5.350	0.01	0.01	0.01	0.01	0.01
5.600	0.01	0.01	0.01	0.01	0.01
5.850	0.01	0.01	0.01	0.01	0.01
6.100	0.01	0.01	0.01	0.01	0.01
6.350	0.01	0.01	0.02	0.02	0.02
6.600	0.02	0.02	0.02	0.02	0.02
6.850	0.02	0.02	0.02	0.02	0.02
7.100	0.02	0.03	0.03	0.03	0.03
7.350	0.03	0.03	0.03	0.03	0.03
7.600	0.03	0.03	0.03	0.04	0.04
7.850	0.04	0.04	0.04	0.04	0.04
8.100	0.04	0.04	0.04	0.05	0.05
8.350	0.05	0.05	0.05	0.05	0.06
8.600	0.06	0.06	0.06	0.06	0.07
8.850	0.07	0.07	0.07	0.07	0.08
9.100	0.08	0.08	0.08	0.08	0.09
9.350	0.09	0.09	0.09	0.09	0.10
9.600	0.10	0.10	0.10	0.11	0.11
9.850	0.11	0.11	0.12	0.12	0.12
10.100	0.12	0.13	0.13	0.14	0.14
10.350	0.14	0.15	0.15	0.16	0.16
10.600	0.17	0.17	0.18	0.18	0.19
10.850	0.19	0.20	0.20	0.20	0.21
11.100	0.22	0.24	0.25	0.27	0.28
11.350	0.30	0.32	0.33	0.35	0.41
11.600	0.49	0.62	0.76	0.92	1.08
11.850	1.24	1.41	2.01	2.75	2.97
12.100	3.06	2.53	1.82	1.53	1.34
12.350	1.17	1.01	0.84	0.67	0.56
12.600	0.47	0.43	0.41	0.39	0.37
12.850	0.36	0.34	0.32	0.30	0.29

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Return Event: 10 years Storm Event: 10

Scenario: Proposed Conditions 10 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
13,100	0.28	0.27	0.27	0.26	0.26
13.350	0.26	0.25	0.25	0.24	0.24
13.600	0.23	0.23	0.22	0.22	0.21
13.850	0.21	0.21	0.20	0.20	0.19
14,100	0.19	0.19	0.18	0.18	0.18
14.350	0.18	0.18	0.17	0.17	0.17
14.600	0.17	0.16	0.16	0.16	0.16
14.850	0.16	0.15	0.15	0.15	0.15
15.100	0.14	0.14	0.14	0.14	0.14
15.350	0.13	0.13	0.13	0.13	0.12
15.600	0.12	0.12	0.12	0.12	0.11
15.850	0.11	0.11	0.11	0.10	0.10
16.100	0.10	0.10	0.10	0.10	0.10
16.350	0.10	0.10	0.09	0.09	0.09
16.600	0.09	0.09	0.09	0.09	0.09
16.850	0.09	0.09	0.08	0.08	0.08
17.100	0.08	0.08	0.08	0.08	0.08
17.350	0.08	0.08	0.07	0.07	0.07
17.600	0.07	0.07	0.07	0.07	0.07
17.850	0.07	0.07	0.07	0.06	0.06
18.100	0.06	0.06	0.06	0.06	0.06
18.350	0.06	0.06	0.06	0.06	0.06
18.600	0.06	0.06	0.06	0.06	0.06
18.850	0.06	0.06	0.06	0.06	0.06
19.100	0.06	0.06	0.06	0.06	0.06
19.350	0.06	0.06	0.05	0.05	0.05
19.600	0.05	0.05	0.05	0.05	0.05
19.850	0.05	0.05	0.05	0.05	0.05
20.100	0.05	0.05	0.05	0.05	0.05
20.350	0.05	0.05	0.05	0.05	0.05
20.600	0.05	0.05	0.05	0.05	0.05
20.850	0.05	0.05	0.05	0.05	0.05
21.100	0.05	0.05	0.05	0.05	0.05
21.350	0.05	0.05	0.05	0.04	0.04
21.600	0.04	0.04	0.04	0.04	0.04
21.850	0.04	0.04	0.04	0.04	0.04
22.100	0.04	0.04	0.04	0.04	0.04
22.350	0.04	0.04	0.04	0.04	0.04
22.600	0.04	0.04	0.04	0.04	0.04
22.850	0.04	0.04	0.04	0.04	0.04
23.100	0.04	0.04	0.04	0.04	0.04
23.350	0.04	0.04	0.04	0.04	0.04
23.600	0.04	0.04	0.04	0.04	0.03

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Return Event: 10 years Storm Event: 10

Scenario: Proposed Conditions 10 Year Storm

### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
23.850	0.03	0.03	0.03	0.03	(N/A)

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Scenario: Proposed Conditions 100 Year Storm

Storm Event	100
Return Event	100 years
Duration	24.000 hours
Depth	9.2 in
Time of Concentration (Composite)	0.083 hours
Area (User Defined)	34,750.336 ft <sup>2</sup>

#### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/S)	(ft³/S)	(ft³/S)	(ft³/S)	(ft³/S)
2.800	0.00	0.00	0.00	0.00	0.00
3.050	0.00	0.00	0.01	0.01	0.01
3.300	0.01	0.01	0.01	0.01	0.01
3.550	0.01	0.01	0.01	0.01	0.01
3.800	0.02	0.02	0.02	0.02	0.02
4.050	0.02	0.02	0.02	0.02	0.02
4.300	0.02	0.02	0.02	0.03	0.03
4.550	0.03	0.03	0.03	0.03	0.03
4.800	0.03	0.03	0.03	0.03	0.03
5.050	0.03	0.04	0.04	0.04	0.04
5.300	0.04	0.04	0.04	0.04	0.04
5.550	0.04	0.04	0.04	0.05	0.05
5.800	0.05	0.05	0.05	0.05	0.05
6.050	0.05	0.05	0.05	0.06	0.06
6.300	0.06	0.06	0.06	0.06	0.06
6.550	0.07	0.07	0.07	0.07	0.07
6.800	0.07	0.08	0.08	0.08	0.08
7.050	0.08	0.08	0.09	0.09	0.09
7.300	0.09	0.09	0.09	0.10	0.10
7.550	0.10	0.10	0.10	0.11	0.11
7.800	0.11	0.11	0.11	0.12	0.12
8.050	0.12	0.12	0.13	0.13	0.13
8.300	0.14	0.14	0.14	0.15	0.15
8.550	0.16	0.16	0.16	0.17	0.17
8.800	0.18	0.18	0.18	0.19	0.19
9.050	0.20	0.20	0.20	0.21	0.21
9.300	0.22	0.22	0.23	0.23	0.24
9.550	0.24	0.24	0.25	0.25	0.26
9.800	0.26	0.27	0.27	0.28	0.28
10.050	0.29	0.29	0.30	0.31	0.32
10.300	0.33	0.33	0.34	0.35	0.36
10.550	0.37	0.38	0.39	0.39	0.40
10.800	0.41	0.42	0.43	0.44	0.45
11.050	0.47	0.49	0.51	0.54	0.58

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Return Event: 100 years Storm Event: 100

Scenario: Proposed Conditions 100 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(nours)	(113/5)	(113/5)	(113/5)	(113/5)	(113/5)
11.300	0.61	0.64	0.67	0.70	0.74
11.550	0.87	1.02	1.28	1.58	1.00
11.800	2.19	2.50	2.82	3.98	5.38
12.050	5./6	5.88	4.84	3.4/	2.90
12.300	2.53	2.20	1.89	1.58	1.20
12.550	1.00	0.88	0.81	0.77	0.73
12.800	0.70	0.67	0.63	0.60	0.57
13.050	0.54	0.52	0.51	0.50	0.49
13.300	0.40	0.40	0.47	0.40	0.45
13.550	0.44	0.43	0.42	0.42	0.41
13.000	0.40	0.39	0.30	0.37	0.30
14.050	0.30	0.35	0.35	0.34	0.34
14.500	0.33	0.33	0.33	0.32	0.32
14.550	0.31	0.31	0.30	0.30	0.30
14.000	0.29	0.29	0.20	0.20	0.20
15 300	0.27	0.27	0.20	0.20	0.20
15,500	0.23	0.23	0.24	0.24	0.25
15,550	0.25	0.25	0.22	0.22	0.21
16.050	0.21	0.21	0.20	0.20	0.19
16 300	0.15	0.15	0.10	0.10	0.10
16 550	0.10	0.10	0.10	0.17	0.17
16,800	0.17	0.17	0.17	0.17	0.10
17 050	0.10	0.10	0.10	0.10	0.15
17.000	0.15	0.13	0.15	0.15	0.15
17,500	0.11	0.11	0.13	0.11	0.11
17.550	0.13	0.13	0.13	0.13	0.13
18 050	0.13	0.12	0.12	0.12	0.12
18,300	0.11	0.11	0.11	0.11	0.11
18,550	0.11	0.11	0.11	0.11	0.11
18.800	0.11	0.11	0.11	0.11	0.11
19.050	0.11	0.10	0.10	0.10	0.10
19,300	0.10	0.10	0.10	0.10	0.10
19.550	0.10	0.10	0.10	0.10	0.10
19.800	0.10	0.10	0.10	0.10	0.09
20.050	0.09	0.09	0.09	0.09	0.09
20.300	0.09	0.09	0.09	0.09	0.09
20.550	0.09	0.09	0.09	0.09	0.09
20.800	0.09	0.09	0.09	0.09	0.09
21.050	0.09	0.09	0.09	0.08	0.08
21.300	0.08	0.08	0.08	0.08	0.08
21.550	0.08	0.08	0.08	0.08	0.08
21.800	0.08	0.08	0.08	0.08	0.08

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Return Event: 100 years Storm Event: 100

Scenario: Proposed Conditions 100 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
22.050	0.08	0.08	0.08	0.08	0.08
22.300	0.08	0.08	0.08	0.07	0.07
22.550	0.07	0.07	0.07	0.07	0.07
22.800	0.07	0.07	0.07	0.07	0.07
23.050	0.07	0.07	0.07	0.07	0.07
23.300	0.07	0.07	0.07	0.07	0.07
23.550	0.07	0.07	0.07	0.06	0.06
23.800	0.06	0.06	0.06	0.06	0.06

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Scenario: Proposed Conditions 1 Year Storm

-

Storm Event	1
Return Event	1 years
Duration	24.000 hours
Depth	2.8 in
Time of Concentration (Composite)	0.181 hours
Area (User Defined)	199,180.163 ft <sup>2</sup>

## HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft <sup>3</sup> /s)	Flow (ft <sup>3</sup> /s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft <sup>3</sup> /s)
8.200	0.00	0.00	0.00	0.00	0.01
8.450	0.01	0.01	0.01	0.02	0.02
8.700	0.02	0.02	0.03	0.03	0.03
8.950	0.03	0.04	0.04	0.04	0.05
9.200	0.05	0.05	0.06	0.06	0.07
9.450	0.07	0.07	0.08	0.08	0.09
9.700	0.09	0.10	0.10	0.10	0.11
9.950	0.11	0.12	0.12	0.13	0.14
10.200	0.14	0.15	0.16	0.17	0.17
10.450	0.18	0.19	0.20	0.21	0.22
10.700	0.23	0.24	0.25	0.26	0.27
10.950	0.28	0.29	0.30	0.32	0.34
11.200	0.36	0.38	0.41	0.44	0.48
11.450	0.51	0.55	0.60	0.69	0.83
11.700	1.04	1.29	1.59	1.92	2.28
11.950	2.84	3.92	5.13	6.04	6.37
12.200	5.88	5.02	4.27	3.71	3.23
12.450	2.80	2.37	1.99	1.65	1.40
12.700	1.24	1.13	1.06	1.00	0.95
12.950	0.90	0.86	0.82	0.78	0.75
13.200	0.73	0.71	0.70	0.69	0.67
13.450	0.66	0.65	0.64	0.63	0.62
13.700	0.61	0.60	0.58	0.57	0.56
13.950	0.55	0.54	0.53	0.52	0.51
14.200	0.50	0.49	0.49	0.48	0.48
14.450	0.47	0.46	0.46	0.45	0.45
14.700	0.44	0.44	0.43	0.43	0.42
14.950	0.41	0.41	0.40	0.40	0.39
15.200	0.39	0.38	0.37	0.37	0.36
15.450	0.36	0.35	0.34	0.34	0.33
15.700	0.33	0.32	0.32	0.31	0.30
15.950	0.30	0.29	0.29	0.28	0.28
16.200	0.27	0.27	0.27	0.26	0.26
16.450	0.26	0.26	0.25	0.25	0.25

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Return Event: 1 years Storm Event: 1

Scenario: Proposed Conditions 1 Year Storm

# HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
16.700	0.25	0.24	0.24	0.24	0.24
16.950	0.23	0.23	0.23	0.23	0.22
17.200	0.22	0.22	0.22	0.21	0.21
17.450	0.21	0.21	0.20	0.20	0.20
17.700	0.19	0.19	0.19	0.19	0.18
17.950	0.18	0.18	0.18	0.17	0.17
18.200	0.17	0.17	0.17	0.17	0.17
18.450	0.17	0.17	0.17	0.16	0.16
18.700	0.16	0.16	0.16	0.16	0.16
18.950	0.16	0.16	0.16	0.16	0.16
19.200	0.16	0.15	0.15	0.15	0.15
19.450	0.15	0.15	0.15	0.15	0.15
19.700	0.15	0.15	0.15	0.15	0.14
19.950	0.14	0.14	0.14	0.14	0.14
20.200	0.14	0.14	0.14	0.14	0.14
20.450	0.14	0.14	0.14	0.14	0.13
20.700	0.13	0.13	0.13	0.13	0.13
20.950	0.13	0.13	0.13	0.13	0.13
21.200	0.13	0.13	0.13	0.13	0.13
21.450	0.13	0.12	0.12	0.12	0.12
21.700	0.12	0.12	0.12	0.12	0.12
21.950	0.12	0.12	0.12	0.12	0.12
22.200	0.12	0.12	0.11	0.11	0.11
22.450	0.11	0.11	0.11	0.11	0.11
22.700	0.11	0.11	0.11	0.11	0.11
22.950	0.11	0.11	0.11	0.11	0.10
23.200	0.10	0.10	0.10	0.10	0.10
23.450	0.10	0.10	0.10	0.10	0.10
23.700	0.10	0.10	0.10	0.10	0.10
23.950	0.09	0.09	(N/A)	(N/A)	(N/A)

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Scenario: Proposed Conditions 10 Year Storm

-

Storm Event	10		
Return Event	10 years		
Duration	24.000 hours		
Depth	5.1 in		
Time of Concentration (Composite)	0.181 hours		
Area (User Defined)	199,180.163 ft <sup>2</sup>		

### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft <sup>3</sup> /s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
5.600	0.00	0.00	0.00	0.01	0.01
5.850	0.01	0.01	0.01	0.01	0.02
6.100	0.02	0.02	0.02	0.02	0.03
6.350	0.03	0.03	0.03	0.04	0.04
6.600	0.04	0.04	0.05	0.05	0.05
6.850	0.06	0.06	0.06	0.06	0.07
7.100	0.07	0.07	0.08	0.08	0.09
7.350	0.09	0.09	0.10	0.10	0.10
7.600	0.11	0.11	0.12	0.12	0.13
7.850	0.13	0.13	0.14	0.14	0.15
8.100	0.15	0.16	0.16	0.17	0.18
8.350	0.19	0.19	0.20	0.21	0.22
8.600	0.23	0.23	0.24	0.25	0.26
8.850	0.27	0.28	0.29	0.30	0.31
9.100	0.32	0.33	0.34	0.35	0.36
9.350	0.37	0.38	0.39	0.40	0.41
9.600	0.43	0.44	0.45	0.46	0.47
9.850	0.49	0.50	0.51	0.52	0.54
10.100	0.55	0.57	0.59	0.61	0.63
10.350	0.65	0.67	0.69	0.72	0.74
10.600	0.76	0.79	0.81	0.84	0.86
10.850	0.89	0.91	0.94	0.96	0.99
11.100	1.03	1.08	1.14	1.21	1.29
11.350	1.37	1.45	1.53	1.62	1.76
11.600	2.01	2.38	2.93	3.58	4.34
11.850	5.13	6.00	7.29	9.79	12.50
12.100	14.39	14.88	13.53	11.40	9.59
12.350	8.26	7.14	6.15	5.18	4.31
12.600	3.57	3.02	2.66	2.43	2.27
12.850	2.14	2.03	1.93	1.83	1.74
13.100	1.66	1.59	1.55	1.51	1.48
13.350	1.45	1.43	1.40	1.38	1.35
13.600	1.33	1.30	1.28	1.25	1.23
13.850	1.20	1.18	1.15	1.13	1.10

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Return Event: 10 years Storm Event: 10

Scenario: Proposed Conditions 10 Year Storm

#### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
14.100	1.08	1.06	1.04	1.03	1.02
14.350	1.00	0.99	0.98	0.97	0.96
14.600	0.94	0.93	0.92	0.91	0.90
14.850	0.88	0.87	0.86	0.85	0.84
15.100	0.82	0.81	0.80	0.79	0.77
15.350	0.76	0.75	0.74	0.73	0.71
15.600	0.70	0.69	0.68	0.66	0.65
15.850	0.64	0.63	0.61	0.60	0.59
16.100	0.58	0.57	0.56	0.56	0.55
16.350	0.54	0.54	0.53	0.53	0.52
16.600	0.52	0.51	0.51	0.50	0.50
16.850	0.49	0.48	0.48	0.47	0.47
17.100	0.46	0.46	0.45	0.45	0.44
17.350	0.44	0.43	0.43	0.42	0.42
17.600	0.41	0.40	0.40	0.39	0.39
17.850	0.38	0.38	0.37	0.37	0.36
18.100	0.36	0.35	0.35	0.35	0.35
18.350	0.34	0.34	0.34	0.34	0.34
18.600	0.34	0.33	0.33	0.33	0.33
18.850	0.33	0.33	0.32	0.32	0.32
19.100	0.32	0.32	0.32	0.31	0.31
19.350	0.31	0.31	0.31	0.31	0.30
19.600	0.30	0.30	0.30	0.30	0.30
19.850	0.30	0.29	0.29	0.29	0.29
20.100	0.29	0.29	0.28	0.28	0.28
20.350	0.28	0.28	0.28	0.28	0.28
20.600	0.27	0.27	0.27	0.27	0.27
20.850	0.27	0.27	0.27	0.27	0.26
21.100	0.26	0.26	0.26	0.26	0.26
21.350	0.26	0.26	0.25	0.25	0.25
21.600	0.25	0.25	0.25	0.25	0.25
21.850	0.24	0.24	0.24	0.24	0.24
22.100	0.24	0.24	0.24	0.23	0.23
22.350	0.23	0.23	0.23	0.23	0.23
22.600	0.23	0.23	0.22	0.22	0.22
22.850	0.22	0.22	0.22	0.22	0.21
23.100	0.21	0.21	0.21	0.21	0.21
23.350	0.21	0.21	0.21	0.20	0.20
23.600	0.20	0.20	0.20	0.20	0.20
23.850	0.20	0.19	0.19	0.19	(N/A)

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

Scenario: Proposed Conditions 100 Year Storm

-

Storm Event	100
Return Event	100 years
Duration	24.000 hours
Depth	9.2 in
Time of Concentration (Composite)	0.181 hours
Area (User Defined)	199,180.163 ft <sup>2</sup>

#### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time (hours)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)	Flow (ft³/s)
3.500	0.00	0.00	0.00	0.01	0.01
3.750	0.01	0.02	0.02	0.02	0.03
4.000	0.03	0.04	0.04	0.04	0.05
4.250	0.05	0.06	0.06	0.06	0.07
4,500	0.07	0.07	0.08	0.08	0.09
4.750	0.09	0.09	0.10	0.10	0.11
5.000	0.11	0.12	0.12	0.12	0.13
5.250	0.13	0.14	0.14	0.14	0.15
5.500	0.15	0.16	0.16	0.17	0.17
5.750	0.18	0.18	0.18	0.19	0.19
6.000	0.20	0.20	0.21	0.21	0.22
6.250	0.22	0.23	0.24	0.25	0.25
6.500	0.26	0.27	0.28	0.28	0.29
6.750	0.30	0.31	0.32	0.33	0.33
7.000	0.34	0.35	0.36	0.37	0.38
7.250	0.39	0.40	0.41	0.42	0.43
7.500	0.44	0.45	0.46	0.47	0.48
7.750	0.49	0.50	0.51	0.52	0.53
8.000	0.54	0.55	0.56	0.58	0.59
8.250	0.61	0.63	0.65	0.66	0.68
8.500	0.70	0.72	0.74	0.76	0.78
8.750	0.81	0.83	0.85	0.87	0.89
9.000	0.91	0.94	0.96	0.98	1.00
9.250	1.03	1.05	1.07	1.10	1.12
9.500	1.15	1.17	1.20	1.22	1.24
9.750	1.27	1.30	1.32	1.35	1.37
10.000	1.40	1.42	1.45	1.49	1.53
10.250	1.57	1.62	1.66	1.71	1.75
10.500	1.80	1.85	1.90	1.95	2.00
10.750	2.05	2.10	2.15	2.20	2.25
11.000	2.30	2.36	2.44	2.54	2.67
11.250	2.82	2.99	3.15	3.33	3.50
11.500	3.69	3.97	4.50	5.31	6.48
11.750	7.85	9.42	11.03	12.76	15.33

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Return Event: 100 years Storm Event: 100

Scenario: Proposed Conditions 100 Year Storm

#### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
12.000	20.31	25.59	29.11	29.81	26.88
12.250	22.51	18.81	16.13	13.88	11.91
12.500	10.00	8.31	6.88	5.81	5.10
12.750	4.67	4.35	4.10	3.88	3.69
13.000	3.49	3.32	3.16	3.04	2.95
13.250	2.88	2.82	2.77	2.71	2.67
13.500	2.62	2.57	2.52	2.47	2.42
13.750	2.38	2.33	2.28	2.23	2.18
14.000	2.13	2.09	2.05	2.01	1.98
14.250	1.95	1.92	1.90	1.88	1.85
14.500	1.83	1.81	1.78	1.76	1.74
14.750	1.71	1.69	1.67	1.65	1.62
15.000	1.60	1.58	1.55	1.53	1.51
15.250	1.48	1.46	1.44	1.41	1.39
15.500	1.37	1.34	1.32	1.30	1.27
15.750	1.25	1.22	1.20	1.18	1.15
16.000	1.13	1.11	1.09	1.07	1.06
16.250	1.04	1.03	1.02	1.01	1.00
16.500	0.99	0.98	0.97	0.96	0.95
16.750	0.94	0.93	0.92	0.91	0.90
17.000	0.89	0.88	0.87	0.86	0.85
17.250	0.84	0.83	0.82	0.81	0.80
17.500	0.79	0.78	0.77	0.76	0.75
17.750	0.74	0.73	0.72	0.71	0.70
18.000	0.69	0.68	0.67	0.66	0.65
18.250	0.65	0.65	0.64	0.64	0.64
18.500	0.63	0.63	0.63	0.63	0.62
18.750	0.62	0.62	0.61	0.61	0.61
19.000	0.60	0.60	0.60	0.59	0.59
19.250	0.59	0.59	0.58	0.58	0.58
19.500	0.57	0.57	0.57	0.56	0.56
19.750	0.56	0.56	0.55	0.55	0.55
20.000	0.54	0.54	0.54	0.53	0.53
20.250	0.53	0.53	0.52	0.52	0.52
20.500	0.52	0.52	0.51	0.51	0.51
20.750	0.51	0.50	0.50	0.50	0.50
21.000	0.49	0.49	0.49	0.49	0.49
21.250	0.48	0.48	0.48	0.48	0.47
21.500	0.47	0.47	0.47	0.47	0.46
21.750	0.46	0.46	0.46	0.45	0.45
22.000	0.45	0.45	0.44	0.44	0.44
22.250	0.44	0.43	0.43	0.43	0.43
22.500	0.43	0.42	0.42	0.42	0.42

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center

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Return Event: 100 years Storm Event: 100

Scenario: Proposed Conditions 100 Year Storm

#### HYDROGRAPH ORDINATES (ft<sup>3</sup>/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row.

Time	Flow	Flow	Flow	Flow	Flow
(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
22.750	0.41	0.41	0.41	0.41	0.41
23.000	0.40	0.40	0.40	0.39	0.39
23.250	0.39	0.39	0.39	0.38	0.38
23.500	0.38	0.38	0.37	0.37	0.37
23.750	0.37	0.37	0.36	0.36	0.36
24.000	0.35	(N/A)	(N/A)	(N/A)	(N/A)

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 101 of 448

#### Subsection: Addition Summary Label: DP Scenario: Existing Conditions 1 Year Storm

# Summary for Hydrograph Addition at 'DP'

Upstream Link	Upstream Node	
<catchment node="" outflow="" to=""></catchment>	EDA-1A	
<catchment node="" outflow="" to=""></catchment>	EDA-1B	

#### **Node Inflows**

Inflow Type	Element	Volume (ft³)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA-1A	29,639.596	12.150	7.71
Flow (From)	EDA-1B	1,983.144	12.750	0.12
Flow (In)	DP	31,622.741	12.150	7.71

Return Event: 1 years Storm Event: 1

#### Subsection: Addition Summary Label: DP

Scenario: Proposed Conditions 1 Year Storm

#### Summary for Hydrograph Addition at 'DP'

Upstream Link	Upstream Node
	Porous Pavement
	INFILTRATION BASIN B
<catchment node="" outflow="" to=""></catchment>	PDA-1D
<catchment node="" outflow="" to=""></catchment>	PDA-1C

#### **Node Inflows**

Inflow Type	Element	Volume (ft³)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)		0.000	0.000	0.00
Flow (From)		0.000	0.000	0.00
Flow (From)	PDA-1D	24,859.696	12.150	6.37
Flow (From)	PDA-1C	5,002.433	12.100	1.42
Flow (In)	DP	29,862.128	12.150	7.56

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 103 of 448

#### Subsection: Addition Summary Label: DP Scenario: Existing Conditions 10 Year Storm

#### Summary for Hydrograph Addition at 'DP'

Upstream Link	Up	stream Node
<catchment node="" outflow="" to=""></catchment>	EDA-1A	
<catchment node="" outflow="" to=""></catchment>	EDA-1B	

#### **Node Inflows**

Inflow Type	Element	Volume (ft³)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA-1A	67,958.090	12.150	17.11
Flow (From)	EDA-1B	13,069.387	12.450	1.96
Flow (In)	DP	81,027.477	12.150	17.98

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#### Subsection: Addition Summary Label: DP

Scenario: Proposed Conditions 10 Year Storm

#### Summary for Hydrograph Addition at 'DP'

Upstream Link	Upstream Node
	Porous Pavement
	INFILTRATION BASIN B
<catchment node="" outflow="" to=""></catchment>	PDA-1D
<catchment node="" outflow="" to=""></catchment>	PDA-1C

#### **Node Inflows**

Inflow Type	Element	Volume (ft³)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)		0.000	0.000	0.00
Flow (From)		0.000	0.000	0.00
Flow (From)	PDA-1D	59,299.250	12.150	14.88
Flow (From)	PDA-1C	11,247.526	12.100	3.06
Flow (In)	DP	70,546.776	12.100	17.45

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Return Event: 10 years Storm Event: 10

#### Subsection: Addition Summary Label: DP Scenario: Existing Conditions 100 Year Storm

### Summary for Hydrograph Addition at 'DP'

Upstream Link		Upstream Node
<catchment node="" outflow="" to=""></catchment>	EDA-1A	
<catchment node="" outflow="" to=""></catchment>	EDA-1B	

#### **Node Inflows**

Inflow Type	Element	Volume (ft³)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)	EDA-1A	138,254.510	12.150	33.35
Flow (From)	EDA-1B	44,148.422	12.400	7.62
Flow (In)	DP	182,402.932	12.150	38.16

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### Subsection: Addition Summary Label: DP

Scenario: Proposed Conditions 100 Year Storm

#### Summary for Hydrograph Addition at 'DP'

Upstream Link	Upstream Node
	Porous Pavement
	INFILTRATION BASIN B
<catchment node="" outflow="" to=""></catchment>	PDA-1D
<catchment node="" outflow="" to=""></catchment>	PDA-1C

#### **Node Inflows**

Inflow Type	Element	Volume (ft³)	Time to Peak (hours)	Flow (Peak) (ft³/s)
Flow (From)		0.000	0.000	0.00
Flow (From)		15,230.238	12.350	5.70
Flow (From)	PDA-1D	123,455.282	12.150	29.81
Flow (From)	PDA-1C	22,623.260	12.100	5.88
Flow (In)	DP	161,308.780	12.150	37.93

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# Subsection: Elevation-Area Volume Curve Label: INFILTRATION BASIN A

Scenario: Proposed Conditions 1 Year Storm

Elevation (ft)	Planimeter (ft <sup>2</sup> )	Area (ft²)	A1+A2+sqr (A1*A2) (ft²)	Volume (ft³)	Volume (Total) (ft³)
366.90	0.0	2,087.059	0.000	0.000	0.000
368.00	0.0	2,750.508	7,233.496	2,652.000	2,652.000
370.00	0.0	4,132.080	10,253.837	6,836.000	9,488.000
371.00	0.0	4,907.689	13,542.987	4,514.000	14,003.000

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# Subsection: Elevation-Area Volume Curve Label: INFILTRATION BASIN A

#### Scenario: Proposed Conditions 10 Year Storm

Elevation (ft)	Planimeter (ft <sup>2</sup> )	Area (ft²)	A1+A2+sqr (A1*A2) (ft²)	Volume (ft³)	Volume (Total) (ft³)
366.90	0.0	2,087.059	0.000	0.000	0.000
368.00	0.0	2,750.508	7,233.496	2,652.000	2,652.000
370.00	0.0	4,132.080	10,253.837	6,836.000	9,488.000
371.00	0.0	4,907.689	13,542.987	4,514.000	14,003.000

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# Subsection: Elevation-Area Volume Curve Label: INFILTRATION BASIN A

Scenario: Proposed Conditions 100 Year Storm

Elevation (ft)	Planimeter (ft²)	Area (ft²)	A1+A2+sqr (A1*A2) (ft²)	Volume (ft³)	Volume (Total) (ft³)
366.90	0.0	2,087.059	0.000	0.000	0.000
368.00	0.0	2,750.508	7,233.496	2,652.000	2,652.000
370.00	0.0	4,132.080	10,253.837	6,836.000	9,488.000
371.00	0.0	4,907.689	13,542.987	4,514.000	14,003.000

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# Subsection: Elevation-Area Volume Curve Label: INFILTRATION BASIN B

Scenario: Proposed Conditions 1 Year Storm

Elevation (ft)	Planimeter (ft <sup>2</sup> )	Area (ft²)	A1+A2+sqr (A1*A2) (ft²)	Volume (ft³)	Volume (Total) (ft³)
366.50	0.0	1,290.220	0.000	0.000	0.000
368.00	0.0	2,127.297	5,074.225	2,537.000	2,537.000
370.00	0.0	3,469.428	8,313.433	5,542.000	8,079.000
371.00	0.0	4,225.317	11,523.509	3,841.000	11,921.000

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# Subsection: Elevation-Area Volume Curve Label: INFILTRATION BASIN B

Scenario: Proposed Conditions 10 Year Storm

Elevation (ft)	Planimeter (ft²)	Area (ft²)	A1+A2+sqr (A1*A2) (ft²)	Volume (ft³)	Volume (Total) (ft³)
366.50	0.0	1,290.220	0.000	0.000	0.000
368.00	0.0	2,127.297	5,074.225	2,537.000	2,537.000
370.00	0.0	3,469.428	8,313.433	5,542.000	8,079.000
371.00	0.0	4,225.317	11,523.509	3,841.000	11,921.000

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# Subsection: Elevation-Area Volume Curve Label: INFILTRATION BASIN B

Scenario: Proposed Conditions 100 Year Storm

Elevation (ft)	Planimeter (ft²)	Area (ft²)	A1+A2+sqr (A1*A2) (ft²)	Volume (ft³)	Volume (Total) (ft³)
366.50	0.0	1,290.220	0.000	0.000	0.000
368.00	0.0	2,127.297	5,074.225	2,537.000	2,537.000
370.00	0.0	3,469.428	8,313.433	5,542.000	8,079.000
371.00	0.0	4,225.317	11,523.509	3,841.000	11,921.000

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# Subsection: Elevation-Area Volume Curve Label: Porous Pavement

#### Return Event: 1 years Storm Event: 1

# Scenario: Proposed Conditions 1 Year StormElevationPlanimeterAreaA1+A2+sqrVolumeVolume (Total)(ft)(ft2)(ft2)(ft3)(ft3)

(ft)	(ft <sup>2</sup> )	(ft <sup>2</sup> )	(A1*A2) (ft²)	(ft <sup>3</sup> )	(ft <sup>3</sup> )
366.71	0.0	15,097.870	0.000	0.000	0.000
368.21	0.0	15,097.870	45,293.609	22,647.000	9,059.000

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#### Volume Complete Filled With Material (Adjust Volumes for Voids)

# Void Space = 40.0 % Elevation (Headwater) (ft) Volume (Total) (ft<sup>3</sup>) Volume (Adjusted) (ft<sup>3</sup>) 366.71 0.000 0.000 368.21 22,646.804 9,058.722

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Return Event: 1 years Storm Event: 1

# Subsection: Elevation-Area Volume Curve Label: Porous Pavement

#### Return Event: 10 years Storm Event: 10

Scenario:	Proposed	Conditions 10	) Year Storm
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Elevation (ft)	Planimeter (ft²)	Area (ft²)	A1+A2+sqr (A1*A2) (ft²)	Volume (ft³)	Volume (Total) (ft³)
366.71	0.0	15,097.870	0.000	0.000	0.000
368.21	0.0	15,097.870	45,293.609	22,647.000	9,059.000

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 116 of 448 Subsection: Volume Void Adjustments Label: Porous Pavement Scenario: Proposed Conditions 10 Year Storm

# Volume Complete Filled With Material (Adjust Volumes for Voids)

# Void Space = 40.0 % Elevation (Headwater) (ft) Volume (Total) (ft<sup>3</sup>) Volume (Adjusted) (ft<sup>3</sup>) 366.71 0.000 0.000 368.21 22,646.804 9,058.722

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Return Event: 10 years Storm Event: 10

# Subsection: Elevation-Area Volume Curve Label: Porous Pavement

#### Return Event: 100 years Storm Event: 100

#### Scenario: Proposed Conditions 100 Year Storm

Elevation	Planimeter	Area	A1+A2+sqr	Volume	Volume (Total)
(ft)	(ft²)	(ft²)	(A1*A2)	(ft³)	(ft³)
			(ft²)		
366.71	0.0	15,097.870	0.000	0.000	0.000
368.21	0.0	15,097.870	45,293.609	22,647.000	9,059.000

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#### Volume Complete Filled With Material (Adjust Volumes for Voids)

# Void Space = 40.0 % Elevation (Headwater) (ft) Volume (Total) (ft<sup>3</sup>) Volume (Adjusted) (ft<sup>3</sup>) 366.71 0.000 0.000 368.21 22,646.804 9,058.722

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Return Event: 1 yea	rs
Storm Event:	1

Requested Pond Water Surface Elevations		
Minimum (Headwater)	366.90 ft	
Increment (Headwater)	0.50 ft	
Maximum (Headwater)	371.00 ft	

#### **Outlet Connectivity**

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Culvert-Circular	Culvert - 1	Forward + Reverse	TW	366.90	371.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

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Return Event: 1 years Storm Event: 1

Structure ID: Culvert - 1 Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	24.0 in
Length	120.00 ft
Length (Computed Barrel)	120.00 ft
Slope (Computed)	0.003 ft/ft
Outlet Control Data	
Manning's n	0.013
Ке	0.200
Kb	0.012
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
К	0.0045
Μ	2.0000
C	0.0317
Υ	0.6900
T1 ratio (HW/D)	1.094
T2 ratio (HW/D)	1.196
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control,

interpolate between flows at T1 & T2...

T1 Elevation	369.09 ft	T1 Flow	15.55 ft³/s
T2 Elevation	369.29 ft	T2 Flow	17.77 ft³/s

#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	0.00	366.50	0.00
367.00	0.04	366.50	0.00
367.10	0.17	366.50	0.00
367.20	0.39	366.50	0.00
367.30	0.68	366.50	0.00
367.40	1.05	366.50	0.00
367.50	1.50	366.50	0.00
367.60	2.01	366.50	0.00
367.70	2.59	366.50	0.00
367.80	3.24	366.50	0.00
367.90	3.93	366.50	0.00
368.00	4.70	366.50	0.00
368.10	5.50	366.50	0.00
368.20	6.34	366.50	0.00
368.30	7.21	366.50	0.00
368.40	8.14	366.50	0.00
368.50	9.06	366.50	0.00
368.60	10.00	366.50	0.00
368.70	10.95	366.50	0.00
368.80	11.90	366.50	0.00
368.90	12.82	366.50	0.00
369.00	13.75	366.50	0.00
369.10	14.62	366.50	0.00
369.20	15.45	366.50	0.00
369.30	16.24	366.50	0.00
369.40	16.93	366.50	0.00
369.50	17.51	366.50	0.00
369.60	18.03	366.50	0.00
369.70	18.54	366.50	0.00
369.80	19.06	366.50	0.00
369.90	19.57	366.50	0.00
370.00	20.07	366.50	0.00
370.10	20.58	366.50	0.00
370.20	21.07	366.50	0.00
370.30	21.56	366.50	0.00
370.40	22.04	366.50	0.00
370.50	22.51	366.50	0.00
370.60	22.99	366.50	0.00
370.70	23.46	366.50	0.00
370.80	23.91	366.50	0.00
370.90	24.36	366.50	0.00
371.00	24.81	366.50	0.00

Contributing Structures

None Contributing

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EDA-PDA.ppc 1/22/2021

# Composite Outflow Summary

Contributing Structures

jj
Culvert - 1
Culvert - I
Culvert - 1

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 1 years Storm Event: 1

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#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	0.00	366.60	0.00
367.00	0.04	366.60	0.00
367.10	0.17	366.60	0.00
367.20	0.39	366.60	0.00
367.30	0.68	366.60	0.00
367.40	1.05	366.60	0.00
367.50	1.50	366.60	0.00
367.60	2.01	366.60	0.00
367.70	2.59	366.60	0.00
367.80	3.24	366.60	0.00
367.90	3.93	366.60	0.00
368.00	4.70	366.60	0.00
368.10	5.50	366.60	0.00
368.20	6.34	366.60	0.00
368.30	7.21	366.60	0.00
368.40	8.14	366.60	0.00
368.50	9.06	366.60	0.00
368.60	10.00	366.60	0.00
368.70	10.95	366.60	0.00
368.80	11.90	366.60	0.00
368.90	12.82	366.60	0.00
369.00	13.75	366.60	0.00
369.10	14.62	366.60	0.00
369.20	15.45	366.60	0.00
369.30	16.24	366.60	0.00
369.40	16.93	366.60	0.00
369.50	17.51	366.60	0.00
369.60	18.03	366.60	0.00
369.70	18.54	366.60	0.00
369.80	19.06	366.60	0.00
369.90	19.57	366.60	0.00
370.00	20.07	366.60	0.00
370.10	20.58	366.60	0.00
370.20	21.07	366.60	0.00
370.30	21.56	366.60	0.00
370.40	22.04	366.60	0.00
370.50	22.51	366.60	0.00
370.60	22.99	366.60	0.00
370.70	23.46	366.60	0.00
370.80	23.91	366.60	0.00
370.90	24.36	366.60	0.00
371.00	24.81	366.60	0.00

Contributing Structures

None Contributing

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# Composite Outflow Summary

Contributing Structures

Culvert - 1
Culvert - 1
Cuivert - 1
Culvert - I
Culvert - 1
Culvert - 1

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#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	0.00	366.70	0.00
367.00	0.04	366.70	0.00
367.10	0.17	366.70	0.00
367.20	0.39	366.70	0.00
367.30	0.68	366.70	0.00
367.40	1.05	366.70	0.00
367.50	1.50	366.70	0.00
367.60	2.01	366.70	0.00
367.70	2.59	366.70	0.00
367.80	3.24	366.70	0.00
367.90	3.93	366.70	0.00
368.00	4.70	366.70	0.00
368.10	5.50	366.70	0.00
368.20	6.34	366.70	0.00
368.30	7.21	366.70	0.00
368.40	8.14	366.70	0.00
368.50	9.06	366.70	0.00
368.60	10.00	366.70	0.00
368.70	10.95	366.70	0.00
368.80	11.90	366.70	0.00
368.90	12.82	366.70	0.00
369.00	13.75	366.70	0.00
369.10	14.62	366.70	0.00
369.20	15.45	366.70	0.00
369.30	16.24	366.70	0.00
369.40	16.93	366.70	0.00
369.50	17.51	366.70	0.00
369.60	18.03	366.70	0.00
369.70	18.54	366.70	0.00
369.80	19.06	366.70	0.00
369.90	19.57	366.70	0.00
370.00	20.07	366.70	0.00
370.10	20.58	366.70	0.00
370.20	21.07	366.70	0.00
370.30	21.56	366.70	0.00
370.40	22.04	366.70	0.00
370.50	22.51	366.70	0.00
370.60	22.99	366.70	0.00
370.70	23.46	366.70	0.00
370.80	23.91	366.70	0.00
370.90	24.36	366.70	0.00
371.00	24.81	366.70	0.00

Contributing Structures

None Contributing

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# Composite Outflow Summary

Contributing Structures

	<b>J</b>
Culvert - 1	
cuivert - 1	

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#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	0.00	366.80	0.00
367.00	0.04	366.80	0.00
367.10	0.17	366.80	0.00
367.20	0.39	366.80	0.00
367.30	0.68	366.80	0.00
367.40	1.05	366.80	0.00
367.50	1.50	366.80	0.00
367.60	2.01	366.80	0.00
367.70	2.59	366.80	0.00
367.80	3.24	366.80	0.00
367.90	3.93	366.80	0.00
368.00	4.70	366.80	0.00
368.10	5.50	366.80	0.00
368.20	6.34	366.80	0.00
368.30	7.21	366.80	0.00
368.40	8.14	366.80	0.00
368.50	9.06	366.80	0.00
368.60	10.00	366.80	0.00
368.70	10.95	366.80	0.00
368.80	11.90	366.80	0.00
368.90	12.82	366.80	0.00
369.00	13.75	366.80	0.00
369.10	14.62	366.80	0.00
369.20	15.45	366.80	0.00
369.30	16.24	366.80	0.00
369.40	16.93	366.80	0.00
369.50	17.51	366.80	0.00
369.60	18.03	366.80	0.00
369.70	18.54	366.80	0.00
369.80	19.06	366.80	0.00
369.90	19.57	366.80	0.00
370.00	20.07	366.80	0.00
370.10	20.58	366.80	0.00
370.20	21.07	366.80	0.00
370.30	21.56	366.80	0.00
370.40	22.04	366.80	0.00
370.50	22.51	366.80	0.00
370.60	22.99	366.80	0.00
370.70	23.46	366.80	0.00
370.80	23.91	366.80	0.00
370.90	24.36	366.80	0.00
371.00	24.81	366.80	0.00

Contributing Structures

None Contributing

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EDA-PDA.ppc 1/22/2021

#### Composite Outflow Summary

Contributing Structures

Culvert - 1
Culvert - 1
Cuivert - 1
Culvert 1
Culvert - 1
Culvert 1
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Culvert - 1
Culvert 1
Culvert - 1

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#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	0.00	366.90	0.00
367.00	0.04	366.90	0.00
367.10	0.17	366.90	0.00
367.20	0.38	366.90	0.00
367.30	0.68	366.90	0.00
367.40	1.06	366.90	0.00
367.50	1.50	366.90	0.00
367.60	2.01	366.90	0.00
367.70	2.59	366.90	0.00
367.80	3.24	366.90	0.00
367.90	3.93	366.90	0.00
368.00	4.70	366.90	0.00
368.10	5.50	366.90	0.00
368.20	6.34	366.90	0.00
368.30	7.21	366.90	0.00
368.40	8.14	366.90	0.00
368.50	9.06	366.90	0.00
368.60	10.00	366.90	0.00
368.70	10.95	366.90	0.00
368.80	11.90	366.90	0.00
368.90	12.82	366.90	0.00
369.00	13.75	366.90	0.00
369.10	14.62	366.90	0.00
369.20	15.45	366.90	0.00
369.30	16.24	366.90	0.00
369.40	16.93	366.90	0.00
369.50	17.51	366.90	0.00
369.60	18.03	366.90	0.00
369.70	18.54	366.90	0.00
369.80	19.06	366.90	0.00
369.90	19.57	366.90	0.00
370.00	20.07	366.90	0.00
370.10	20.58	366.90	0.00
370.20	21.07	366.90	0.00
370.30	21.56	366.90	0.00
370.40	22.04	366.90	0.00
370.50	22.51	366.90	0.00
370.60	22.99	366.90	0.00
370.70	23.46	366.90	0.00
370.80	23.91	366.90	0.00
370.90	24.36	366.90	0.00
371.00	24.81	366.90	0.00

Contributing Structures

None Contributing

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# Composite Outflow Summary

Contributing Structures

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EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 1 years Storm Event: 1

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#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-0.03	367.00	0.00
367.00	0.00	367.00	0.00
367.10	0.17	367.00	0.00
367.20	0.39	367.00	0.00
367.30	0.68	367.00	0.00
367.40	1.05	367.00	0.00
367.50	1.50	367.00	0.00
367.60	2.02	367.00	0.00
367.70	2.59	367.00	0.00
367.80	3.24	367.00	0.00
367.90	3.93	367.00	0.00
368.00	4.70	367.00	0.00
368.10	5.50	367.00	0.00
368.20	6.34	367.00	0.00
368.30	7.21	367.00	0.00
368.40	8.14	367.00	0.00
368.50	9.06	367.00	0.00
368.60	10.00	367.00	0.00
368.70	10.95	367.00	0.00
368.80	11.90	367.00	0.00
368.90	12.82	367.00	0.00
369.00	13.75	367.00	0.00
369.10	14.62	367.00	0.00
369.20	15.45	367.00	0.00
369.30	16.24	367.00	0.00
369.40	16.93	367.00	0.00
369.50	17.51	367.00	0.00
369.60	18.03	367.00	0.00
369.70	18.54	367.00	0.00
369.80	19.06	367.00	0.00
369.90	19.57	367.00	0.00
370.00	20.07	367.00	0.00
370.10	20.58	367.00	0.00
370.20	21.07	367.00	0.00
370.30	21.56	367.00	0.00
370.40	22.04	367.00	0.00
370.50	22.51	367.00	0.00
370.60	22.99	367.00	0.00
370.70	23.46	367.00	0.00
370.80	23.91	367.00	0.00
370.90	24.36	367.00	0.00
371.00	24.81	367.00	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)		- 1	
366.90	-0.14	367.10	0.00
367.00	-0.14	367.10	0.00
367.10	0.00	367.10	0.00
367.20	0.38	367.10	0.00
367.30	0.68	367.10	0.00
367.40	1.05	367.10	0.00
367.50	1.50	367.10	0.00
367.60	2.01	367.10	0.00
367.70	2.59	367.10	0.00
367.80	3.24	367.10	0.00
367.90	3.93	367.10	0.00
368.00	4.70	367.10	0.00
368.10	5.50	367.10	0.00
368.20	6.34	367.10	0.00
368.30	7.21	367.10	0.00
368.40	8.14	367.10	0.00
368.50	9.06	367.10	0.00
368.60	10.00	367.10	0.00
368.70	10.95	367.10	0.00
368.80	11.90	367.10	0.00
368.90	12.82	367.10	0.00
369.00	13.75	367.10	0.00
369.10	14.62	367.10	0.00
369.20	15.45	367.10	0.00
369.30	16.24	367.10	0.00
369.40	16.93	367.10	0.00
369.50	17.51	367.10	0.00
369.60	18.03	367.10	0.00
369.70	18.54	367.10	0.00
369.80	19.06	367.10	0.00
369.90	19.57	367.10	0.00
370.00	20.07	367.10	0.00
370.10	20.58	367.10	0.00
370.20	21.07	367.10	0.00
370.30	21.56	367.10	0.00
370.40	22.04	367.10	0.00
370.50	22.51	367.10	0.00
370.60	22.99	367.10	0.00
370.70	23.46	367.10	0.00
370.80	23.91	367.10	0.00
370.90	24.36	367.10	0.00
371.00	24.81	367.10	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

Contributing Structures

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-0.32	367.20	0.00
367.00	-0.32	367.20	0.00
367.10	-0.32	367.20	0.00
367.20	0.00	367.20	0.00
367.30	0.66	367.20	0.00
367.40	1.05	367.20	0.00
367.50	1.50	367.20	0.00
367.60	2.01	367.20	0.00
367.70	2.59	367.20	0.00
367.80	3.24	367.20	0.00
367.90	3.95	367.20	0.00
368.00	4.70	367.20	0.00
368.10	5.50	367.20	0.00
368.20	6.34	367.20	0.00
368.30	7.21	367.20	0.00
368.40	8.14	367.20	0.00
368.50	9.06	367.20	0.00
368.60	10.00	367.20	0.00
368.70	10.95	367.20	0.00
368.80	11.90	367.20	0.00
368.90	12.82	367.20	0.00
369.00	13.75	367.20	0.00
369.10	14.62	367.20	0.00
369.20	15.45	367.20	0.00
369.30	16.24	367.20	0.00
369.40	16.93	367.20	0.00
369.50	17.51	367.20	0.00
369.60	18.03	367.20	0.00
369.70	18.54	367.20	0.00
369.80	19.06	367.20	0.00
369.90	19.57	367.20	0.00
370.00	20.07	367.20	0.00
370.10	20.58	367.20	0.00
370.20	21.07	367.20	0.00
370.30	21.56	367.20	0.00
370.40	22.04	367.20	0.00
370.50	22.51	367.20	0.00
370.60	22.99	367.20	0.00
370.70	23.46	367.20	0.00
370.80	23.91	367.20	0.00
370.90	24.36	367.20	0.00
371.00	24.81	367.20	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)	T		
366.90	-0.58	367.30	0.00
367.00	-0.58	367.30	0.00
367.10	-0.58	367.30	0.00
367.20	-0.56	367.30	0.00
367.30	0.00	367.30	0.00
367.40	0.98	367.30	0.00
367.50	1.49	367.30	0.00
367.60	2.01	367.30	0.00
367.70	2.59	367.30	0.00
367.80	3.24	367.30	0.00
367.90	3.94	367.30	0.00
368.00	4.68	367.30	0.00
368.10	5.50	367.30	0.00
368.20	6.34	367.30	0.00
368.30	7.21	367.30	0.00
368.40	8.14	367.30	0.00
368.50	9.06	367.30	0.00
368.60	10.00	367.30	0.00
368.70	10.95	367.30	0.00
368.80	11.90	367.30	0.00
368.90	12.82	367.30	0.00
369.00	13./5	367.30	0.00
369.10	14.62	367.30	0.00
369.20	15.45	367.30	0.00
369.30	16.24	367.30	0.00
369.40	16.93	367.30	0.00
369.50	17.51	367.30	0.00
369.60	18.03	367.30	0.00
369.70	18.54	367.30	0.00
369.80	19.06	367.30	0.00
369.90	19.57	367.30	0.00
3/0.00	20.07	367.30	0.00
3/0.10	20.58	367.30	0.00
370.20	21.07	367.30	0.00
3/0.30	21.56	367.30	0.00
3/0.40	22.04	367.30	0.00
3/0.50	22.51	367.30	0.00
3/0.60	22.99	367.30	0.00
3/0./0	23.46	367.30	0.00
3/0.80	23.91	367.30	0.00
3/0.90	24.36	367.30	0.00
3/1.00	24.81	367.30	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-0.90	367.40	0.00
367.00	-0.90	367.40	0.00
367.10	-0.90	367.40	0.00
367.20	-0.90	367.40	0.00
367.30	-0.83	367.40	0.00
367.40	0.00	367.40	0.00
367.50	1.33	367.40	0.00
367.60	1.97	367.40	0.00
367.70	2.58	367.40	0.00
367.80	3.24	367.40	0.00
367.90	3.94	367.40	0.00
368.00	4.69	367.40	0.00
368.10	5.50	367.40	0.00
368.20	6.34	367.40	0.00
368.30	7.21	367.40	0.00
368.40	8.14	367.40	0.00
368.50	9.06	367.40	0.00
368.60	10.00	367.40	0.00
368.70	10.95	367.40	0.00
368.80	11.90	367.40	0.00
368.90	12.82	367.40	0.00
369.00	13.75	367.40	0.00
369.10	14.62	367.40	0.00
369.20	15.45	367.40	0.00
369.30	16.24	367.40	0.00
369.40	16.93	367.40	0.00
369.50	17.51	367.40	0.00
369.60	18.03	367.40	0.00
369.70	18.54	367.40	0.00
369.80	19.06	367.40	0.00
369.90	19.57	367.40	0.00
370.00	20.07	367.40	0.00
370.10	20.58	367.40	0.00
370.20	21.07	367.40	0.00
370.30	21.56	367.40	0.00
370.40	22.04	367.40	0.00
370.50	22.51	367.40	0.00
370.60	22.99	367.40	0.00
370.70	23.46	367.40	0.00
370.80	23.91	367.40	0.00
370.90	24.36	367.40	0.00
371.00	24.81	367.40	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation (ff)	(ft³/S)	(π)	(π)
	-1 20	367 50	0.00
367.00	-1 29	367.50	0.00
367.00	_1 20	367.50	0.00
367.20	-1 29	367.50	0.00
367.20	-1 29	367.50	0.00
367.40	-1 14	367.50	0.00
367 50	0.00	367.50	0.00
367.50	1 71	367.50	0.00
367.00	2 50	367 50	0.00
367.80	3 21	367 50	0.00
367.90	3.93	367.50	0.00
368.00	4 68	367.50	0.00
368.10	5.50	367.50	0.00
368.20	6.34	367.50	0.00
368.30	7.21	367.50	0.00
368.40	8.14	367.50	0.00
368.50	9.06	367.50	0.00
368.60	10.00	367.50	0.00
368.70	10.95	367.50	0.00
368.80	11.90	367.50	0.00
368.90	12.82	367.50	0.00
369.00	13.75	367.50	0.00
369.10	14.62	367.50	0.00
369.20	15.45	367.50	0.00
369.30	16.24	367.50	0.00
369.40	16.93	367.50	0.00
369.50	17.51	367.50	0.00
369.60	18.03	367.50	0.00
369.70	18.54	367.50	0.00
369.80	19.06	367.50	0.00
369.90	19.57	367.50	0.00
370.00	20.07	367.50	0.00
370.10	20.58	367.50	0.00
370.20	21.07	367.50	0.00
370.30	21.56	367.50	0.00
370.40	22.04	367.50	0.00
370.50	22.51	367.50	0.00
370.60	22.99	367.50	0.00
370.70	23.46	367.50	0.00
370.80	23.91	367.50	0.00
370.90	24.36	367.50	0.00
371.00	24.81	367.50	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)		T	-
366.90	-1.74	367.60	0.00
367.00	-1.74	367.60	0.00
367.10	-1.74	367.60	0.00
367.20	-1.74	367.60	0.00
367.30	-1.74	367.60	0.00
367.40	-1.73	367.60	0.00
367.50	-1.48	367.60	0.00
367.60	0.00	367.60	0.00
367.70	2.09	367.60	0.00
367.80	3.05	367.60	0.00
367.90	3.86	367.60	0.00
368.00	4.66	367.60	0.00
368.10	5.47	367.60	0.00
368.20	6.34	367.60	0.00
368.30	7.21	367.60	0.00
368.40	8.12	367.60	0.00
368.50	9.06	367.60	0.00
368.60	10.00	367.60	0.00
368.70	10.95	367.60	0.00
368.80	11.90	367.60	0.00
368.90	12.82	367.60	0.00
369.00	13.75	367.60	0.00
369.10	14.62	367.60	0.00
369.20	15.45	367.60	0.00
369.30	16.24	367.60	0.00
369.40	16.93	367.60	0.00
369.50	17.51	367.60	0.00
369.60	18.03	367.60	0.00
369.70	18.54	367.60	0.00
369.80	19.06	367.60	0.00
369.90	19.57	367.60	0.00
370.00	20.07	367.60	0.00
370.10	20.58	367.60	0.00
370.20	21.07	367.60	0.00
370.30	21.56	367.60	0.00
370.40	22.04	367.60	0.00
370.50	22.51	367.60	0.00
370.60	22.99	367.60	0.00
370.70	23.46	367.60	0.00
370.80	23.91	367.60	0.00
370.90	24.36	367.60	0.00
371.00	24.81	367.60	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-2.25	367.70	0.00
367.00	-2.25	367.70	0.00
367.10	-2.25	367.70	0.00
367.20	-2.25	367.70	0.00
367.30	-2.25	367.70	0.00
367.40	-2.25	367.70	0.00
367.50	-2.19	367.70	0.00
367.60	-1.81	367.70	0.00
367.70	0.00	367.70	0.00
367.80	2.48	367.70	0.00
367.90	3.59	367.70	0.00
368.00	4.54	367.70	0.00
368.10	5.43	367.70	0.00
368.20	6.30	367.70	0.00
368.30	7.20	367.70	0.00
368.40	8.12	367.70	0.00
368.50	9.04	367.70	0.00
368.60	9.99	367.70	0.00
368.70	10.95	367.70	0.00
368.80	11.90	367.70	0.00
368.90	12.82	367.70	0.00
369.00	13.75	367.70	0.00
369.10	14.62	367.70	0.00
369.20	15.45	367.70	0.00
369.30	16.24	367.70	0.00
369.40	16.93	367.70	0.00
369.50	17.51	367.70	0.00
369.60	18.03	367.70	0.00
369.70	18.54	367.70	0.00
369.80	19.06	367.70	0.00
369.90	19.57	367.70	0.00
370.00	20.07	367.70	0.00
370.10	20.58	367.70	0.00
370.20	21.07	367.70	0.00
370.30	21.56	367.70	0.00
370.40	22.04	367.70	0.00
370.50	22.51	367.70	0.00
370.60	22.99	367.70	0.00
370.70	23.46	367.70	0.00
370.80	23.91	367.70	0.00
370.90	24.36	367.70	0.00
371.00	24.81	367.70	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

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## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-2.81	367.80	0.00
367.00	-2.81	367.80	0.00
367.10	-2.81	367.80	0.00
367.20	-2.81	367.80	0.00
367.30	-2.81	367.80	0.00
367.40	-2.81	367.80	0.00
367.50	-2.81	367.80	0.00
367.60	-2.68	367.80	0.00
367.70	-2.17	367.80	0.00
367.80	0.00	367.80	0.00
367.90	2.88	367.80	0.00
368.00	4.17	367.80	0.00
368.10	5.22	367.80	0.00
368.20	6.18	367.80	0.00
368.30	7.12	367.80	0.00
368.40	8.07	367.80	0.00
368.50	9.02	367.80	0.00
368.60	9.99	367.80	0.00
368.70	10.93	367.80	0.00
368.80	11.90	367.80	0.00
368.90	12.82	367.80	0.00
369.00	13.75	367.80	0.00
369.10	14.62	367.80	0.00
369.20	15.45	367.80	0.00
369.30	16.24	367.80	0.00
369.40	16.93	367.80	0.00
369.50	17.51	367.80	0.00
369.60	18.03	367.80	0.00
369.70	18.54	367.80	0.00
369.80	19.06	367.80	0.00
369.90	19.57	367.80	0.00
370.00	20.07	367.80	0.00
370.10	20.58	367.80	0.00
370.20	21.07	367.80	0.00
370.30	21.56	367.80	0.00
370.40	22.04	367.80	0.00
370.50	22.51	367.80	0.00
370.60	22.99	367.80	0.00
370.70	23.46	367.80	0.00
370.80	23.91	367.80	0.00
370.90	24.36	367.80	0.00
371.00	24.81	367.80	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation (ft)	(ft³/S)	(π)	(π)
366.90	-3.43	367.90	0.00
367.00	-3 43	367.90	0.00
367.00	-3.43	367.90	0.00
367.20	-3 43	367.90	0.00
367.20	-3 43	367.90	0.00
367.40	-3 43	367.90	0.00
367 50	-3 43	367.90	0.00
367.60	-3 42	367.90	0.00
367.00	-3.12	367.90	0.00
367.80	-2 53	367.90	0.00
367.90	0.00	367.90	0.00
368.00	3.25	367.90	0.00
368.10	4.71	367.90	0.00
368.20	5.86	367.90	0.00
368.30	6.93	367.90	0.00
368.40	7.93	367.90	0.00
368.50	8.93	367.90	0.00
368.60	9.91	367.90	0.00
368.70	10.90	367.90	0.00
368.80	11.86	367.90	0.00
368.90	12.81	367.90	0.00
369.00	13.75	367.90	0.00
369.10	14.62	367.90	0.00
369.20	15.45	367.90	0.00
369.30	16.24	367.90	0.00
369.40	16.93	367.90	0.00
369.50	17.51	367.90	0.00
369.60	18.03	367.90	0.00
369.70	18.54	367.90	0.00
369.80	19.06	367.90	0.00
369.90	19.57	367.90	0.00
370.00	20.07	367.90	0.00
370.10	20.58	367.90	0.00
370.20	21.07	367.90	0.00
370.30	21.56	367.90	0.00
370.40	22.04	367.90	0.00
370.50	22.51	367.90	0.00
370.60	22.99	367.90	0.00
370.70	23.46	367.90	0.00
370.80	23.91	367.90	0.00
370.90	24.36	367.90	0.00
371.00	24.81	367.90	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(it)		200 00	
366.90	-4.10	368.00	0.00
367.00	-4.10	368.00	0.00
367.10	-4.10	368.00	0.00
367.20	-4.10	368.00	0.00
367.30	-4.10	368.00	0.00
367.40	-4.10	368.00	0.00
367.50	-4.10	368.00	0.00
367.60	-4.10	368.00	0.00
367.70	-4.03	368.00	0.00
367.80	-3.70	368.00	0.00
367.90	-2.91	368.00	0.00
368.00	0.00	368.00	0.00
368.10	3.64	368.00	0.00
368.20	5.22	368.00	0.00
368.30	6.49	368.00	0.00
368.40	7.64	368.00	0.00
368.50	8.72	368.00	0.00
368.60	9.76	368.00	0.00
368.70	10.77	368.00	0.00
368.80	11.77	368.00	0.00
368.90	12.74	368.00	0.00
369.00	13.69	368.00	0.00
369.10	14.59	368.00	0.00
369.20	15.44	368.00	0.00
369.30	16.24	368.00	0.00
369.40	16.93	368.00	0.00
369.50	17.51	368.00	0.00
369.60	18.03	368.00	0.00
369.70	18.54	368.00	0.00
369.80	19.06	368.00	0.00
369.90	19.57	368.00	0.00
370.00	20.07	368.00	0.00
370.10	20.58	368.00	0.00
370.20	21.07	368.00	0.00
370.30	21.56	368.00	0.00
370.40	22.04	368.00	0.00
370.50	22.51	368.00	0.00
370.60	22.99	368.00	0.00
370.70	23.46	368.00	0.00
370.80	23.91	368.00	0.00
370.90	24.36	368.00	0.00
371.00	24.81	368.00	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(tt)	1		
366.90	-4.79	368.10	0.00
367.00	-4.79	368.10	0.00
367.10	-4.79	368.10	0.00
367.20	-4.79	368.10	0.00
367.30	-4.79	368.10	0.00
367.40	-4.79	368.10	0.00
367.50	-4.79	368.10	0.00
367.60	-4.79	368.10	0.00
367.70	-4.79	368.10	0.00
367.80	-4.65	368.10	0.00
367.90	-4.20	368.10	0.00
368.00	-3.24	368.10	0.00
368.10	0.00	368.10	0.00
368.20	4.00	368.10	0.00
368.30	5.70	368.10	0.00
368.40	7.09	368.10	0.00
368.50	8.33	368.10	0.00
368.60	9.45	368.10	0.00
368.70	10.54	368.10	0.00
368.80	11.57	368.10	0.00
368.90	12.58	368.10	0.00
369.00	13.55	368.10	0.00
369.10	14.48	368.10	0.00
369.20	15.35	368.10	0.00
369.30	16.16	368.10	0.00
369.40	16.88	368.10	0.00
369.50	17.48	368.10	0.00
369.60	18.01	368.10	0.00
369.70	18.54	368.10	0.00
369.80	19.06	368.10	0.00
369.90	19.57	368.10	0.00
3/0.00	20.07	368.10	0.00
3/0.10	20.58	368.10	0.00
370.20	21.07	368.10	0.00
3/0.30	21.56	368.10	0.00
3/0.40	22.04	368.10	0.00
3/0.50	22.51	368.10	0.00
3/0.60	22.99	368.10	0.00
3/0./0	23.46	368.10	0.00
3/0.80	23.91	368.10	0.00
3/0.90	24.36	368.10	0.00
3/1.00	24.81	368.10	0.00

Contributing Structures

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## Composite Outflow Summary

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-5.53	368.20	0.00
367.00	-5.53	368.20	0.00
367.10	-5.53	368.20	0.00
367.20	-5.53	368.20	0.00
367.30	-5.53	368.20	0.00
367.40	-5.53	368.20	0.00
367.50	-5.53	368.20	0.00
367.60	-5.53	368.20	0.00
367.70	-5.53	368.20	0.00
367.80	-5.51	368.20	0.00
367.90	-5.25	368.20	0.00
368.00	-4.70	368.20	0.00
368.10	-3.60	368.20	0.00
368.20	0.00	368.20	0.00
368.30	4.28	368.20	0.00
368.40	6.17	368.20	0.00
368.50	7.63	368.20	0.00
368.60	8.93	368.20	0.00
368.70	10.11	368.20	0.00
368.80	11.23	368.20	0.00
368.90	12.29	368.20	0.00
369.00	13.29	368.20	0.00
369.10	14.25	368.20	0.00
369.20	15.14	368.20	0.00
369.30	15.96	368.20	0.00
369.40	16.69	368.20	0.00
369.50	17.28	368.20	0.00
369.60	17.85	368.20	0.00
369.70	18.41	368.20	0.00
369.80	18.96	368.20	0.00
369.90	19.50	368.20	0.00
370.00	20.03	368.20	0.00
370.10	20.55	368.20	0.00
370.20	21.06	368.20	0.00
370.30	21.55	368.20	0.00
370.40	22.04	368.20	0.00
370.50	22.52	368.20	0.00
370.60	22.99	368.20	0.00
370.70	23.46	368.20	0.00
370.80	23.91	368.20	0.00
370.90	24.36	368.20	0.00
371.00	24.81	368.20	0.00

Contributing Structures

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## Composite Outflow Summary

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-6.29	368.30	0.00
367.00	-6.29	368.30	0.00
367.10	-6.29	368.30	0.00
367.20	-6.29	368.30	0.00
367.30	-6.29	368.30	0.00
367.40	-6.29	368.30	0.00
367.50	-6.29	368.30	0.00
367.60	-6.29	368.30	0.00
367.70	-6.29	368.30	0.00
367.80	-6.29	368.30	0.00
367.90	-6.20	368.30	0.00
368.00	-5.84	368.30	0.00
368.10	-5.17	368.30	0.00
368.20	-3.91	368.30	0.00
368.30	0.00	368.30	0.00
368.40	4.58	368.30	0.00
368.50	6.55	368.30	0.00
368.60	8.10	368.30	0.00
368.70	9.45	368.30	0.00
368.80	10.66	368.30	0.00
368.90	11.81	368.30	0.00
369.00	12.86	368.30	0.00
369.10	13.85	368.30	0.00
369.20	14.76	368.30	0.00
369.30	15.59	368.30	0.00
369.40	16.31	368.30	0.00
369.50	16.90	368.30	0.00
369.60	17.51	368.30	0.00
369.70	18.09	368.30	0.00
369.80	18.67	368.30	0.00
369.90	19.26	368.30	0.00
370.00	19.81	368.30	0.00
370.10	20.36	368.30	0.00
370.20	20.90	368.30	0.00
370.30	21.42	368.30	0.00
370.40	21.93	368.30	0.00
370.50	22.43	368.30	0.00
370.60	22.93	368.30	0.00
370.70	23.41	368.30	0.00
370.80	23.88	368.30	0.00
370.90	24.35	368.30	0.00
371.00	24.79	368.30	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-7.08	368.40	0.00
367.00	-7.08	368.40	0.00
367.10	-7.08	368.40	0.00
367.20	-7.08	368.40	0.00
367.30	-7.08	368.40	0.00
367.40	-7.08	368.40	0.00
367.50	-7.08	368.40	0.00
367.60	-7.08	368.40	0.00
367.70	-7.08	368.40	0.00
367.80	-7.08	368.40	0.00
367.90	-7.06	368.40	0.00
368.00	-6.87	368.40	0.00
368.10	-6.41	368.40	0.00
368.20	-5.63	368.40	0.00
368.30	-4.24	368.40	0.00
368.40	0.00	368.40	0.00
368.50	4.82	368.40	0.00
368.60	6.85	368.40	0.00
368.70	8.45	368.40	0.00
368.80	9.85	368.40	0.00
368.90	11.06	368.40	0.00
369.00	12.20	368.40	0.00
369.10	13.22	368.40	0.00
369.20	14.16	368.40	0.00
369.30	14.96	368.40	0.00
369.40	15.61	368.40	0.00
369.50	16.26	368.40	0.00
369.60	16.91	368.40	0.00
369.70	17.56	368.40	0.00
369.80	18.19	368.40	0.00
369.90	18.81	368.40	0.00
370.00	19.41	368.40	0.00
370.10	19.98	368.40	0.00
370.20	20.55	368.40	0.00
370.30	21.11	368.40	0.00
370.40	21.65	368.40	0.00
370.50	22.17	368.40	0.00
370.60	22.69	368.40	0.00
370.70	23.19	368.40	0.00
370.80	23.69	368.40	0.00
370.90	24.18	368.40	0.00
371.00	24.65	368.40	0.00

Contributing Structures

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# Composite Outflow Summary

Contributing Structures

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-7.87	368.50	0.00
367.00	-7.87	368.50	0.00
367.10	-7.87	368.50	0.00
367.20	-7.87	368.50	0.00
367.30	-7.87	368.50	0.00
367.40	-7.87	368.50	0.00
367.50	-7.87	368.50	0.00
367.60	-7.87	368.50	0.00
367.70	-7.87	368.50	0.00
367.80	-7.87	368.50	0.00
367.90	-7.87	368.50	0.00
368.00	-7.80	368.50	0.00
368.10	-7.51	368.50	0.00
368.20	-6.94	368.50	0.00
368.30	-6.01	368.50	0.00
368.40	-4.48	368.50	0.00
368.50	0.00	368.50	0.00
368.60	4.94	368.50	0.00
368.70	7.00	368.50	0.00
368.80	8.64	368.50	0.00
368.90	10.00	368.50	0.00
369.00	11.18	368.50	0.00
369.10	12.21	368.50	0.00
369.20	13.03	368.50	0.00
369.30	13.74	368.50	0.00
369.40	14.58	368.50	0.00
369.50	15.36	368.50	0.00
369.60	16.12	368.50	0.00
369.70	16.84	368.50	0.00
369.80	17.53	368.50	0.00
369.90	18.18	368.50	0.00
370.00	18.82	368.50	0.00
370.10	19.44	368.50	0.00
370.20	20.04	368.50	0.00
370.30	20.62	368.50	0.00
370.40	21.18	368.50	0.00
370.50	21.74	368.50	0.00
370.60	22.27	368.50	0.00
370.70	22.79	368.50	0.00
370.80	23.30	368.50	0.00
370.90	23.80	368.50	0.00
371.00	24.30	368.50	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-8.65	368.60	0.00
367.00	-8.65	368.60	0.00
367.10	-8.65	368.60	0.00
367.20	-8.65	368.60	0.00
367.30	-8.65	368.60	0.00
367.40	-8.65	368.60	0.00
367.50	-8.65	368.60	0.00
367.60	-8.65	368.60	0.00
367.70	-8.65	368.60	0.00
367.80	-8.65	368.60	0.00
367.90	-8.65	368.60	0.00
368.00	-8.64	368.60	0.00
368.10	-8.48	368.60	0.00
368.20	-8.08	368.60	0.00
368.30	-7.39	368.60	0.00
368.40	-6.34	368.60	0.00
368.50	-4.63	368.60	0.00
368.60	0.00	368.60	0.00
368.70	4.95	368.60	0.00
368.80	6.98	368.60	0.00
368.90	8.53	368.60	0.00
369.00	9.82	368.60	0.00
369.10	10.89	368.60	0.00
369.20	11.90	368.60	0.00
369.30	12.85	368.60	0.00
369.40	13.74	368.60	0.00
369.50	14.58	368.60	0.00
369.60	15.36	368.60	0.00
369.70	16.12	368.60	0.00
369.80	16.83	368.60	0.00
369.90	17.52	368.60	0.00
370.00	18.19	368.60	0.00
370.10	18.83	368.60	0.00
370.20	19.44	368.60	0.00
370.30	20.03	368.60	0.00
370.40	20.62	368.60	0.00
370.50	21.18	368.60	0.00
370.60	21.73	368.60	0.00
370.70	22.27	368.60	0.00
370.80	22.79	368.60	0.00
370.90	23.31	368.60	0.00
371.00	23.80	368.60	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 1 years Storm Event: 1

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-9.42	368.70	0.00
367.00	-9.42	368.70	0.00
367.10	-9.42	368.70	0.00
367.20	-9.42	368.70	0.00
367.30	-9.42	368.70	0.00
367.40	-9.42	368.70	0.00
367.50	-9.42	368.70	0.00
367.60	-9.42	368.70	0.00
367.70	-9.42	368.70	0.00
367.80	-9.42	368.70	0.00
367.90	-9.42	368.70	0.00
368.00	-9.42	368.70	0.00
368.10	-9.35	368.70	0.00
368.20	-9.06	368.70	0.00
368.30	-8.56	368.70	0.00
368.40	-7.75	368.70	0.00
368.50	-6.56	368.70	0.00
368.60	-4.77	368.70	0.00
368.70	0.00	368.70	0.00
368.80	4.89	368.70	0.00
368.90	6.93	368.70	0.00
369.00	8.45	368.70	0.00
369.10	9.71	368.70	0.00
369.20	10.86	368.70	0.00
369.30	11.91	368.70	0.00
369.40	12.86	368.70	0.00
369.50	13.74	368.70	0.00
369.60	14.57	368.70	0.00
369.70	15.37	368.70	0.00
369.80	16.12	368.70	0.00
369.90	16.84	368.70	0.00
370.00	17.52	368.70	0.00
370.10	18.19	368.70	0.00
370.20	18.82	368.70	0.00
370.30	19.43	368.70	0.00
370.40	20.04	368.70	0.00
370.50	20.62	368.70	0.00
370.60	21.18	368.70	0.00
370.70	21.73	368.70	0.00
370.80	22.26	368.70	0.00
370.90	22.80	368.70	0.00
371.00	23.31	368.70	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

Contributing Structures

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## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-10.16	368.80	0.00
367.00	-10.16	368.80	0.00
367.10	-10.16	368.80	0.00
367.20	-10.16	368.80	0.00
367.30	-10.16	368.80	0.00
367.40	-10.16	368.80	0.00
367.50	-10.16	368.80	0.00
367.60	-10.16	368.80	0.00
367.70	-10.16	368.80	0.00
367.80	-10.16	368.80	0.00
367.90	-10.16	368.80	0.00
368.00	-10.16	368.80	0.00
368.10	-10.13	368.80	0.00
368.20	-9.95	368.80	0.00
368.30	-9.56	368.80	0.00
368.40	-8.94	368.80	0.00
368.50	-8.01	368.80	0.00
368.60	-6.72	368.80	0.00
368.70	-4.82	368.80	0.00
368.80	0.00	368.80	0.00
368.90	4.85	368.80	0.00
369.00	6.87	368.80	0.00
369.10	8.43	368.80	0.00
369.20	9.72	368.80	0.00
369.30	10.86	368.80	0.00
369.40	11.91	368.80	0.00
369.50	12.86	368.80	0.00
369.60	13.74	368.80	0.00
369.70	14.58	368.80	0.00
369.80	15.37	368.80	0.00
369.90	16.12	368.80	0.00
370.00	16.83	368.80	0.00
370.10	17.52	368.80	0.00
370.20	18.19	368.80	0.00
370.30	18.82	368.80	0.00
370.40	19.44	368.80	0.00
370.50	20.04	368.80	0.00
370.60	20.62	368.80	0.00
370.70	21.18	368.80	0.00
370.80	21.73	368.80	0.00
370.90	22.27	368.80	0.00
371.00	22.79	368.80	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-10.87	368.90	0.00
367.00	-10.87	368.90	0.00
367.10	-10.87	368.90	0.00
367.20	-10.87	368.90	0.00
367.30	-10.87	368.90	0.00
367.40	-10.87	368.90	0.00
367.50	-10.87	368.90	0.00
367.60	-10.87	368.90	0.00
367.70	-10.87	368.90	0.00
367.80	-10.87	368.90	0.00
367.90	-10.87	368.90	0.00
368.00	-10.87	368.90	0.00
368.10	-10.87	368.90	0.00
368.20	-10.78	368.90	0.00
368.30	-10.49	368.90	0.00
368.40	-9.99	368.90	0.00
368.50	-9.25	368.90	0.00
368.60	-8.20	368.90	0.00
368.70	-6.82	368.90	0.00
368.80	-4.86	368.90	0.00
368.90	0.00	368.90	0.00
369.00	4.87	368.90	0.00
369.10	6.86	368.90	0.00
369.20	8.42	368.90	0.00
369.30	9.73	368.90	0.00
369.40	10.87	368.90	0.00
369.50	11.91	368.90	0.00
369.60	12.86	368.90	0.00
369.70	13.74	368.90	0.00
369.80	14.58	368.90	0.00
369.90	15.37	368.90	0.00
370.00	16.12	368.90	0.00
370.10	16.84	368.90	0.00
370.20	17.52	368.90	0.00
370.30	18.18	368.90	0.00
370.40	18.82	368.90	0.00
370.50	19.44	368.90	0.00
370.60	20.04	368.90	0.00
370.70	20.61	368.90	0.00
370.80	21.18	368.90	0.00
370.90	21.74	368.90	0.00
371.00	22.27	368.90	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
	11 50	200.00	0.00
300.90	-11.58	369.00	0.00
307.00	-11.58	369.00	0.00
307.10	-11.58	369.00	0.00
307.20	-11.58	369.00	0.00
307.30	-11.58	369.00	0.00
307.40	-11.58	369.00	0.00
307.50	-11.58	309.00	0.00
267,00	-11.50	309.00	0.00
267.90	-11.50	309.00	0.00
307.80	-11.58	309.00	0.00
307.90	-11.50	309.00	0.00
269.10	-11.50	309.00	0.00
269.20	-11.50	309.00	0.00
269.20	-11.54	369.00	0.00
269.40	-11.54	309.00	0.00
368 50	-10.94	369.00	0.00
368.60	-10.52	369.00	0.00
368 70	-9.77	369.00	0.00
368.80	-6.87	369.00	0.00
368.90	-4.86	369.00	0.00
369.00	4.00	369.00	0.00
369.10	4 84	369.00	0.00
369.20	6.88	369.00	0.00
369.30	8 41	369.00	0.00
369.40	9.72	369.00	0.00
369.50	10.86	369.00	0.00
369.60	11.90	369.00	0.00
369.70	12.85	369.00	0.00
369.80	13.74	369.00	0.00
369.90	14.58	369.00	0.00
370.00	15.37	369.00	0.00
370.10	16.12	369.00	0.00
370.20	16.84	369.00	0.00
370.30	17.52	369.00	0.00
370.40	18.18	369.00	0.00
370.50	18.82	369.00	0.00
370.60	19.43	369.00	0.00
370.70	20.04	369.00	0.00
370.80	20.62	369.00	0.00
370.90	21.18	369.00	0.00
371.00	21.73	369.00	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

Contributing Structures

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# Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-12.25	369.10	0.00
367.00	-12.25	369.10	0.00
367.10	-12.25	369.10	0.00
367.20	-12.25	369.10	0.00
367.30	-12.25	369.10	0.00
367.40	-12.25	369.10	0.00
367.50	-12.25	369.10	0.00
367.60	-12.25	369.10	0.00
367.70	-12.25	369.10	0.00
367.80	-12.25	369.10	0.00
367.90	-12.25	369.10	0.00
368.00	-12.25	369.10	0.00
368.10	-12.25	369.10	0.00
368.20	-12.25	369.10	0.00
368.30	-12.12	369.10	0.00
368.40	-11.80	369.10	0.00
368.50	-11.30	369.10	0.00
368.60	-10.56	369.10	0.00
368.70	-9.61	369.10	0.00
368.80	-8.39	369.10	0.00
368.90	-6.87	369.10	0.00
369.00	-4.86	369.10	0.00
369.10	0.00	369.10	0.00
369.20	4.85	369.10	0.00
369.30	6.87	369.10	0.00
369.40	8.42	369.10	0.00
369.50	9.72	369.10	0.00
369.60	10.86	369.10	0.00
369.70	11.90	369.10	0.00
369.80	12.86	369.10	0.00
369.90	13.74	369.10	0.00
370.00	14.58	369.10	0.00
370.10	15.37	369.10	0.00
370.20	16.12	369.10	0.00
370.30	16.84	369.10	0.00
370.40	17.53	369.10	0.00
370.50	18.18	369.10	0.00
370.60	18.82	369.10	0.00
370.70	19.43	369.10	0.00
370.80	20.03	369.10	0.00
370.90	20.62	369.10	0.00
371.00	21.18	369.10	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-12.92	369.20	0.00
367.00	-12.92	369.20	0.00
367.10	-12.92	369.20	0.00
367.20	-12.92	369.20	0.00
367.30	-12.92	369.20	0.00
367.40	-12.92	369.20	0.00
367.50	-12.92	369.20	0.00
367.60	-12.92	369.20	0.00
367.70	-12.92	369.20	0.00
367.80	-12.92	369.20	0.00
367.90	-12.92	369.20	0.00
368.00	-12.92	369.20	0.00
368.10	-12.92	369.20	0.00
368.20	-12.92	369.20	0.00
368.30	-12.85	369.20	0.00
368.40	-12.61	369.20	0.00
368.50	-12.18	369.20	0.00
368.60	-11.56	369.20	0.00
368.70	-10.73	369.20	0.00
368.80	-9.70	369.20	0.00
368.90	-8.42	369.20	0.00
369.00	-6.87	369.20	0.00
369.10	-4.86	369.20	0.00
369.20	0.00	369.20	0.00
369.30	4.88	369.20	0.00
369.40	6.87	369.20	0.00
369.50	8.42	369.20	0.00
369.60	9.71	369.20	0.00
369.70	10.88	369.20	0.00
369.80	11.91	369.20	0.00
369.90	12.87	369.20	0.00
370.00	13.75	369.20	0.00
370.10	14.58	369.20	0.00
370.20	15.37	369.20	0.00
370.30	16.12	369.20	0.00
370.40	16.84	369.20	0.00
370.50	17.52	369.20	0.00
370.60	18.19	369.20	0.00
370.70	18.82	369.20	0.00
370.80	19.43	369.20	0.00
370.90	20.04	369.20	0.00
371.00	20.62	369.20	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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# Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-13.58	369.30	0.00
367.00	-13.58	369.30	0.00
367.10	-13.58	369.30	0.00
367.20	-13.58	369.30	0.00
367.30	-13.58	369.30	0.00
367.40	-13.58	369.30	0.00
367.50	-13.58	369.30	0.00
367.60	-13.58	369.30	0.00
367.70	-13.58	369.30	0.00
367.80	-13.58	369.30	0.00
367.90	-13.58	369.30	0.00
368.00	-13.58	369.30	0.00
368.10	-13.58	369.30	0.00
368.20	-13.58	369.30	0.00
368.30	-13.55	369.30	0.00
368.40	-13.38	369.30	0.00
368.50	-13.02	369.30	0.00
368.60	-12.48	369.30	0.00
368.70	-11.75	369.30	0.00
368.80	-10.82	369.30	0.00
368.90	-9.73	369.30	0.00
369.00	-8.42	369.30	0.00
369.10	-6.87	369.30	0.00
369.20	-4.86	369.30	0.00
369.30	0.00	369.30	0.00
369.40	4.86	369.30	0.00
369.50	6.86	369.30	0.00
369.60	8.41	369.30	0.00
369.70	9.72	369.30	0.00
369.80	10.87	369.30	0.00
369.90	11.90	369.30	0.00
370.00	12.86	369.30	0.00
370.10	13.75	369.30	0.00
370.20	14.58	369.30	0.00
370.30	15.36	369.30	0.00
370.40	16.12	369.30	0.00
370.50	16.83	369.30	0.00
370.60	17.52	369.30	0.00
370.70	18.19	369.30	0.00
370.80	18.82	369.30	0.00
370.90	19.44	369.30	0.00
371.00	20.04	369.30	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 178 of 448

# Composite Outflow Summary

Contributing Structures

	<b>J</b>
Culvert - 1	
cuivert - 1	

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 1 years Storm Event: 1

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-14.22	369.40	0.00
367.00	-14.22	369.40	0.00
367.10	-14.22	369.40	0.00
367.20	-14.22	369.40	0.00
367.30	-14.22	369.40	0.00
367.40	-14.22	369.40	0.00
367.50	-14.22	369.40	0.00
367.60	-14.22	369.40	0.00
367.70	-14.22	369.40	0.00
367.80	-14.22	369.40	0.00
367.90	-14.22	369.40	0.00
368.00	-14.22	369.40	0.00
368.10	-14.22	369.40	0.00
368.20	-14.22	369.40	0.00
368.30	-14.22	369.40	0.00
368.40	-14.09	369.40	0.00
368.50	-13.80	369.40	0.00
368.60	-13.33	369.40	0.00
368.70	-12.68	369.40	0.00
368.80	-11.86	369.40	0.00
368.90	-10.87	369.40	0.00
369.00	-9.73	369.40	0.00
369.10	-8.42	369.40	0.00
369.20	-6.87	369.40	0.00
369.30	-4.86	369.40	0.00
369.40	0.00	369.40	0.00
369.50	4.88	369.40	0.00
369.60	6.86	369.40	0.00
369.70	8.43	369.40	0.00
369.80	9.72	369.40	0.00
369.90	10.87	369.40	0.00
370.00	11.90	369.40	0.00
370.10	12.86	369.40	0.00
370.20	13.74	369.40	0.00
370.30	14.58	369.40	0.00
370.40	15.37	369.40	0.00
370.50	16.12	369.40	0.00
370.60	16.83	369.40	0.00
370.70	17.51	369.40	0.00
370.80	18.19	369.40	0.00
370.90	18.82	369.40	0.00
371.00	19.44	369.40	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 180 of 448

# Composite Outflow Summary

Contributing Structures

	<b>J</b>
Culvert - 1	
cuivert - 1	

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 1 years Storm Event: 1

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## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-14.84	369.50	0.00
367.00	-14.84	369.50	0.00
367.10	-14.84	369.50	0.00
367.20	-14.84	369.50	0.00
367.30	-14.84	369.50	0.00
367.40	-14.84	369.50	0.00
367.50	-14.84	369.50	0.00
367.60	-14.84	369.50	0.00
367.70	-14.84	369.50	0.00
367.80	-14.84	369.50	0.00
367.90	-14.84	369.50	0.00
368.00	-14.84	369.50	0.00
368.10	-14.84	369.50	0.00
368.20	-14.84	369.50	0.00
368.30	-14.84	369.50	0.00
368.40	-14.77	369.50	0.00
368.50	-14.53	369.50	0.00
368.60	-14.13	369.50	0.00
368.70	-13.55	369.50	0.00
368.80	-12.80	369.50	0.00
368.90	-11.90	369.50	0.00
369.00	-10.87	369.50	0.00
369.10	-9.73	369.50	0.00
369.20	-8.42	369.50	0.00
369.30	-6.87	369.50	0.00
369.40	-4.86	369.50	0.00
369.50	0.00	369.50	0.00
369.60	4.84	369.50	0.00
369.70	6.87	369.50	0.00
369.80	8.42	369.50	0.00
369.90	9.73	369.50	0.00
370.00	10.86	369.50	0.00
370.10	11.90	369.50	0.00
370.20	12.86	369.50	0.00
370.30	13.75	369.50	0.00
370.40	14.58	369.50	0.00
370.50	15.36	369.50	0.00
370.60	16.11	369.50	0.00
370.70	16.83	369.50	0.00
370.80	17.52	369.50	0.00
370.90	18.18	369.50	0.00
371.00	18.82	369.50	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 182 of 448

# Composite Outflow Summary

Contributing Structures

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Culvert - 1	
cuivert - 1	

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 1 years Storm Event: 1

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-15.46	369.60	0.00
367.00	-15.46	369.60	0.00
367.10	-15.46	369.60	0.00
367.20	-15.46	369.60	0.00
367.30	-15.46	369.60	0.00
367.40	-15.46	369.60	0.00
367.50	-15.46	369.60	0.00
367.60	-15.46	369.60	0.00
367.70	-15.46	369.60	0.00
367.80	-15.46	369.60	0.00
367.90	-15.46	369.60	0.00
368.00	-15.46	369.60	0.00
368.10	-15.46	369.60	0.00
368.20	-15.46	369.60	0.00
368.30	-15.46	369.60	0.00
368.40	-15.43	369.60	0.00
368.50	-15.23	369.60	0.00
368.60	-14.88	369.60	0.00
368.70	-14.36	369.60	0.00
368.80	-13.69	369.60	0.00
368.90	-12.85	369.60	0.00
369.00	-11.90	369.60	0.00
369.10	-10.87	369.60	0.00
369.20	-9.73	369.60	0.00
369.30	-8.42	369.60	0.00
369.40	-6.87	369.60	0.00
369.50	-4.86	369.60	0.00
369.60	0.00	369.60	0.00
369.70	4.85	369.60	0.00
369.80	6.87	369.60	0.00
369.90	8.41	369.60	0.00
370.00	9.73	369.60	0.00
370.10	10.87	369.60	0.00
370.20	11.90	369.60	0.00
370.30	12.87	369.60	0.00
370.40	13.75	369.60	0.00
370.50	14.58	369.60	0.00
370.60	15.36	369.60	0.00
370.70	16.12	369.60	0.00
370.80	16.83	369.60	0.00
370.90	17.52	369.60	0.00
371.00	18.19	369.60	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 184 of 448

## Composite Outflow Summary

Contributing Structures

	<b>J</b>
Culvert - 1	
cuivert - 1	

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 185 of 448

#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/S)	(π)	(π)
	_16.06	360 70	0.00
300.90	-10.00	309.70	0.00
307.00	-10.00	269.70	0.00
267.20	-10.00	260 70	0.00
267.20	-10.00	260.70	0.00
267.40	-10.00	260.70	0.00
267 50	-10.00	260.70	0.00
367.50	-10.00	369.70	0.00
367.00	-16.06	369.70	0.00
367.80	-16.06	369.70	0.00
367.00	-16.06	369.70	0.00
368.00	-16.06	369.70	0.00
368 10	-16.06	369.70	0.00
368.20	-16.06	369.70	0.00
368 30	-16.06	369.70	0.00
368 40	-16.05	369.70	0.00
368 50	-15 90	369.70	0.00
368.60	-15 59	369.70	0.00
368 70	-15.14	369.70	0.00
368.80	-14 52	369.70	0.00
368.90	-13.74	369.70	0.00
369.00	-12.85	369.70	0.00
369.10	-11.90	369.70	0.00
369.20	-10.87	369.70	0.00
369.30	-9.73	369.70	0.00
369.40	-8.42	369.70	0.00
369.50	-6.87	369.70	0.00
369.60	-4.86	369.70	0.00
369.70	0.00	369.70	0.00
369.80	4.85	369.70	0.00
369.90	6.87	369.70	0.00
370.00	8.41	369.70	0.00
370.10	9.73	369.70	0.00
370.20	10.87	369.70	0.00
370.30	11.91	369.70	0.00
370.40	12.86	369.70	0.00
370.50	13.75	369.70	0.00
370.60	14.58	369.70	0.00
370.70	15.37	369.70	0.00
370.80	16.12	369.70	0.00
370.90	16.84	369.70	0.00
371.00	17.52	369.70	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

Contributing Structures

	<b>J</b>
Culvert - 1	
cuivert - 1	

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 1 years Storm Event: 1

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# Composite Outflow Summary

Water Surface	Flow (ft3/s)	Tailwater Elevation	Convergence Error
(ft)	(11-75)	(10)	(11)
366.90	-16.64	369.80	0.00
367.00	-16.64	369.80	0.00
367.10	-16.64	369.80	0.00
367.20	-16.64	369.80	0.00
367.30	-16.64	369.80	0.00
367.40	-16.64	369.80	0.00
367.50	-16.64	369.80	0.00
367.60	-16.64	369.80	0.00
367.70	-16.64	369.80	0.00
367.80	-16.64	369.80	0.00
367.90	-16.64	369.80	0.00
368.00	-16.64	369.80	0.00
368.10	-16.64	369.80	0.00
368.20	-16.64	369.80	0.00
368.30	-16.64	369.80	0.00
368.40	-16.64	369.80	0.00
368.50	-16.55	369.80	0.00
368.60	-16.28	369.80	0.00
368.70	-15.85	369.80	0.00
368.80	-15.29	369.80	0.00
368.90	-14.58	369.80	0.00
369.00	-13.74	369.80	0.00
369.10	-12.85	369.80	0.00
369.20	-11.90	369.80	0.00
369.30	-10.87	369.80	0.00
369.40	-9.73	369.80	0.00
369.50	-8.42	369.80	0.00
369.60	-6.87	369.80	0.00
369.70	-4.86	369.80	0.00
369.80	0.00	369.80	0.00
369.90	4.87	369.80	0.00
370.00	6.88	369.80	0.00
370.10	8.42	369.80	0.00
3/0.20	9.73	369.80	0.00
370.30	10.86	369.80	0.00
3/0.40	11.90	369.80	0.00
3/0.50	12.86	369.80	0.00
3/0.60	13./5	369.80	0.00
3/0./0	14.58	369.80	0.00
3/0.80	15.37	369.80	0.00
370.90	10.11	303.80	0.00
371.00	16.84	369.80	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 188 of 448

## Composite Outflow Summary

Contributing Structures

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Culvert - 1	
cuivert - 1	

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 189 of 448

#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Lievation (ft)(ft)(ft) $366.90$ $-17.21$ $369.90$ $0.00$ $367.00$ $-17.21$ $369.90$ $0.00$ $367.10$ $-17.21$ $369.90$ $0.00$ $367.30$ $-17.21$ $369.90$ $0.00$ $367.30$ $-17.21$ $369.90$ $0.00$ $367.50$ $-17.21$ $369.90$ $0.00$ $367.60$ $-17.21$ $369.90$ $0.00$ $367.70$ $-17.21$ $369.90$ $0.00$ $367.60$ $-17.21$ $369.90$ $0.00$ $367.70$ $-17.21$ $369.90$ $0.00$ $367.80$ $-17.21$ $369.90$ $0.00$ $367.80$ $-17.21$ $369.90$ $0.00$ $368.00$ $-17.21$ $369.90$ $0.00$ $368.00$ $-17.21$ $369.90$ $0.00$ $368.10$ $-17.21$ $369.90$ $0.00$ $368.20$ $-17.21$ $369.90$ $0.00$ $368.30$ $-17.21$ $369.90$ $0.00$ $368.40$ $-17.21$ $369.90$ $0.00$ $368.50$ $-17.15$ $369.90$ $0.00$ $368.60$ $-16.57$ $369.90$ $0.00$ $368.70$ $-16.57$ $369.90$ $0.00$ $368.90$ $-15.37$ $369.90$ $0.00$ $369.90$ $-12.85$ $369.90$ $0.00$ $369.90$ $-12.85$ $369.90$ $0.00$ $369.90$ $-12.85$ $369.90$ $0.00$ $369.90$ $-10.87$ $369.90$ $0.00$ $369.90$
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368.20 $-17.21$ $369.90$ $0.00$ $368.30$ $-17.21$ $369.90$ $0.00$ $368.40$ $-17.21$ $369.90$ $0.00$ $368.50$ $-17.15$ $369.90$ $0.00$ $368.60$ $-16.94$ $369.90$ $0.00$ $368.70$ $-16.57$ $369.90$ $0.00$ $368.80$ $-16.03$ $369.90$ $0.00$ $368.90$ $-15.37$ $369.90$ $0.00$ $369.90$ $-14.58$ $369.90$ $0.00$ $369.10$ $-13.74$ $369.90$ $0.00$ $369.20$ $-12.85$ $369.90$ $0.00$ $369.30$ $-11.90$ $369.90$ $0.00$ $369.40$ $-10.87$ $369.90$ $0.00$ $369.50$ $-9.73$ $369.90$ $0.00$ $369.60$ $-8.42$ $369.90$ $0.00$ $369.70$ $-6.87$ $369.90$ $0.00$ $369.80$ $-4.86$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.70$ $-6.87$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$ $369.90$ $0.00$
366.30 -17.21 369.90 0.00   368.40 -17.21 369.90 0.00   368.50 -17.15 369.90 0.00   368.60 -16.94 369.90 0.00   368.70 -16.57 369.90 0.00   368.80 -16.03 369.90 0.00   368.90 -15.37 369.90 0.00   369.00 -14.58 369.90 0.00   369.10 -13.74 369.90 0.00   369.20 -12.85 369.90 0.00   369.30 -11.90 369.90 0.00   369.50 -9.73 369.90 0.00   369.60 -8.42 369.90 0.00   369.70 -6.87 369.90 0.00   369.80 -4.86 369.90 0.00   369.90 0.00 369.90 0.00   369.80 -4.86 369.90 0.00   369.90 0.00 369.90 0.00 <t< td=""></t<>
308.40-17.21309.900.00368.50-17.15369.900.00368.60-16.94369.900.00368.70-16.57369.900.00368.80-16.03369.900.00368.90-15.37369.900.00369.00-14.58369.900.00369.10-13.74369.900.00369.20-12.85369.900.00369.30-11.90369.900.00369.40-10.87369.900.00369.50-9.73369.900.00369.60-8.42369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00370.004.84369.900.00370.106.89369.900.00
308.30-17.13309.900.00368.60-16.94369.900.00368.70-16.57369.900.00368.80-16.03369.900.00368.90-15.37369.900.00369.00-14.58369.900.00369.10-13.74369.900.00369.20-12.85369.900.00369.30-11.90369.900.00369.40-10.87369.900.00369.50-9.73369.900.00369.60-8.42369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00369.70-6.87369.900.00369.70-6.87369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00370.004.84369.900.00370.106.89369.900.00
368.00-10.94369.900.00368.70-16.57369.900.00368.80-16.03369.900.00368.90-15.37369.900.00369.00-14.58369.900.00369.10-13.74369.900.00369.20-12.85369.900.00369.30-11.90369.900.00369.40-10.87369.900.00369.50-9.73369.900.00369.60-8.42369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00370.004.84369.900.00370.106.89369.900.00
308.70-10.37309.900.00368.80-16.03369.900.00368.90-15.37369.900.00369.00-14.58369.900.00369.10-13.74369.900.00369.20-12.85369.900.00369.30-11.90369.900.00369.40-10.87369.900.00369.50-9.73369.900.00369.60-8.42369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.70-6.87369.900.00369.900.00369.900.00369.900.00369.900.00370.004.84369.900.00370.106.89369.900.00
308.00-10.03309.900.00368.90-15.37369.900.00369.00-14.58369.900.00369.10-13.74369.900.00369.20-12.85369.900.00369.30-11.90369.900.00369.40-10.87369.900.00369.50-9.73369.900.00369.60-8.42369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00370.004.84369.900.00370.106.89369.900.00
368.30-15.37369.900.00369.00-14.58369.900.00369.10-13.74369.900.00369.20-12.85369.900.00369.30-11.90369.900.00369.40-10.87369.900.00369.50-9.73369.900.00369.60-8.42369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00369.900.00370.004.84369.900.00370.106.89369.900.00
369.00-14.38369.900.00369.10-13.74369.900.00369.20-12.85369.900.00369.30-11.90369.900.00369.40-10.87369.900.00369.50-9.73369.900.00369.60-8.42369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00370.004.84369.900.00370.106.89369.900.00
369.10-13.74369.900.00369.20-12.85369.900.00369.30-11.90369.900.00369.40-10.87369.900.00369.50-9.73369.900.00369.60-8.42369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00370.004.84369.900.00370.106.89369.900.00
369.20-12.83369.900.00369.30-11.90369.900.00369.40-10.87369.900.00369.50-9.73369.900.00369.60-8.42369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00369.900.00369.900.00370.004.84369.900.00370.106.89369.900.00
369.30 -11.90 369.90 0.00   369.40 -10.87 369.90 0.00   369.50 -9.73 369.90 0.00   369.60 -8.42 369.90 0.00   369.70 -6.87 369.90 0.00   369.80 -4.86 369.90 0.00   369.90 0.00 369.90 0.00   369.90 0.00 369.90 0.00   369.90 0.00 369.90 0.00   369.90 0.00 369.90 0.00   370.00 4.84 369.90 0.00   370.10 6.89 369.90 0.00
309.40-10.87309.900.00369.50-9.73369.900.00369.60-8.42369.900.00369.70-6.87369.900.00369.80-4.86369.900.00369.900.00369.900.00370.004.84369.900.00370.106.89369.900.00
369.30 -9.73 369.90 0.00   369.60 -8.42 369.90 0.00   369.70 -6.87 369.90 0.00   369.80 -4.86 369.90 0.00   369.90 0.00 369.90 0.00   369.90 0.00 369.90 0.00   370.00 4.84 369.90 0.00   370.10 6.89 369.90 0.00
369.00 -6.42 369.90 0.00   369.70 -6.87 369.90 0.00   369.80 -4.86 369.90 0.00   369.90 0.00 369.90 0.00   369.90 0.00 369.90 0.00   370.00 4.84 369.90 0.00   370.10 6.89 369.90 0.00
369.80 -4.86 369.90 0.00   369.90 0.00 369.90 0.00   370.00 4.84 369.90 0.00   370.10 6.89 369.90 0.00
369.90 0.00 369.90 0.00   370.00 4.84 369.90 0.00   370.10 6.89 369.90 0.00
370.00 4.84 369.90 0.00   370.10 6.89 369.90 0.00
370.10 6.89 369.90 0.00
370.20 8.41 369.90 0.00
370.20 0.71 305.50 0.00
370.30 3.73 303.30 0.00
370.60 12.86 360.00 0.00
370.00 12.00 305.90 0.00
370.80 14.57 360.00 0.00
370.00 15.36 369.90 0.00
371.00 16.11 369.90 0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 190 of 448

# Composite Outflow Summary

Contributing Structures

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Culvert - 1	
cuivert - 1	

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(tt)	1		
366.90	-17.79	370.00	0.00
367.00	-17.79	370.00	0.00
367.10	-17.79	370.00	0.00
367.20	-17.79	370.00	0.00
367.30	-17.79	370.00	0.00
367.40	-17.79	370.00	0.00
367.50	-17.79	370.00	0.00
367.60	-17.79	370.00	0.00
367.70	-17.79	370.00	0.00
367.80	-17.79	370.00	0.00
367.90	-17.79	370.00	0.00
368.00	-17.79	370.00	0.00
368.10	-17.79	370.00	0.00
368.20	-17.79	370.00	0.00
368.30	-17.79	370.00	0.00
368.40	-17.79	370.00	0.00
368.50	-17.75	370.00	0.00
368.60	-17.57	370.00	0.00
368.70	-17.24	370.00	0.00
368.80	-16.75	370.00	0.00
368.90	-16.12	370.00	0.00
369.00	-15.3/	3/0.00	0.00
369.10	-14.58	3/0.00	0.00
369.20	-13.74	370.00	0.00
369.30	-12.85	370.00	0.00
369.40	-11.90	3/0.00	0.00
369.50	-10.8/	3/0.00	0.00
369.60	-9./3	3/0.00	0.00
369.70	-8.42	3/0.00	0.00
369.80	-6.8/	370.00	0.00
369.90	-4.86	370.00	0.00
3/0.00	0.00	3/0.00	0.00
3/0.10	4.84	370.00	0.00
3/0.20	6.88	370.00	0.00
3/0.30	8.43	3/0.00	0.00
3/0.40	9.72	3/0.00	0.00
3/0.50	10.87	3/0.00	0.00
370.60	11.90	370.00	0.00
370.70	12.86	370.00	0.00
370.80	13.74	370.00	0.00
370.90	14.58	370.00	0.00
371.00	15.37	370.00	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 192 of 448

## Composite Outflow Summary

Contributing Structures

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cuivert - 1	

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#### Return Event: 1 years Storm Event: 1

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-18.33	370.10	0.00
367.00	-18.33	3/0.10	0.00
367.10	-18.33	3/0.10	0.00
367.20	-18.33	3/0.10	0.00
367.30	-18.33	3/0.10	0.00
367.40	-18.33	370.10	0.00
367.50	-18.33	3/0.10	0.00
367.60	-18.33	3/0.10	0.00
367.70	-18.33	370.10	0.00
367.80	-18.33	370.10	0.00
367.90	-18.33	3/0.10	0.00
368.00	-18.33	370.10	0.00
368.10	-18.33	370.10	0.00
368.20	-18.33	370.10	0.00
368.30	-18.33	3/0.10	0.00
368.40	-18.33	3/0.10	0.00
368.50	-18.32	3/0.10	0.00
368.60	-18.17	370.10	0.00
368.70	-17.88	3/0.10	0.00
368.80	-17.43	3/0.10	0.00
368.90	-16.83	370.10	0.00
369.00	-16.12	3/0.10	0.00
369.10	-15.3/	3/0.10	0.00
369.20	-14.58	3/0.10	0.00
369.30	-13.74	3/0.10	0.00
369.40	-12.85	3/0.10	0.00
369.50	-11.90	3/0.10	0.00
369.60	-10.8/	3/0.10	0.00
369.70	-9.73	3/0.10	0.00
369.80	-8.42	3/0.10	0.00
369.90	-6.8/	3/0.10	0.00
3/0.00	-4.86	3/0.10	0.00
3/0.10	0.00	3/0.10	0.00
3/0.20	4.88	3/0.10	0.00
3/0.30	6.86	3/0.10	0.00
3/0.40	8.41	3/0.10	0.00
3/0.50	9./1	3/0.10	0.00
3/0.60	10.8/	3/0.10	0.00
3/0./0	11.91	3/0.10	0.00
3/0.80	12.86	3/0.10	0.00
3/0.90	13./4	3/0.10	0.00
3/1.00	14.58	3/0.10	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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# Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-18.87	370.20	0.00
367.00	-18.87	370.20	0.00
367.10	-18.87	370.20	0.00
367.20	-18.87	370.20	0.00
367.30	-18.87	370.20	0.00
367.40	-18.87	370.20	0.00
367.50	-18.87	370.20	0.00
367.60	-18.87	370.20	0.00
367.70	-18.87	370.20	0.00
367.80	-18.87	370.20	0.00
367.90	-18.87	370.20	0.00
368.00	-18.87	370.20	0.00
368.10	-18.87	370.20	0.00
368.20	-18.87	370.20	0.00
368.30	-18.87	370.20	0.00
368.40	-18.87	370.20	0.00
368.50	-18.87	370.20	0.00
368.60	-18.76	370.20	0.00
368.70	-18.50	370.20	0.00
368.80	-18.08	370.20	0.00
368.90	-17.52	370.20	0.00
369.00	-16.83	370.20	0.00
369.10	-16.12	370.20	0.00
369.20	-15.37	370.20	0.00
369.30	-14.58	370.20	0.00
369.40	-13.74	370.20	0.00
369.50	-12.85	370.20	0.00
369.60	-11.90	370.20	0.00
369.70	-10.87	370.20	0.00
369.80	-9.73	370.20	0.00
369.90	-8.42	370.20	0.00
370.00	-6.87	370.20	0.00
370.10	-4.86	370.20	0.00
370.20	0.00	370.20	0.00
370.30	4.85	370.20	0.00
370.40	6.88	370.20	0.00
370.50	8.43	370.20	0.00
370.60	9.72	370.20	0.00
370.70	10.86	370.20	0.00
370.80	11.91	370.20	0.00
370.90	12.86	370.20	0.00
371.00	13.74	370.20	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 196 of 448

## Composite Outflow Summary

Contributing Structures

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## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-19.42	370.30	0.00
367.00	-19.42	370.30	0.00
367.10	-19.42	370.30	0.00
367.20	-19.42	370.30	0.00
367.30	-19.42	370.30	0.00
367.40	-19.42	370.30	0.00
367.50	-19.42	370.30	0.00
367.60	-19.42	370.30	0.00
367.70	-19.42	370.30	0.00
367.80	-19.42	370.30	0.00
367.90	-19.42	370.30	0.00
368.00	-19.42	370.30	0.00
368.10	-19.42	370.30	0.00
368.20	-19.42	370.30	0.00
368.30	-19.42	370.30	0.00
368.40	-19.42	370.30	0.00
368.50	-19.42	370.30	0.00
368.60	-19.34	370.30	0.00
368.70	-19.10	370.30	0.00
368.80	-18.73	370.30	0.00
368.90	-18.18	370.30	0.00
369.00	-17.52	370.30	0.00
369.10	-16.83	370.30	0.00
369.20	-16.12	370.30	0.00
369.30	-15.37	370.30	0.00
369.40	-14.58	370.30	0.00
369.50	-13.74	370.30	0.00
369.60	-12.85	370.30	0.00
369.70	-11.90	370.30	0.00
369.80	-10.87	370.30	0.00
369.90	-9.73	370.30	0.00
370.00	-8.42	370.30	0.00
370.10	-6.87	370.30	0.00
370.20	-4.86	370.30	0.00
370.30	0.00	370.30	0.00
370.40	4.87	370.30	0.00
370.50	6.89	370.30	0.00
370.60	8.40	370.30	0.00
370.70	9.71	370.30	0.00
370.80	10.87	370.30	0.00
370.90	11.90	370.30	0.00
371.00	12.85	370.30	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
(ft)	(11-75)	(11)	(10)
366.90	-19 94	370 40	0.00
367.00	-19.94	370.40	0.00
367.00	-19 94	370.10	0.00
367.20	-19.94	370.10	0.00
367.30	-19.94	370.40	0.00
367.40	-19.94	370.40	0.00
367.50	-19.94	370.40	0.00
367.60	-19.94	370.40	0.00
367.70	-19.94	370.40	0.00
367.80	-19.94	370.40	0.00
367.90	-19.94	370.40	0.00
368.00	-19.94	370.40	0.00
368.10	-19.94	370.40	0.00
368.20	-19.94	370.40	0.00
368.30	-19.94	370.40	0.00
368.40	-19.94	370.40	0.00
368.50	-19.94	370.40	0.00
368.60	-19.88	370.40	0.00
368.70	-19.68	370.40	0.00
368.80	-19.34	370.40	0.00
368.90	-18.82	370.40	0.00
369.00	-18.18	370.40	0.00
369.10	-17.52	370.40	0.00
369.20	-16.83	370.40	0.00
369.30	-16.12	370.40	0.00
369.40	-15.37	370.40	0.00
369.50	-14.58	370.40	0.00
369.60	-13.74	370.40	0.00
369.70	-12.85	370.40	0.00
369.80	-11.90	370.40	0.00
369.90	-10.87	370.40	0.00
370.00	-9.73	370.40	0.00
370.10	-8.42	370.40	0.00
370.20	-6.87	370.40	0.00
370.30	-4.86	370.40	0.00
370.40	0.00	370.40	0.00
370.50	4.88	370.40	0.00
370.60	6.88	370.40	0.00
370.70	8.42	370.40	0.00
370.80	9.72	370.40	0.00
370.90	10.87	370.40	0.00
371.00	11.90	370.40	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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# Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-20.46	370.50	0.00
367.00	-20.46	370.50	0.00
367.10	-20.46	370.50	0.00
367.20	-20.46	370.50	0.00
367.30	-20.46	370.50	0.00
367.40	-20.46	370.50	0.00
367.50	-20.46	370.50	0.00
367.60	-20.46	370.50	0.00
367.70	-20.46	370.50	0.00
367.80	-20.46	370.50	0.00
367.90	-20.46	370.50	0.00
368.00	-20.46	370.50	0.00
368.10	-20.46	370.50	0.00
368.20	-20.46	370.50	0.00
368.30	-20.46	370.50	0.00
368.40	-20.46	370.50	0.00
368.50	-20.46	370.50	0.00
368.60	-20.43	370.50	0.00
368.70	-20.25	370.50	0.00
368.80	-19.93	370.50	0.00
368.90	-19.44	370.50	0.00
369.00	-18.82	370.50	0.00
369.10	-18.18	370.50	0.00
369.20	-17.52	370.50	0.00
369.30	-16.83	370.50	0.00
369.40	-16.12	370.50	0.00
369.50	-15.37	370.50	0.00
369.60	-14.58	370.50	0.00
369.70	-13.74	370.50	0.00
369.80	-12.85	370.50	0.00
369.90	-11.90	370.50	0.00
370.00	-10.87	370.50	0.00
370.10	-9.73	370.50	0.00
370.20	-8.42	370.50	0.00
370.30	-6.87	370.50	0.00
370.40	-4.86	370.50	0.00
370.50	0.00	370.50	0.00
370.60	4.86	370.50	0.00
370.70	6.87	370.50	0.00
370.80	8.43	370.50	0.00
370.90	9.71	370.50	0.00
371.00	10.86	370.50	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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# Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-20.96	370.60	0.00
367.00	-20.96	370.60	0.00
367.10	-20.96	370.60	0.00
367.20	-20.96	370.60	0.00
367.30	-20.96	370.60	0.00
367.40	-20.96	370.60	0.00
367.50	-20.96	370.60	0.00
367.60	-20.96	370.60	0.00
367.70	-20.96	370.60	0.00
367.80	-20.96	370.60	0.00
367.90	-20.96	370.60	0.00
368.00	-20.96	370.60	0.00
368.10	-20.96	370.60	0.00
368.20	-20.96	370.60	0.00
368.30	-20.96	370.60	0.00
368.40	-20.96	370.60	0.00
368.50	-20.96	370.60	0.00
368.60	-20.95	370.60	0.00
368.70	-20.80	370.60	0.00
368.80	-20.50	370.60	0.00
368.90	-20.04	370.60	0.00
369.00	-19.44	370.60	0.00
369.10	-18.82	370.60	0.00
369.20	-18.18	370.60	0.00
369.30	-17.52	370.60	0.00
369.40	-16.83	370.60	0.00
369.50	-16.12	370.60	0.00
369.60	-15.37	370.60	0.00
369.70	-14.58	370.60	0.00
369.80	-13.74	370.60	0.00
369.90	-12.85	370.60	0.00
370.00	-11.90	370.60	0.00
370.10	-10.87	370.60	0.00
370.20	-9.73	370.60	0.00
370.30	-8.42	370.60	0.00
370.40	-6.87	370.60	0.00
370.50	-4.86	370.60	0.00
370.60	0.00	370.60	0.00
370.70	4.88	370.60	0.00
370.80	6.87	370.60	0.00
370.90	8.43	370.60	0.00
371.00	9.73	370.60	0.00

Contributing Structures

Culvert - 1

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## Composite Outflow Summary

Contributing Structures

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## Return Event: 1 years Storm Event: 1

# Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-21.46	370.70	0.00
367.00	-21.46	370.70	0.00
367.10	-21.46	370.70	0.00
367.20	-21.46	370.70	0.00
367.30	-21.46	370.70	0.00
367.40	-21.46	370.70	0.00
367.50	-21.46	370.70	0.00
367.60	-21.46	370.70	0.00
367.70	-21.46	370.70	0.00
367.80	-21.46	370.70	0.00
367.90	-21.46	370.70	0.00
368.00	-21.46	370.70	0.00
368.10	-21.46	370.70	0.00
368.20	-21.46	370.70	0.00
368.30	-21.46	370.70	0.00
368.40	-21.46	370.70	0.00
368.50	-21.46	370.70	0.00
368.60	-21.46	370.70	0.00
368.70	-21.33	370.70	0.00
368.80	-21.06	370.70	0.00
368.90	-20.62	370.70	0.00
369.00	-20.04	370.70	0.00
369.10	-19.44	370.70	0.00
369.20	-18.82	370.70	0.00
369.30	-18.18	370.70	0.00
369.40	-17.52	370.70	0.00
369.50	-16.83	370.70	0.00
369.60	-16.12	370.70	0.00
369.70	-15.3/	3/0./0	0.00
369.80	-14.58	3/0./0	0.00
369.90	-13./4	3/0./0	0.00
370.00	-12.85	370.70	0.00
3/0.10	-11.90	3/0./0	0.00
370.20	-10.87	370.70	0.00
370.30	-9.73	370.70	0.00
370.40	-8.42	370.70	0.00
370.50	-6.87	370.70	0.00
370.60	-4.86	370.70	0.00
370.70	0.00	370.70	0.00
370.80	4.87	370.70	0.00
370.90	6.86	370.70	0.00
371.00	8.42	370.70	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

Contributing Structures

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# Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-21.95	370.80	0.00
367.00	-21.95	370.80	0.00
367.10	-21.95	370.80	0.00
367.20	-21.95	370.80	0.00
367.30	-21.95	370.80	0.00
367.40	-21.95	370.80	0.00
367.50	-21.95	370.80	0.00
367.60	-21.95	370.80	0.00
367.70	-21.95	370.80	0.00
367.80	-21.95	370.80	0.00
367.90	-21.95	370.80	0.00
368.00	-21.95	370.80	0.00
368.10	-21.95	370.80	0.00
368.20	-21.95	370.80	0.00
368.30	-21.95	370.80	0.00
368.40	-21.95	370.80	0.00
368.50	-21.95	370.80	0.00
368.60	-21.95	370.80	0.00
368.70	-21.86	370.80	0.00
368.80	-21.61	370.80	0.00
368.90	-21.18	370.80	0.00
369.00	-20.62	370.80	0.00
369.10	-20.04	370.80	0.00
369.20	-19.44	370.80	0.00
369.30	-18.82	370.80	0.00
369.40	-18.18	370.80	0.00
369.50	-17.52	370.80	0.00
369.60	-16.83	370.80	0.00
369.70	-16.12	370.80	0.00
369.80	-15.37	370.80	0.00
369.90	-14.58	370.80	0.00
370.00	-13.74	370.80	0.00
370.10	-12.85	370.80	0.00
370.20	-11.90	370.80	0.00
370.30	-10.87	370.80	0.00
370.40	-9.73	370.80	0.00
370.50	-8.42	370.80	0.00
370.60	-6.87	370.80	0.00
370.70	-4.86	370.80	0.00
370.80	0.00	370.80	0.00
370.90	4.86	370.80	0.00
371.00	6.88	370.80	0.00

Contributing Structures

Culvert - 1

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# Composite Outflow Summary

Contributing Structures

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Return Event: 1 years Storm Event: 1

## Return Event: 1 years Storm Event: 1

# Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-22.47	370.90	0.00
367.00	-22.47	370.90	0.00
367.10	-22.47	370.90	0.00
367.20	-22.47	370.90	0.00
367.30	-22.47	370.90	0.00
367.40	-22.47	370.90	0.00
367.50	-22.47	370.90	0.00
367.60	-22.47	370.90	0.00
367.70	-22.47	370.90	0.00
367.80	-22.47	370.90	0.00
367.90	-22.47	370.90	0.00
368.00	-22.47	370.90	0.00
368.10	-22.47	370.90	0.00
368.20	-22.47	370.90	0.00
368.30	-22.47	370.90	0.00
368.40	-22.47	370.90	0.00
368.50	-22.47	370.90	0.00
368.60	-22.47	370.90	0.00
368.70	-22.38	370.90	0.00
368.80	-22.14	370.90	0.00
368.90	-21.73	370.90	0.00
369.00	-21.18	370.90	0.00
369.10	-20.62	370.90	0.00
369.20	-20.04	370.90	0.00
369.30	-19.44	370.90	0.00
369.40	-18.82	370.90	0.00
369.50	-18.18	370.90	0.00
369.60	-17.52	370.90	0.00
369.70	-16.83	370.90	0.00
369.80	-16.12	370.90	0.00
369.90	-15.37	370.90	0.00
370.00	-14.58	370.90	0.00
370.10	-13.74	370.90	0.00
370.20	-12.85	370.90	0.00
370.30	-11.90	370.90	0.00
370.40	-10.87	370.90	0.00
370.50	-9.73	370.90	0.00
370.60	-8.42	370.90	0.00
370.70	-6.87	370.90	0.00
370.80	-4.86	370.90	0.00
370.90	0.00	370.90	0.00
371.00	4.86	370.90	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 210 of 448

# Composite Outflow Summary

Contributing Structures

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# Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-22.94	371.00	0.00
367.00	-22.94	371.00	0.00
367.10	-22.94	371.00	0.00
367.20	-22.94	371.00	0.00
367.30	-22.94	371.00	0.00
367.40	-22.94	371.00	0.00
367.50	-22.94	371.00	0.00
367.60	-22.94	371.00	0.00
367.70	-22.94	371.00	0.00
367.80	-22.94	371.00	0.00
367.90	-22.94	371.00	0.00
368.00	-22.94	371.00	0.00
368.10	-22.94	371.00	0.00
368.20	-22.94	371.00	0.00
368.30	-22.94	371.00	0.00
368.40	-22.94	371.00	0.00
368.50	-22.94	371.00	0.00
368.60	-22.94	371.00	0.00
368.70	-22.88	371.00	0.00
368.80	-22.66	371.00	0.00
368.90	-22.27	371.00	0.00
369.00	-21.73	371.00	0.00
369.10	-21.18	371.00	0.00
369.20	-20.62	371.00	0.00
369.30	-20.04	371.00	0.00
369.40	-19.44	371.00	0.00
369.50	-18.82	371.00	0.00
369.60	-18.18	371.00	0.00
369.70	-17.52	371.00	0.00
369.80	-16.83	371.00	0.00
369.90	-16.12	371.00	0.00
370.00	-15.37	371.00	0.00
370.10	-14.58	371.00	0.00
370.20	-13.74	371.00	0.00
370.30	-12.85	371.00	0.00
370.40	-11.90	371.00	0.00
370.50	-10.87	371.00	0.00
370.60	-9.73	371.00	0.00
370.70	-8.42	371.00	0.00
370.80	-6.87	371.00	0.00
370.90	-4.86	371.00	0.00
371.00	0.00	371.00	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 212 of 448

# Composite Outflow Summary

Contributing Structures

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PondPack CONNECT Edition [10.02.00.01] Page 213 of 448 Subsection: Outlet Input Data Label: OCS-A Scenario: Proposed Conditions 10 Year Storm Return Event: 10 years Storm Event: 10

Requested Pond Water Surface ElevationsMinimum (Headwater)366.90 ftIncrement (Headwater)0.50 ftMaximum (Headwater)371.00 ft

# **Outlet Connectivity**

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Culvert-Circular	Culvert - 1	Forward + Reverse	TW	366.90	371.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

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Structure ID: Culvert - 1 Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	24.0 in
Length	120.00 ft
Length (Computed Barrel)	120.00 ft
Slope (Computed)	0.003 ft/ft
Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.012
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
К	0.0045
Μ	2.0000
С	0.0317
Y	0.6900
T1 ratio (HW/D)	1.094
T2 ratio (HW/D)	1.196
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control,

interpolate between flows at T1 & T2...

T1 Elevation	369.09 ft	T1 Flow	15.55 ft³/s
T2 Elevation	369.29 ft	T2 Flow	17.77 ft <sup>3</sup> /s

# Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	0.00	366.50	0.00
367.00	0.04	366.50	0.00
367.10	0.17	366.50	0.00
367.20	0.39	366.50	0.00
367.30	0.68	366.50	0.00
367.40	1.05	366.50	0.00
367.50	1.50	366.50	0.00
367.60	2.01	366.50	0.00
367.70	2.59	366.50	0.00
367.80	3.24	366.50	0.00
367.90	3.93	366.50	0.00
368.00	4.70	366.50	0.00
368.10	5.50	366.50	0.00
368.20	6.34	366.50	0.00
368.30	7.21	366.50	0.00
368.40	8.14	366.50	0.00
368.50	9.06	366.50	0.00
368.60	10.00	366.50	0.00
368.70	10.95	366.50	0.00
368.80	11.90	366.50	0.00
368.90	12.82	366.50	0.00
369.00	13.75	366.50	0.00
369.10	14.62	366.50	0.00
369.20	15.45	366.50	0.00
369.30	16.24	366.50	0.00
369.40	16.93	366.50	0.00
369.50	17.51	366.50	0.00
369.60	18.03	366.50	0.00
369.70	18.54	366.50	0.00
369.80	19.06	366.50	0.00
369.90	19.57	366.50	0.00
370.00	20.07	366.50	0.00
370.10	20.58	366.50	0.00
370.20	21.07	366.50	0.00
370.30	21.56	366.50	0.00
370.40	22.04	366.50	0.00
370.50	22.51	366.50	0.00
370.60	22.99	366.50	0.00
370.70	23.46	366.50	0.00
370.80	23.91	366.50	0.00
370.90	24.36	366.50	0.00
371.00	24.81	366.50	0.00

Contributing Structures

None Contributing

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### Composite Outflow Summary

Contributing Structures

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# Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)		I	
366.90	0.00	366.60	0.00
367.00	0.04	366.60	0.00
367.10	0.17	366.60	0.00
367.20	0.39	366.60	0.00
367.30	0.68	366.60	0.00
367.40	1.05	366.60	0.00
367.50	1.50	366.60	0.00
367.60	2.01	366.60	0.00
367.70	2.59	366.60	0.00
367.80	3.24	366.60	0.00
367.90	3.93	366.60	0.00
368.00	4.70	366.60	0.00
368.10	5.50	366.60	0.00
368.20	6.34	366.60	0.00
368.30	7.21	366.60	0.00
368.40	8.14	366.60	0.00
368.50	9.06	366.60	0.00
368.60	10.00	366.60	0.00
368.70	10.95	366.60	0.00
368.80	11.90	366.60	0.00
368.90	12.82	366.60	0.00
369.00	13.75	366.60	0.00
369.10	14.62	366.60	0.00
369.20	15.45	366.60	0.00
369.30	16.24	366.60	0.00
369.40	16.93	366.60	0.00
369.50	17.51	366.60	0.00
369.60	18.03	366.60	0.00
369.70	18.54	366.60	0.00
369.80	19.06	366.60	0.00
369.90	19.57	366.60	0.00
370.00	20.07	366.60	0.00
370.10	20.58	366.60	0.00
370.20	21.07	366.60	0.00
370.30	21.56	366.60	0.00
370.40	22.04	366.60	0.00
370.50	22.51	366.60	0.00
370.60	22.99	366.60	0.00
370.70	23.46	366.60	0.00
370.80	23.91	366.60	0.00
370.90	24.36	366.60	0.00
371.00	24.81	366.60	0.00

Contributing Structures

None Contributing

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### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 10 years Storm Event: 10

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# Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(tt)			
366.90	0.00	366.70	0.00
367.00	0.04	366.70	0.00
367.10	0.17	366.70	0.00
367.20	0.39	366.70	0.00
367.30	0.68	366.70	0.00
367.40	1.05	366.70	0.00
367.50	1.50	366.70	0.00
367.60	2.01	366.70	0.00
367.70	2.59	366.70	0.00
367.80	3.24	366.70	0.00
367.90	3.93	366.70	0.00
368.00	4.70	366.70	0.00
368.10	5.50	366.70	0.00
368.20	6.34	366.70	0.00
368.30	7.21	366.70	0.00
368.40	8.14	366.70	0.00
368.50	9.06	366.70	0.00
368.60	10.00	366.70	0.00
368.70	10.95	366.70	0.00
368.80	11.90	366.70	0.00
368.90	12.82	366.70	0.00
369.00	13.75	366.70	0.00
369.10	14.62	366.70	0.00
369.20	15.45	366.70	0.00
369.30	16.24	366.70	0.00
369.40	16.93	366.70	0.00
369.50	17.51	366.70	0.00
369.60	18.03	366.70	0.00
369.70	18.54	366.70	0.00
369.80	19.06	366.70	0.00
369.90	19.57	366.70	0.00
370.00	20.07	366.70	0.00
370.10	20.58	366.70	0.00
370.20	21.07	366.70	0.00
370.30	21.56	366.70	0.00
370.40	22.04	366.70	0.00
370.50	22.51	366.70	0.00
370.60	22.99	366.70	0.00
370.70	23.46	366.70	0.00
370.80	23.91	366.70	0.00
370.90	24.36	366.70	0.00
371.00	24.81	366.70	0.00

Contributing Structures

None Contributing

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 220 of 448

### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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# Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)		T	
366.90	0.00	366.80	0.00
367.00	0.04	366.80	0.00
367.10	0.17	366.80	0.00
367.20	0.39	366.80	0.00
367.30	0.68	366.80	0.00
367.40	1.05	366.80	0.00
367.50	1.50	366.80	0.00
367.60	2.01	366.80	0.00
367.70	2.59	366.80	0.00
367.80	3.24	366.80	0.00
367.90	3.93	366.80	0.00
368.00	4.70	366.80	0.00
368.10	5.50	366.80	0.00
368.20	6.34	366.80	0.00
368.30	7.21	366.80	0.00
368.40	8.14	366.80	0.00
368.50	9.06	366.80	0.00
368.60	10.00	366.80	0.00
368.70	10.95	366.80	0.00
368.80	11.90	366.80	0.00
368.90	12.82	366.80	0.00
369.00	13.75	366.80	0.00
369.10	14.62	366.80	0.00
369.20	15.45	366.80	0.00
369.30	16.24	366.80	0.00
369.40	16.93	366.80	0.00
369.50	17.51	366.80	0.00
369.60	18.03	366.80	0.00
369.70	18.54	366.80	0.00
369.80	19.06	366.80	0.00
369.90	19.57	366.80	0.00
3/0.00	20.07	366.80	0.00
3/0.10	20.58	366.80	0.00
3/0.20	21.07	366.80	0.00
370.30	21.56	366.80	0.00
3/0.40	22.04	366.80	0.00
3/0.50	22.51	366.80	0.00
370.60	22.99	366.80	0.00
370.70	23.46	366.80	0.00
3/0.80	23.91	366.80	0.00
3/0.90	24.36	366.80	0.00
3/1.00	24.81	366.80	0.00

Contributing Structures

None Contributing

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 222 of 448

### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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# Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(tt)			
366.90	0.00	366.90	0.00
367.00	0.04	366.90	0.00
367.10	0.17	366.90	0.00
367.20	0.38	366.90	0.00
367.30	0.68	366.90	0.00
367.40	1.06	366.90	0.00
367.50	1.50	366.90	0.00
367.60	2.01	366.90	0.00
367.70	2.59	366.90	0.00
367.80	3.24	366.90	0.00
367.90	3.93	366.90	0.00
368.00	4.70	366.90	0.00
368.10	5.50	366.90	0.00
368.20	6.34	366.90	0.00
368.30	7.21	366.90	0.00
368.40	8.14	366.90	0.00
368.50	9.06	366.90	0.00
368.60	10.00	366.90	0.00
368.70	10.95	366.90	0.00
368.80	11.90	366.90	0.00
368.90	12.82	366.90	0.00
369.00	13.75	366.90	0.00
369.10	14.62	366.90	0.00
369.20	15.45	366.90	0.00
369.30	16.24	366.90	0.00
369.40	16.93	366.90	0.00
369.50	17.51	366.90	0.00
369.60	18.03	366.90	0.00
369.70	18.54	366.90	0.00
369.80	19.06	366.90	0.00
369.90	19.57	366.90	0.00
370.00	20.07	366.90	0.00
370.10	20.58	366.90	0.00
370.20	21.07	366.90	0.00
370.30	21.56	366.90	0.00
370.40	22.04	366.90	0.00
370.50	22.51	366.90	0.00
370.60	22.99	366.90	0.00
370.70	23.46	366.90	0.00
370.80	23.91	366.90	0.00
370.90	24.36	366.90	0.00
371.00	24.81	366.90	0.00

Contributing Structures

None Contributing

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### Composite Outflow Summary

Contributing Structures

Contributing Structur
Culvert - 1

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## Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(π)	0.02	267.00	0.00
300.90	-0.03	307.00	0.00
367.00	0.00	307.00	0.00
367.10	0.17	367.00	0.00
307.20	0.39	307.00	0.00
367.30	0.08	307.00	0.00
307.40	1.05	307.00	0.00
307.50	1.50	307.00	0.00
307.00	2.02	307.00	0.00
367.70	2.59	307.00	0.00
367.80	3.24	307.00	0.00
307.90	3.93	307.00	0.00
300.00	4.70	307.00	0.00
300.10	5.50	307.00	0.00
300.20	0.34	307.00	0.00
368.40	7.21	367.00	0.00
269 50	0.14	307.00	0.00
368.50	9.00	367.00	0.00
368 70	10.00	367.00	0.00
368.80	11.00	367.00	0.00
368.90	11.90	367.00	0.00
369.00	12.02	367.00	0.00
369.10	14.62	367.00	0.00
369.20	15.45	367.00	0.00
369.20	16.74	367.00	0.00
369.40	16.93	367.00	0.00
369.50	17.51	367.00	0.00
369.60	18.03	367.00	0.00
369.70	18.54	367.00	0.00
369.80	19.06	367.00	0.00
369.90	19.57	367.00	0.00
370.00	20.07	367.00	0.00
370.10	20.58	367.00	0.00
370.20	21.07	367.00	0.00
370.30	21.56	367.00	0.00
370.40	22.04	367.00	0.00
370.50	22.51	367.00	0.00
370.60	22.99	367.00	0.00
370.70	23.46	367.00	0.00
370.80	23.91	367.00	0.00
370.90	24.36	367.00	0.00
371.00	24.81	367.00	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)		T	
366.90	-0.14	367.10	0.00
367.00	-0.14	367.10	0.00
367.10	0.00	367.10	0.00
367.20	0.38	367.10	0.00
367.30	0.68	367.10	0.00
367.40	1.05	367.10	0.00
367.50	1.50	367.10	0.00
367.60	2.01	367.10	0.00
367.70	2.59	367.10	0.00
367.80	3.24	367.10	0.00
367.90	3.93	367.10	0.00
368.00	4.70	367.10	0.00
368.10	5.50	367.10	0.00
368.20	6.34	367.10	0.00
368.30	7.21	367.10	0.00
368.40	8.14	367.10	0.00
368.50	9.06	367.10	0.00
368.60	10.00	367.10	0.00
368.70	10.95	367.10	0.00
368.80	11.90	367.10	0.00
368.90	12.82	367.10	0.00
369.00	13.75	367.10	0.00
369.10	14.62	367.10	0.00
369.20	15.45	367.10	0.00
369.30	16.24	367.10	0.00
369.40	16.93	367.10	0.00
369.50	17.51	367.10	0.00
369.60	18.03	367.10	0.00
369.70	18.54	367.10	0.00
369.80	19.06	367.10	0.00
369.90	19.57	367.10	0.00
370.00	20.07	367.10	0.00
370.10	20.58	367.10	0.00
370.20	21.07	367.10	0.00
370.30	21.56	367.10	0.00
370.40	22.04	367.10	0.00
370.50	22.51	367.10	0.00
370.60	22.99	367.10	0.00
370.70	23.46	367.10	0.00
370.80	23.91	367.10	0.00
370.90	24.36	367.10	0.00
371.00	24.81	367.10	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Contributing Structur
Culvert - 1

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# Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-0.32	367.20	0.00
367.00	-0.32	367.20	0.00
367.10	-0.32	367.20	0.00
367.20	0.00	367.20	0.00
367.30	0.66	367.20	0.00
367.40	1.05	367.20	0.00
367.50	1.50	367.20	0.00
367.60	2.01	367.20	0.00
367.70	2.59	367.20	0.00
367.80	3.24	367.20	0.00
367.90	3.95	367.20	0.00
368.00	4.70	367.20	0.00
368.10	5.50	367.20	0.00
368.20	6.34	367.20	0.00
368.30	7.21	367.20	0.00
368.40	8.14	367.20	0.00
368.50	9.06	367.20	0.00
368.60	10.00	367.20	0.00
368.70	10.95	367.20	0.00
368.80	11.90	367.20	0.00
368.90	12.82	367.20	0.00
369.00	13.75	367.20	0.00
369.10	14.62	367.20	0.00
369.20	15.45	367.20	0.00
369.30	16.24	367.20	0.00
369.40	16.93	367.20	0.00
369.50	17.51	367.20	0.00
369.60	18.03	367.20	0.00
369.70	18.54	367.20	0.00
369.80	19.06	367.20	0.00
369.90	19.57	367.20	0.00
370.00	20.07	367.20	0.00
370.10	20.58	367.20	0.00
370.20	21.07	367.20	0.00
370.30	21.56	367.20	0.00
370.40	22.04	367.20	0.00
370.50	22.51	367.20	0.00
370.60	22.99	367.20	0.00
370.70	23.46	367.20	0.00
370.80	23.91	367.20	0.00
370.90	24.36	367.20	0.00
371.00	24.81	367.20	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-0.58	367.30	0.00
367.00	-0.58	367.30	0.00
367.10	-0.58	367.30	0.00
367.20	-0.56	367.30	0.00
367.30	0.00	367.30	0.00
367.40	0.98	367.30	0.00
367.50	1.49	367.30	0.00
367.60	2.01	367.30	0.00
367.70	2.59	367.30	0.00
367.80	3.24	367.30	0.00
367.90	3.94	367.30	0.00
368.00	4.68	367.30	0.00
368.10	5.50	367.30	0.00
368.20	6.34	367.30	0.00
368.30	7.21	367.30	0.00
368.40	8.14	367.30	0.00
368.50	9.06	367.30	0.00
368.60	10.00	367.30	0.00
368.70	10.95	367.30	0.00
368.80	11.90	367.30	0.00
368.90	12.82	367.30	0.00
369.00	13.75	367.30	0.00
369.10	14.62	367.30	0.00
369.20	15.45	367.30	0.00
369.30	16.24	367.30	0.00
369.40	16.93	367.30	0.00
369.50	17.51	367.30	0.00
369.60	18.03	367.30	0.00
369.70	18.54	367.30	0.00
369.80	19.06	367.30	0.00
369.90	19.57	367.30	0.00
370.00	20.07	367.30	0.00
370.10	20.58	367.30	0.00
370.20	21.07	367.30	0.00
370.30	21.56	367.30	0.00
370.40	22.04	367.30	0.00
370.50	22.51	367.30	0.00
370.60	22.99	367.30	0.00
370.70	23.46	367.30	0.00
370.80	23.91	367.30	0.00
370.90	24.36	367.30	0.00
371.00	24.81	367.30	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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# Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-0.90	367.40	0.00
367.00	-0.90	367.40	0.00
367.10	-0.90	367.40	0.00
367.20	-0.90	367.40	0.00
367.30	-0.83	367.40	0.00
367.40	0.00	367.40	0.00
367.50	1.33	367.40	0.00
367.60	1.97	367.40	0.00
367.70	2.58	367.40	0.00
367.80	3.24	367.40	0.00
367.90	3.94	367.40	0.00
368.00	4.69	367.40	0.00
368.10	5.50	367.40	0.00
368.20	6.34	367.40	0.00
368.30	7.21	367.40	0.00
368.40	8.14	367.40	0.00
368.50	9.06	367.40	0.00
368.60	10.00	367.40	0.00
368.70	10.95	367.40	0.00
368.80	11.90	367.40	0.00
368.90	12.82	367.40	0.00
369.00	13.75	367.40	0.00
369.10	14.62	367.40	0.00
369.20	15.45	367.40	0.00
369.30	16.24	367.40	0.00
369.40	16.93	367.40	0.00
369.50	17.51	367.40	0.00
369.60	18.03	367.40	0.00
369.70	18.54	367.40	0.00
369.80	19.06	367.40	0.00
369.90	19.57	367.40	0.00
370.00	20.07	367.40	0.00
3/0.10	20.58	367.40	0.00
370.20	21.07	367.40	0.00
370.30	21.56	367.40	0.00
370.40	22.04	367.40	0.00
370.50	22.51	367.40	0.00
370.60	22.99	367.40	0.00
370.70	23.46	367.40	0.00
370.80	23.91	367.40	0.00
370.90	24.36	367.40	0.00
371.00	24.81	367.40	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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# Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-1.29	367.50	0.00
367.00	-1.29	367.50	0.00
367.10	-1.29	367.50	0.00
367.20	-1.29	367.50	0.00
367.30	-1.29	367.50	0.00
367.40	-1.14	367.50	0.00
367.50	0.00	367.50	0.00
367.60	1.71	367.50	0.00
367.70	2.50	367.50	0.00
367.80	3.21	367.50	0.00
367.90	3.93	367.50	0.00
368.00	4.68	367.50	0.00
368.10	5.50	367.50	0.00
368.20	6.34	367.50	0.00
368.30	7.21	367.50	0.00
368.40	8.14	367.50	0.00
368.50	9.06	367.50	0.00
368.60	10.00	367.50	0.00
368.70	10.95	367.50	0.00
368.80	11.90	367.50	0.00
368.90	12.82	367.50	0.00
369.00	13.75	367.50	0.00
369.10	14.62	367.50	0.00
369.20	15.45	367.50	0.00
369.30	16.24	367.50	0.00
369.40	16.93	367.50	0.00
369.50	17.51	367.50	0.00
369.60	18.03	367.50	0.00
369.70	18.54	367.50	0.00
369.80	19.06	367.50	0.00
369.90	19.57	367.50	0.00
370.00	20.07	367.50	0.00
370.10	20.58	367.50	0.00
370.20	21.07	367.50	0.00
370.30	21.56	367.50	0.00
370.40	22.04	367.50	0.00
370.50	22.51	367.50	0.00
370.60	22.99	367.50	0.00
370.70	23.46	367.50	0.00
370.80	23.91	367.50	0.00
370.90	24.36	367.50	0.00
371.00	24.81	367.50	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Contributing Structur
Culvert - 1

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## Return Event: 10 years Storm Event: 10

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
		267.60	0.00
366.90	-1.74	367.60	0.00
367.00	-1.74	367.60	0.00
367.10	-1.74	367.60	0.00
367.20	-1.74	367.60	0.00
367.30	-1.74	367.60	0.00
367.40	-1./3	367.60	0.00
367.50	-1.48	367.60	0.00
367.60	0.00	367.60	0.00
367.70	2.09	367.60	0.00
367.80	3.05	367.60	0.00
367.90	3.86	367.60	0.00
368.00	4.00	367.60	0.00
308.10	5.47	367.60	0.00
368.20	0.34	367.60	0.00
308.30	7.21	307.00	0.00
308.40	8.12	307.00	0.00
308.50	9.00	307.00	0.00
308.00	10.00	307.00	0.00
200.70	11.95	307.00	0.00
200.00	11.90	307.00	0.00
300.90	12.02	307.00	0.00
260.10	14.62	307.00	0.00
369.20	15.02	367.60	0.00
260.20	15.45	307.00	0.00
369.40	16.03	367.60	0.00
369.50	17.51	367.60	0.00
369.60	18.03	367.60	0.00
369.70	18.54	367.60	0.00
369.80	19.06	367.60	0.00
369.90	19.57	367.60	0.00
370.00	20.07	367.60	0.00
370.10	20.58	367.60	0.00
370.20	21.07	367.60	0.00
370.30	21.56	367.60	0.00
370.40	22.04	367.60	0.00
370.50	22.51	367.60	0.00
370.60	22.99	367.60	0.00
370.70	23,46	367.60	0.00
370.80	23.91	367.60	0.00
370.90	24.36	367.60	0.00
371.00	24.81	367.60	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-2.25	367.70	0.00
367.00	-2.25	367.70	0.00
367.10	-2.25	367.70	0.00
367.20	-2.25	367.70	0.00
367.30	-2.25	367.70	0.00
367.40	-2.25	367.70	0.00
367.50	-2.19	367.70	0.00
367.60	-1.81	367.70	0.00
367.70	0.00	367.70	0.00
367.80	2.48	367.70	0.00
367.90	3.59	367.70	0.00
368.00	4.54	367.70	0.00
368.10	5.43	367.70	0.00
368.20	6.30	367.70	0.00
368.30	7.20	367.70	0.00
368.40	8.12	367.70	0.00
368.50	9.04	367.70	0.00
368.60	9.99	367.70	0.00
368.70	10.95	367.70	0.00
368.80	11.90	367.70	0.00
368.90	12.82	367.70	0.00
369.00	13.75	367.70	0.00
369.10	14.62	367.70	0.00
369.20	15.45	367.70	0.00
369.30	16.24	367.70	0.00
369.40	16.93	367.70	0.00
369.50	17.51	367.70	0.00
369.60	18.03	367.70	0.00
369.70	18.54	367.70	0.00
369.80	19.06	367.70	0.00
369.90	19.57	367.70	0.00
370.00	20.07	367.70	0.00
370.10	20.58	367.70	0.00
370.20	21.07	367.70	0.00
370.30	21.56	367.70	0.00
370.40	22.04	367.70	0.00
370.50	22.51	367.70	0.00
370.60	22.99	367.70	0.00
370.70	23.46	367.70	0.00
370.80	23.91	367.70	0.00
370.90	24.36	367.70	0.00
371.00	24.81	367.70	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Contributing Structure
Culvert - 1

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-2.81	367.80	0.00
367.00	-2.81	367.80	0.00
367.10	-2.81	367.80	0.00
367.20	-2.81	367.80	0.00
367.30	-2.81	367.80	0.00
367.40	-2.81	367.80	0.00
367.50	-2.81	367.80	0.00
367.60	-2.68	367.80	0.00
367.70	-2.17	367.80	0.00
367.80	0.00	367.80	0.00
367.90	2.88	367.80	0.00
368.00	4.17	367.80	0.00
368.10	5.22	367.80	0.00
368.20	6.18	367.80	0.00
368.30	7.12	367.80	0.00
368.40	8.07	367.80	0.00
368.50	9.02	367.80	0.00
368.60	9.99	367.80	0.00
368.70	10.93	367.80	0.00
368.80	11.90	367.80	0.00
368.90	12.82	367.80	0.00
369.00	13.75	367.80	0.00
369.10	14.62	367.80	0.00
369.20	15.45	367.80	0.00
369.30	16.24	367.80	0.00
369.40	16.93	367.80	0.00
369.50	17.51	367.80	0.00
369.60	18.03	367.80	0.00
369.70	18.54	367.80	0.00
369.80	19.06	367.80	0.00
369.90	19.57	367.80	0.00
370.00	20.07	367.80	0.00
370.10	20.58	367.80	0.00
370.20	21.07	367.80	0.00
370.30	21.56	367.80	0.00
370.40	22.04	367.80	0.00
370.50	22.51	367.80	0.00
370.60	22.99	367.80	0.00
370.70	23.46	367.80	0.00
370.80	23.91	367.80	0.00
370.90	24.36	367.80	0.00
371.00	24.81	367.80	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-3.43	367.90	0.00
367.00	-3.43	367.90	0.00
367.10	-3.43	367.90	0.00
367.20	-3.43	367.90	0.00
367.30	-3.43	367.90	0.00
367.40	-3.43	367.90	0.00
367.50	-3.43	367.90	0.00
367.60	-3.42	367.90	0.00
367.70	-3.18	367.90	0.00
367.80	-2.53	367.90	0.00
367.90	0.00	367.90	0.00
368.00	3.25	367.90	0.00
368.10	4.71	367.90	0.00
368.20	5.86	367.90	0.00
368.30	6.93	367.90	0.00
368.40	7.93	367.90	0.00
368.50	8.93	367.90	0.00
368.60	9.91	367.90	0.00
368.70	10.90	367.90	0.00
368.80	11.86	367.90	0.00
368.90	12.81	367.90	0.00
369.00	13.75	367.90	0.00
369.10	14.62	367.90	0.00
369.20	15.45	367.90	0.00
369.30	16.24	367.90	0.00
369.40	16.93	367.90	0.00
369.50	17.51	367.90	0.00
369.60	18.03	367.90	0.00
369.70	18.54	367.90	0.00
369.80	19.06	367.90	0.00
369.90	19.57	367.90	0.00
370.00	20.07	367.90	0.00
370.10	20.58	367.90	0.00
370.20	21.07	367.90	0.00
370.30	21.56	367.90	0.00
370.40	22.04	367.90	0.00
370.50	22.51	367.90	0.00
370.60	22.99	367.90	0.00
370.70	23.46	367.90	0.00
370.80	23.91	367.90	0.00
370.90	24.36	367.90	0.00
371.00	24.81	367.90	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-4.10	368.00	0.00
367.00	-4.10	368.00	0.00
367.10	-4.10	368.00	0.00
367.20	-4.10	368.00	0.00
367.30	-4.10	368.00	0.00
367.40	-4.10	368.00	0.00
367.50	-4.10	368.00	0.00
367.60	-4.10	368.00	0.00
367.70	-4.03	368.00	0.00
367.80	-3.70	368.00	0.00
367.90	-2.91	368.00	0.00
368.00	0.00	368.00	0.00
368.10	3.64	368.00	0.00
368.20	5.22	368.00	0.00
368.30	6.49	368.00	0.00
368.40	7.64	368.00	0.00
368.50	8.72	368.00	0.00
368.60	9.76	368.00	0.00
368.70	10.77	368.00	0.00
368.80	11.77	368.00	0.00
368.90	12.74	368.00	0.00
369.00	13.69	368.00	0.00
369.10	14.59	368.00	0.00
369.20	15.44	368.00	0.00
369.30	16.24	368.00	0.00
369.40	16.93	368.00	0.00
369.50	17.51	368.00	0.00
369.60	18.03	368.00	0.00
369.70	18.54	368.00	0.00
369.80	19.06	368.00	0.00
369.90	19.57	368.00	0.00
370.00	20.07	368.00	0.00
370.10	20.58	368.00	0.00
370.20	21.07	368.00	0.00
370.30	21.56	368.00	0.00
370.40	22.04	368.00	0.00
370.50	22.51	368.00	0.00
370.60	22.99	368.00	0.00
370.70	23.46	368.00	0.00
370.80	23.91	368.00	0.00
370.90	24.36	368.00	0.00
371.00	24.81	368.00	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-4.79	368.10	0.00
367.00	-4.79	368.10	0.00
367.10	-4.79	368.10	0.00
367.20	-4.79	368.10	0.00
367.30	-4.79	368.10	0.00
367.40	-4.79	368.10	0.00
367.50	-4.79	368.10	0.00
367.60	-4.79	368.10	0.00
367.70	-4.79	368.10	0.00
367.80	-4.65	368.10	0.00
367.90	-4.20	368.10	0.00
368.00	-3.24	368.10	0.00
368.10	0.00	368.10	0.00
368.20	4.00	368.10	0.00
368.30	5.70	368.10	0.00
368.40	7.09	368.10	0.00
368.50	8.33	368.10	0.00
368.60	9.45	368.10	0.00
368.70	10.54	368.10	0.00
368.80	11.57	368.10	0.00
368.90	12.58	368.10	0.00
369.00	13.55	368.10	0.00
369.10	14.48	368.10	0.00
369.20	15.35	368.10	0.00
369.30	16.16	368.10	0.00
369.40	16.88	368.10	0.00
369.50	17.48	368.10	0.00
369.60	18.01	368.10	0.00
369.70	18.54	368.10	0.00
369.80	19.06	368.10	0.00
369.90	19.57	368.10	0.00
3/0.00	20.07	368.10	0.00
3/0.10	20.58	368.10	0.00
3/0.20	21.07	368.10	0.00
3/0.30	21.56	368.10	0.00
3/0.40	22.04	368.10	0.00
3/0.50	22.51	368.10	0.00
370.60	22.99	368.10	0.00
370.70	23.46	368.10	0.00
3/0.80	23.91	368.10	0.00
3/0.90	24.36	368.10	0.00
3/1.00	24.81	368.10	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation (ff)	(ft³/S)	(ft)	(π)
	E E2	260 20	0.00
300.90	-3.33	300.20	0.00
307.00	-3.33	269.20	0.00
367.10	-5.55	269.20	0.00
307.20	-3.33	269.20	0.00
307.30	-3.33	269.20	0.00
307.40	-3.33	269.20	0.00
307.50	-5.55	269.20	0.00
307.00	-3.33	269.20	0.00
307.70	-5.55	269.20	0.00
367.00	-5.31	368.20	0.00
307.90	-5.25	269.20	0.00
368.00	-4.70	368.20	0.00
308.10	-3.00	269.20	0.00
368.30	4.28	368.20	0.00
368.40	6.17	368.20	0.00
368 50	7.63	368.20	0.00
368.60	8.03	368.20	0.00
368 70	10 11	368.20	0.00
368.80	11.23	368.20	0.00
368.90	12.29	368.20	0.00
369.00	13.29	368.20	0.00
369.10	14.25	368.20	0.00
369.20	15.14	368.20	0.00
369.30	15.96	368.20	0.00
369.40	16.69	368.20	0.00
369.50	17.28	368.20	0.00
369.60	17.85	368.20	0.00
369.70	18.41	368.20	0.00
369.80	18.96	368.20	0.00
369.90	19.50	368.20	0.00
370.00	20.03	368.20	0.00
370.10	20.55	368.20	0.00
370.20	21.06	368.20	0.00
370.30	21.55	368.20	0.00
370.40	22.04	368.20	0.00
370.50	22.52	368.20	0.00
370.60	22.99	368.20	0.00
370.70	23.46	368.20	0.00
370.80	23.91	368.20	0.00
370.90	24.36	368.20	0.00
371.00	24.81	368.20	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structure
Culvert - 1

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# Return Event: 10 years Storm Event: 10

#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-6.29	368.30	0.00
367.00	-6.29	368.30	0.00
367.10	-6.29	368.30	0.00
367.20	-6.29	368.30	0.00
367.30	-6.29	368.30	0.00
367.40	-6.29	368.30	0.00
367.50	-6.29	368.30	0.00
367.60	-6.29	368.30	0.00
367.70	-6.29	368.30	0.00
367.80	-6.29	368.30	0.00
367.90	-6.20	368.30	0.00
368.00	-5.84	368.30	0.00
368.10	-5.17	368.30	0.00
368.20	-3.91	368.30	0.00
368.30	0.00	368.30	0.00
368.40	4.58	368.30	0.00
368.50	6.55	368.30	0.00
368.60	8.10	368.30	0.00
368.70	9.45	368.30	0.00
368.80	10.66	368.30	0.00
368.90	11.81	368.30	0.00
369.00	12.86	368.30	0.00
369.10	13.85	368.30	0.00
369.20	14.76	368.30	0.00
369.30	15.59	368.30	0.00
369.40	16.31	368.30	0.00
369.50	16.90	368.30	0.00
369.60	17.51	368.30	0.00
369.70	18.09	368.30	0.00
369.80	18.6/	368.30	0.00
369.90	19.26	368.30	0.00
370.00	19.81	368.30	0.00
3/0.10	20.36	368.30	0.00
370.20	20.90	368.30	0.00
370.30	21.42	368.30	0.00
3/0.40	21.93	368.30	0.00
3/0.50	22.43	368.30	0.00
370.60	22.93	368.30	0.00
370.70	23.41	368.30	0.00
3/0.80	23.88	368.30	0.00
3/0.90	24.35	368.30	0.00
3/1.00	24.79	368.30	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structu
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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-7.08	368.40	0.00
367.00	-7.08	368.40	0.00
367.10	-7.08	368.40	0.00
367.20	-7.08	368.40	0.00
367.30	-7.08	368.40	0.00
367.40	-7.08	368.40	0.00
367.50	-7.08	368.40	0.00
367.60	-7.08	368.40	0.00
367.70	-7.08	368.40	0.00
367.80	-7.08	368.40	0.00
367.90	-7.06	368.40	0.00
368.00	-6.87	368.40	0.00
368.10	-6.41	368.40	0.00
368.20	-5.63	368.40	0.00
368.30	-4.24	368.40	0.00
368.40	0.00	368.40	0.00
368.50	4.82	368.40	0.00
368.60	6.85	368.40	0.00
368.70	8.45	368.40	0.00
368.80	9.85	368.40	0.00
368.90	11.06	368.40	0.00
369.00	12.20	368.40	0.00
369.10	13.22	368.40	0.00
369.20	14.16	368.40	0.00
369.30	14.96	368.40	0.00
369.40	15.61	368.40	0.00
369.50	16.26	368.40	0.00
369.60	16.91	368.40	0.00
369.70	17.56	368.40	0.00
369.80	18.19	368.40	0.00
369.90	18.81	368.40	0.00
370.00	19.41	368.40	0.00
370.10	19.98	368.40	0.00
370.20	20.55	368.40	0.00
370.30	21.11	368.40	0.00
370.40	21.65	368.40	0.00
370.50	22.17	368.40	0.00
370.60	22.69	368.40	0.00
370.70	23.19	368.40	0.00
370.80	23.69	368.40	0.00
370.90	24.18	368.40	0.00
371.00	24.65	368.40	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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### Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(π)	7.07	200 50	0.00
366.90	-/.8/	308.50	0.00
367.00	-7.87	368.50	0.00
367.10	-7.87	368.50	0.00
367.20	-7.87	308.50	0.00
307.30	-7.87	308.50	0.00
367.40	-7.87	368.50	0.00
367.50	-7.87	368.50	0.00
307.00	-7.87	308.50	0.00
367.70	-7.87	368.50	0.00
367.80	-7.87	368.50	0.00
367.90	-7.87	308.50	0.00
308.00	-7.80	308.50	0.00
308.10	-7.51	308.50	0.00
368.20	-6.94	368.50	0.00
308.30	-0.01	308.50	0.00
308.40	-4.48	308.50	0.00
308.50	0.00	308.50	0.00
368.60	4.94	368.50	0.00
308.70	7.00	308.50	0.00
308.80	8.04	308.50	0.00
308.90	10.00	308.50	0.00
309.00	11.10	200.50	0.00
369.10	12.21	308.50	0.00
369.20	13.03	308.50	0.00
369.30	13.74	308.50	0.00
309.40	14.30	200.50	0.00
369.50	15.30	308.50	0.00
309.00	10.12	308.50	0.00
209.70	10.04	200.50	0.00
309.00	17.55	200.50	0.00
309.90	18.18	308.50	0.00
3/0.00	10.82	308.50	0.00
3/0.10	19.44	308.50	0.00
370.20	20.04	308.50	0.00
370.30	20.62	308.50 260 ED	0.00
370.40 370 FO	21.18	208.2U	0.00
370.50	21./4	308.50	0.00
370.00	22.27	308.50	0.00
3/0./0	22.79	308.50	0.00
3/0.80	23.30	308.50	0.00
3/0.90	23.80	368.50	0.00
3/1.00	24.30	308.50	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(tt)			
366.90	-8.65	368.60	0.00
367.00	-8.65	368.60	0.00
367.10	-8.65	368.60	0.00
367.20	-8.65	368.60	0.00
367.30	-8.65	368.60	0.00
367.40	-8.65	368.60	0.00
367.50	-8.65	368.60	0.00
367.60	-8.65	368.60	0.00
367.70	-8.65	368.60	0.00
367.80	-8.65	368.60	0.00
367.90	-8.65	368.60	0.00
368.00	-8.64	368.60	0.00
368.10	-8.48	368.60	0.00
368.20	-8.08	368.60	0.00
368.30	-7.39	368.60	0.00
368.40	-6.34	368.60	0.00
368.50	-4.63	368.60	0.00
368.60	0.00	368.60	0.00
368.70	4.95	368.60	0.00
368.80	6.98	368.60	0.00
368.90	8.53	368.60	0.00
369.00	9.82	368.60	0.00
369.10	10.89	368.60	0.00
369.20	11.90	368.60	0.00
369.30	12.85	368.60	0.00
369.40	13.74	368.60	0.00
369.50	14.58	368.60	0.00
369.60	15.36	368.60	0.00
369.70	16.12	368.60	0.00
369.80	16.83	368.60	0.00
369.90	17.52	368.60	0.00
370.00	18.19	368.60	0.00
370.10	18.83	368.60	0.00
370.20	19.44	368.60	0.00
370.30	20.03	368.60	0.00
370.40	20.62	368.60	0.00
370.50	21.18	368.60	0.00
370.60	21.73	368.60	0.00
370.70	22.27	368.60	0.00
370.80	22.79	368.60	0.00
370.90	23.31	368.60	0.00
371.00	23.80	368.60	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
	0.42	260 70	0.00
300.90	-9.42	260.70 269.70	0.00
307.00	-9.42	269.70	0.00
267.10	-9.42	300.70	0.00
267.20	-9.42	269 70	0.00
307.30	-9.42	269.70	0.00
307.40	-9.42	269.70	0.00
307.30	-9.42	269 70	0.00
307.00	-9.42	269 70	0.00
267.90	-9.42	269 70	0.00
367.00	-9.42	368 70	0.00
307.90	-9.42	269 70	0.00
360.00	-9.42	260.70	0.00
300.10	-9.55	269 70	0.00
368 30	-9.00	368 70	0.00
368.40	-7 75	368 70	0.00
368 50	-6.56	368 70	0.00
368.60	-4 77	368 70	0.00
368 70	0.00	368 70	0.00
368.80	4 89	368 70	0.00
368.90	6.93	368 70	0.00
369.00	8.45	368.70	0.00
369.10	9.71	368.70	0.00
369.20	10.86	368.70	0.00
369.30	11.91	368.70	0.00
369.40	12.86	368.70	0.00
369.50	13.74	368.70	0.00
369.60	14.57	368.70	0.00
369.70	15.37	368.70	0.00
369.80	16.12	368.70	0.00
369.90	16.84	368.70	0.00
370.00	17.52	368.70	0.00
370.10	18.19	368.70	0.00
370.20	18.82	368.70	0.00
370.30	19.43	368.70	0.00
370.40	20.04	368.70	0.00
370.50	20.62	368.70	0.00
370.60	21.18	368.70	0.00
370.70	21.73	368.70	0.00
370.80	22.26	368.70	0.00
370.90	22.80	368.70	0.00
371.00	23.31	368.70	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-10.16	368.80	0.00
367.00	-10.16	368.80	0.00
367.10	-10.16	368.80	0.00
367.20	-10.16	368.80	0.00
367.30	-10.16	368.80	0.00
367.40	-10.16	368.80	0.00
367.50	-10.16	368.80	0.00
367.60	-10.16	368.80	0.00
367.70	-10.16	368.80	0.00
367.80	-10.16	368.80	0.00
367.90	-10.16	368.80	0.00
368.00	-10.16	368.80	0.00
368.10	-10.13	368.80	0.00
368.20	-9.95	368.80	0.00
368.30	-9.56	368.80	0.00
368.40	-8.94	368.80	0.00
368.50	-8.01	368.80	0.00
368.60	-6.72	368.80	0.00
368.70	-4.82	368.80	0.00
368.80	0.00	368.80	0.00
368.90	4.85	368.80	0.00
369.00	6.87	368.80	0.00
369.10	8.43	368.80	0.00
369.20	9.72	368.80	0.00
369.30	10.86	368.80	0.00
369.40	11.91	368.80	0.00
369.50	12.86	368.80	0.00
369.60	13.74	368.80	0.00
369.70	14.58	368.80	0.00
369.80	15.37	368.80	0.00
369.90	16.12	368.80	0.00
370.00	16.83	368.80	0.00
370.10	17.52	368.80	0.00
370.20	18.19	368.80	0.00
370.30	18.82	368.80	0.00
370.40	19.44	368.80	0.00
370.50	20.04	368.80	0.00
370.60	20.62	368.80	0.00
370.70	21.18	368.80	0.00
370.80	21.73	368.80	0.00
370.90	22.27	368.80	0.00
371.00	22.79	368.80	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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### Return Event: 10 years Storm Event: 10

#### Composite Outflow Summary

Water Surface Flevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)	(11/1)		(,
366.90	-10.87	368.90	0.00
367.00	-10.87	368.90	0.00
367.10	-10.87	368.90	0.00
367.20	-10.87	368.90	0.00
367.30	-10.87	368.90	0.00
367.40	-10.87	368.90	0.00
367.50	-10.87	368.90	0.00
367.60	-10.87	368.90	0.00
367.70	-10.87	368.90	0.00
367.80	-10.87	368.90	0.00
367.90	-10.87	368.90	0.00
368.00	-10.87	368.90	0.00
368.10	-10.87	368.90	0.00
368.20	-10.78	368.90	0.00
368.30	-10.49	368.90	0.00
368.40	-9.99	368.90	0.00
368.50	-9.25	368.90	0.00
368.60	-8.20	368.90	0.00
368.70	-6.82	368.90	0.00
368.80	-4.86	368.90	0.00
368.90	0.00	368.90	0.00
369.00	4.87	368.90	0.00
369.10	6.86	368.90	0.00
369.20	8.42	368.90	0.00
369.30	9.73	368.90	0.00
369.40	10.87	368.90	0.00
369.50	11.91	368.90	0.00
369.60	12.86	368.90	0.00
369.70	13.74	368.90	0.00
369.80	14.58	368.90	0.00
369.90	15.37	368.90	0.00
370.00	16.12	368.90	0.00
370.10	16.84	368.90	0.00
370.20	17.52	368.90	0.00
370.30	18.18	368.90	0.00
370.40	18.82	368.90	0.00
370.50	19.44	368.90	0.00
370.60	20.04	368.90	0.00
370.70	20.61	368.90	0.00
370.80	21.18	368.90	0.00
370.90	21.74	368.90	0.00
371.00	22.27	368.90	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(π)	11 50	200.00	0.00
366.90	-11.58	369.00	0.00
367.00	-11.58	369.00	0.00
367.10	-11.58	369.00	0.00
367.20	-11.58	369.00	0.00
367.30	-11.58	369.00	0.00
367.40	-11.58	369.00	0.00
367.50	-11.58	369.00	0.00
367.60	-11.58	369.00	0.00
367.70	-11.58	369.00	0.00
367.80	-11.58	369.00	0.00
307.90	-11.58	369.00	0.00
368.00	-11.58	369.00	0.00
368.10	-11.58	369.00	0.00
308.20	-11.54	309.00	0.00
269.40	-11.54	309.00	0.00
269 50	-10.94	369.00	0.00
269.60	-10.52	309.00	0.00
269 70	-9.4/	369.00	0.00
269.90	-0.J+ 6 97	309.00	0.00
269.00	-0.07	309.00	0.00
369.00	-4.80	369.00	0.00
369.10	0.00 4 84	369.00	0.00
369.20	6.88	369.00	0.00
369.20	0.00 8 41	369.00	0.00
369.40	9.72	369.00	0.00
369.50	10.86	369.00	0.00
369.60	11.90	369.00	0.00
369.70	12.85	369.00	0.00
369.80	13.74	369.00	0.00
369.90	14.58	369.00	0.00
370.00	15.37	369.00	0.00
370.10	16.12	369.00	0.00
370.20	16.84	369.00	0.00
370.30	17.52	369.00	0.00
370.40	18.18	369.00	0.00
370.50	18.82	369.00	0.00
370.60	19.43	369.00	0.00
370.70	20.04	369.00	0.00
370.80	20.62	369.00	0.00
370.90	21.18	369.00	0.00
371.00	21.73	369.00	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-12.25	369.10	0.00
367.00	-12.25	369.10	0.00
367.10	-12.25	369.10	0.00
367.20	-12.25	369.10	0.00
367.30	-12.25	369.10	0.00
367.40	-12.25	369.10	0.00
367.50	-12.25	369.10	0.00
367.60	-12.25	369.10	0.00
367.70	-12.25	369.10	0.00
367.80	-12.25	369.10	0.00
367.90	-12.25	369.10	0.00
368.00	-12.25	369.10	0.00
368.10	-12.25	369.10	0.00
368.20	-12.25	369.10	0.00
368.30	-12.12	369.10	0.00
368.40	-11.80	369.10	0.00
368.50	-11.30	369.10	0.00
368.60	-10.56	369.10	0.00
368.70	-9.61	369.10	0.00
368.80	-8.39	369.10	0.00
368.90	-6.87	369.10	0.00
369.00	-4.86	369.10	0.00
369.10	0.00	369.10	0.00
369.20	4.85	369.10	0.00
369.30	6.87	369.10	0.00
369.40	8.42	369.10	0.00
369.50	9.72	369.10	0.00
369.60	10.86	369.10	0.00
369.70	11.90	369.10	0.00
369.80	12.86	369.10	0.00
369.90	13.74	369.10	0.00
370.00	14.58	369.10	0.00
370.10	15.37	369.10	0.00
370.20	16.12	369.10	0.00
370.30	16.84	369.10	0.00
370.40	17.53	369.10	0.00
370.50	18.18	369.10	0.00
370.60	18.82	369.10	0.00
370.70	19.43	369.10	0.00
370.80	20.03	369.10	0.00
370.90	20.62	369.10	0.00
371.00	21.18	369.10	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
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# Return Event: 10 years Storm Event: 10

#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(tt)	T		
366.90	-12.92	369.20	0.00
367.00	-12.92	369.20	0.00
367.10	-12.92	369.20	0.00
367.20	-12.92	369.20	0.00
367.30	-12.92	369.20	0.00
367.40	-12.92	369.20	0.00
367.50	-12.92	369.20	0.00
367.60	-12.92	369.20	0.00
367.70	-12.92	369.20	0.00
367.80	-12.92	369.20	0.00
367.90	-12.92	369.20	0.00
368.00	-12.92	369.20	0.00
368.10	-12.92	369.20	0.00
368.20	-12.92	369.20	0.00
368.30	-12.85	369.20	0.00
368.40	-12.61	369.20	0.00
368.50	-12.18	369.20	0.00
368.60	-11.56	369.20	0.00
368.70	-10.73	369.20	0.00
368.80	-9.70	369.20	0.00
368.90	-8.42	369.20	0.00
369.00	-6.87	369.20	0.00
369.10	-4.86	369.20	0.00
369.20	0.00	369.20	0.00
369.30	4.88	369.20	0.00
369.40	6.87	369.20	0.00
369.50	8.42	369.20	0.00
369.60	9.71	369.20	0.00
369.70	10.88	369.20	0.00
369.80	11.91	369.20	0.00
369.90	12.87	369.20	0.00
370.00	13.75	369.20	0.00
370.10	14.58	369.20	0.00
370.20	15.37	369.20	0.00
370.30	16.12	369.20	0.00
370.40	16.84	369.20	0.00
370.50	17.52	369.20	0.00
370.60	18.19	369.20	0.00
370.70	18.82	369.20	0.00
370.80	19.43	369.20	0.00
370.90	20.04	369.20	0.00
371.00	20.62	369.20	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
(ft)	(119/5)	(11)	(11)
366.90	-13.58	369.30	0.00
367.00	-13.58	369.30	0.00
367.10	-13.58	369.30	0.00
367.20	-13.58	369.30	0.00
367.30	-13.58	369.30	0.00
367.40	-13.58	369.30	0.00
367.50	-13.58	369.30	0.00
367.60	-13.58	369.30	0.00
367.70	-13.58	369.30	0.00
367.80	-13.58	369.30	0.00
367.90	-13.58	369.30	0.00
368.00	-13.58	369.30	0.00
368.10	-13.58	369.30	0.00
368.20	-13.58	369.30	0.00
368.30	-13.55	369.30	0.00
368.40	-13.38	369.30	0.00
368.50	-13.02	369.30	0.00
368.60	-12.48	369.30	0.00
368.70	-11.75	369.30	0.00
368.80	-10.82	369.30	0.00
368.90	-9.73	369.30	0.00
369.00	-8.42	369.30	0.00
369.10	-6.87	369.30	0.00
369.20	-4.86	369.30	0.00
369.30	0.00	369.30	0.00
369.40	4.86	369.30	0.00
369.50	6.86	369.30	0.00
369.60	8.41	369.30	0.00
369.70	9.72	369.30	0.00
369.80	10.87	369.30	0.00
369.90	11.90	369.30	0.00
370.00	12.86	369.30	0.00
370.10	13.75	369.30	0.00
370.20	14.58	369.30	0.00
370.30	15.36	369.30	0.00
370.40	16.12	369.30	0.00
370.50	16.83	369.30	0.00
370.60	17.52	369.30	0.00
370.70	18.19	369.30	0.00
3/0.80	18.82	369.30	0.00
3/0.90	19.44	369.30	0.00
3/1.00	20.04	369.30	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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# Return Event: 10 years Storm Event: 10

#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation (ff)	(ft³/S)	(π)	(π)
	-14 22	360 40	0.00
367.00	-14 22	369.40	0.00
367.10	-14 22	369.40	0.00
367.20	-14.22	369.40	0.00
367.30	-14.22	369.40	0.00
367.40	-14.22	369.40	0.00
367.50	-14.22	369.40	0.00
367.60	-14.22	369.40	0.00
367.70	-14.22	369.40	0.00
367.80	-14.22	369.40	0.00
367.90	-14.22	369.40	0.00
368.00	-14.22	369.40	0.00
368.10	-14.22	369.40	0.00
368.20	-14.22	369.40	0.00
368.30	-14.22	369.40	0.00
368.40	-14.09	369.40	0.00
368.50	-13.80	369.40	0.00
368.60	-13.33	369.40	0.00
368.70	-12.68	369.40	0.00
368.80	-11.86	369.40	0.00
368.90	-10.87	369.40	0.00
369.00	-9.73	369.40	0.00
369.10	-8.42	369.40	0.00
369.20	-6.87	369.40	0.00
369.30	-4.86	369.40	0.00
369.40	0.00	369.40	0.00
369.50	4.88	369.40	0.00
369.60	6.86	369.40	0.00
369.70	8.43	369.40	0.00
369.80	9.72	369.40	0.00
369.90	10.87	369.40	0.00
370.00	11.90	369.40	0.00
370.10	12.86	369.40	0.00
370.20	13./4	369.40	0.00
370.30	14.58	369.40	0.00
370.40	15.3/	369.40	0.00
370.50	16.12	309.40	0.00
370.00		309.40	0.00
3/0./0	1/.51	309.40	0.00
370.00	10.19	260 40	0.00
370.50	10.02	369.40 369.40	

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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### Return Event: 10 years Storm Event: 10

#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
	14.04	260 50	0.00
300.90	-14.84	309.50	0.00
367.00	-14.84	309.50	0.00
367.10	-14.84	369.50	0.00
307.20	-14.84	309.50	0.00
367.30	-14.84	309.50	0.00
367.40	-14.84	369.50	0.00
307.50	-14.84	309.50	0.00
307.00	-14.84	309.50	0.00
367.70	-14.84	309.50	0.00
307.80	-14.84	309.50	0.00
307.90	-14.04	309.50	0.00
300.00	-14.04	309.50	0.00
300.10	-14.04	309.50	0.00
308.20	-14.84	369.50	0.00
300.30	-14.04	309.30	0.00
269 50	-14.77	309.30	0.00
300.30	-14.55	309.30	0.00
368 70	-14.15	369.50	0.00
368.80	-13.55	369.50	0.00
368.00	-12.00	369.50	0.00
369.00	-11.90	369.50	0.00
369.10	-9.73	369.50	0.00
369.20	-8.42	369.50	0.00
369.20	-6.87	369.50	0.00
369.40	-4.86	369.50	0.00
369.50	0.00	369.50	0.00
369.60	4 84	369.50	0.00
369.70	6.87	369.50	0.00
369.80	8.42	369.50	0.00
369.90	9.73	369.50	0.00
370.00	10.86	369.50	0.00
370.10	11.90	369.50	0.00
370.20	12.86	369.50	0.00
370.30	13.75	369.50	0.00
370.40	14.58	369.50	0.00
370.50	15.36	369.50	0.00
370.60	16.11	369.50	0.00
370.70	16.83	369.50	0.00
370.80	17.52	369.50	0.00
370.90	18.18	369.50	0.00
371.00	18.82	369.50	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			( -7
366.90	-15.46	369.60	0.00
367.00	-15.46	369.60	0.00
367.10	-15.46	369.60	0.00
367.20	-15.46	369.60	0.00
367.30	-15.46	369.60	0.00
367.40	-15.46	369.60	0.00
367.50	-15.46	369.60	0.00
367.60	-15.46	369.60	0.00
367.70	-15.46	369.60	0.00
367.80	-15.46	369.60	0.00
367.90	-15.46	369.60	0.00
368.00	-15.46	369.60	0.00
368.10	-15.46	369.60	0.00
368.20	-15.46	369.60	0.00
368.30	-15.46	369.60	0.00
368.40	-15.43	369.60	0.00
368.50	-15.23	369.60	0.00
368.60	-14.88	369.60	0.00
368.70	-14.36	369.60	0.00
368.80	-13.69	369.60	0.00
368.90	-12.85	369.60	0.00
369.00	-11.90	369.60	0.00
369.10	-10.87	369.60	0.00
369.20	-9.73	369.60	0.00
369.30	-8.42	369.60	0.00
369.40	-6.87	369.60	0.00
369.50	-4.86	369.60	0.00
369.60	0.00	369.60	0.00
369.70	4.85	369.60	0.00
369.80	6.87	369.60	0.00
369.90	8.41	369.60	0.00
370.00	9.73	369.60	0.00
370.10	10.87	369.60	0.00
370.20	11.90	369.60	0.00
370.30	12.87	369.60	0.00
370.40	13.75	369.60	0.00
370.50	14.58	369.60	0.00
370.60	15.36	369.60	0.00
370.70	16.12	369.60	0.00
370.80	16.83	369.60	0.00
370.90	17.52	369.60	0.00
371.00	18.19	369.60	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structur
Culvert - 1

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-16.06	369.70	0.00
367.00	-16.06	369.70	0.00
367.10	-16.06	369.70	0.00
367.20	-16.06	369.70	0.00
367.30	-16.06	369.70	0.00
367.40	-16.06	369.70	0.00
367.50	-16.06	369.70	0.00
367.60	-16.06	369.70	0.00
367.70	-16.06	369.70	0.00
367.80	-16.06	369.70	0.00
367.90	-16.06	369.70	0.00
368.00	-16.06	369.70	0.00
368.10	-16.06	369.70	0.00
368.20	-16.06	369.70	0.00
368.30	-16.06	369.70	0.00
368.40	-16.05	369.70	0.00
368.50	-15.90	369.70	0.00
368.60	-15.59	369.70	0.00
368.70	-15.14	369.70	0.00
368.80	-14.52	369.70	0.00
368.90	-13.74	369.70	0.00
369.00	-12.85	369.70	0.00
369.10	-11.90	369.70	0.00
369.20	-10.87	369.70	0.00
369.30	-9.73	369.70	0.00
369.40	-8.42	369.70	0.00
369.50	-6.87	369.70	0.00
369.60	-4.86	369.70	0.00
369.70	0.00	369.70	0.00
369.80	4.85	369.70	0.00
369.90	6.87	369.70	0.00
370.00	8.41	369.70	0.00
370.10	9.73	369.70	0.00
370.20	10.87	369.70	0.00
370.30	11.91	369.70	0.00
370.40	12.86	369.70	0.00
370.50	13.75	369.70	0.00
370.60	14.58	369.70	0.00
370.70	15.37	369.70	0.00
370.80	16.12	369.70	0.00
370.90	16.84	369.70	0.00
371.00	17.52	369.70	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-16.64	369.80	0.00
367.00	-16.64	369.80	0.00
367.10	-16.64	369.80	0.00
367.20	-16.64	369.80	0.00
367.30	-16.64	369.80	0.00
367.40	-16.64	369.80	0.00
367.50	-16.64	369.80	0.00
367.60	-16.64	369.80	0.00
367.70	-16.64	369.80	0.00
367.80	-16.64	369.80	0.00
367.90	-16.64	369.80	0.00
368.00	-16.64	369.80	0.00
368.10	-16.64	369.80	0.00
368.20	-16.64	369.80	0.00
368.30	-16.64	369.80	0.00
368.40	-16.64	369.80	0.00
368.50	-16.55	369.80	0.00
368.60	-16.28	369.80	0.00
368.70	-15.85	369.80	0.00
368.80	-15.29	369.80	0.00
368.90	-14.58	369.80	0.00
369.00	-13.74	369.80	0.00
369.10	-12.85	369.80	0.00
369.20	-11.90	369.80	0.00
369.30	-10.87	369.80	0.00
369.40	-9.73	369.80	0.00
369.50	-8.42	369.80	0.00
369.60	-6.87	369.80	0.00
369.70	-4.86	369.80	0.00
369.80	0.00	369.80	0.00
369.90	4.87	369.80	0.00
370.00	6.88	369.80	0.00
370.10	8.42	369.80	0.00
370.20	9.73	369.80	0.00
370.30	10.86	369.80	0.00
370.40	11.90	369.80	0.00
370.50	12.86	369.80	0.00
370.60	13.75	369.80	0.00
370.70	14.58	369.80	0.00
370.80	15.37	369.80	0.00
370.90	16.11	369.80	0.00
371.00	16.84	369.80	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structu
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# Return Event: 10 years Storm Event: 10

#### Composite Outflow Summary

Water Surface Elevation	Flow (ft <sup>3</sup> /s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)		( )	( )
366.90	-17.21	369.90	0.00
367.00	-17.21	369.90	0.00
367.10	-17.21	369.90	0.00
367.20	-17.21	369.90	0.00
367.30	-17.21	369.90	0.00
367.40	-17.21	369.90	0.00
367.50	-17.21	369.90	0.00
367.60	-17.21	369.90	0.00
367.70	-17.21	369.90	0.00
367.80	-17.21	369.90	0.00
367.90	-17.21	369.90	0.00
368.00	-17.21	369.90	0.00
368.10	-17.21	369.90	0.00
368.20	-17.21	369.90	0.00
368.30	-17.21	369.90	0.00
368.40	-17.21	369.90	0.00
368.50	-17.15	369.90	0.00
368.60	-16.94	369.90	0.00
368.70	-16.57	369.90	0.00
368.80	-16.03	369.90	0.00
368.90	-15.37	369.90	0.00
369.00	-14.58	369.90	0.00
369.10	-13.74	369.90	0.00
369.20	-12.85	369.90	0.00
369.30	-11.90	369.90	0.00
369.40	-10.87	369.90	0.00
369.50	-9.73	369.90	0.00
369.60	-8.42	369.90	0.00
369.70	-6.87	369.90	0.00
369.80	-4.86	369.90	0.00
369.90	0.00	369.90	0.00
370.00	4.84	369.90	0.00
370.10	6.89	369.90	0.00
370.20	8.41	369.90	0.00
370.30	9.73	369.90	0.00
370.40	10.87	369.90	0.00
370.50	11.90	369.90	0.00
370.60	12.86	369.90	0.00
370.70	13.75	369.90	0.00
370.80	14.57	369.90	0.00
370.90	15.36	369.90	0.00
371.00	16.11	369.90	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 284 of 448

#### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 10 years Storm Event: 10

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-17.79	370.00	0.00
367.00	-17.79	370.00	0.00
367.10	-17.79	370.00	0.00
367.20	-17.79	370.00	0.00
367.30	-17.79	370.00	0.00
367.40	-17.79	370.00	0.00
367.50	-17.79	370.00	0.00
367.60	-17.79	370.00	0.00
367.70	-17.79	370.00	0.00
367.80	-17.79	370.00	0.00
367.90	-17.79	370.00	0.00
368.00	-17.79	370.00	0.00
368.10	-17.79	370.00	0.00
368.20	-17.79	370.00	0.00
368.30	-17.79	370.00	0.00
368.40	-17.79	370.00	0.00
368.50	-17.75	370.00	0.00
368.60	-17.57	370.00	0.00
368.70	-17.24	370.00	0.00
368.80	-16.75	370.00	0.00
368.90	-16.12	370.00	0.00
369.00	-15.37	370.00	0.00
369.10	-14.58	370.00	0.00
369.20	-13.74	370.00	0.00
369.30	-12.85	370.00	0.00
369.40	-11.90	370.00	0.00
369.50	-10.87	370.00	0.00
369.60	-9.73	370.00	0.00
369.70	-8.42	370.00	0.00
369.80	-6.87	370.00	0.00
369.90	-4.86	370.00	0.00
370.00	0.00	370.00	0.00
370.10	4.84	370.00	0.00
370.20	6.88	370.00	0.00
370.30	8.43	370.00	0.00
370.40	9.72	370.00	0.00
370.50	10.87	370.00	0.00
370.60	11.90	370.00	0.00
370.70	12.86	370.00	0.00
370.80	13.74	370.00	0.00
370.90	14.58	370.00	0.00
371.00	15.37	370.00	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 286 of 448

#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow (ft3/c)	Tailwater Elevation	Convergence Error
(ft)	(11/3)	(10)	(10)
366.90	-18.33	370.10	0.00
367.00	-18.33	370.10	0.00
367.10	-18.33	370.10	0.00
367.20	-18.33	370.10	0.00
367.30	-18.33	370.10	0.00
367.40	-18.33	370.10	0.00
367.50	-18.33	370.10	0.00
367.60	-18.33	370.10	0.00
367.70	-18.33	370.10	0.00
367.80	-18.33	370.10	0.00
367.90	-18.33	370.10	0.00
368.00	-18.33	370.10	0.00
368.10	-18.33	370.10	0.00
368.20	-18.33	370.10	0.00
368.30	-18.33	370.10	0.00
368.40	-18.33	370.10	0.00
368.50	-18.32	370.10	0.00
368.60	-18.17	370.10	0.00
368.70	-17.88	370.10	0.00
368.80	-17.43	370.10	0.00
368.90	-16.83	370.10	0.00
369.00	-16.12	370.10	0.00
369.10	-15.37	370.10	0.00
369.20	-14.58	370.10	0.00
369.30	-13.74	370.10	0.00
369.40	-12.85	370.10	0.00
369.50	-11.90	370.10	0.00
369.60	-10.87	370.10	0.00
369.70	-9.73	370.10	0.00
369.80	-8.42	370.10	0.00
369.90	-6.87	370.10	0.00
370.00	-4.86	370.10	0.00
370.10	0.00	370.10	0.00
370.20	4.88	370.10	0.00
370.30	6.86	370.10	0.00
370.40	8.41	370.10	0.00
370.50	9.71	370.10	0.00
370.60	10.87	370.10	0.00
370.70	11.91	370.10	0.00
370.80	12.86	370.10	0.00
370.90	13.74	370.10	0.00
371.00	14.58	370.10	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structur
Culvert - 1

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow (ft3/c)	Tailwater Elevation	Convergence Error
(ff)	(11/3)	(10)	(10)
366.90	-18.87	370.20	0.00
367.00	-18.87	370.20	0.00
367.10	-18.87	370.20	0.00
367.20	-18.87	370.20	0.00
367.30	-18.87	370.20	0.00
367,40	-18.87	370.20	0.00
367.50	-18.87	370.20	0.00
367.60	-18.87	370.20	0.00
367.70	-18.87	370.20	0.00
367.80	-18.87	370.20	0.00
367.90	-18.87	370.20	0.00
368.00	-18.87	370.20	0.00
368.10	-18.87	370.20	0.00
368.20	-18.87	370.20	0.00
368.30	-18.87	370.20	0.00
368.40	-18.87	370.20	0.00
368.50	-18.87	370.20	0.00
368.60	-18.76	370.20	0.00
368.70	-18.50	370.20	0.00
368.80	-18.08	370.20	0.00
368.90	-17.52	370.20	0.00
369.00	-16.83	370.20	0.00
369.10	-16.12	370.20	0.00
369.20	-15.37	370.20	0.00
369.30	-14.58	370.20	0.00
369.40	-13.74	370.20	0.00
369.50	-12.85	370.20	0.00
369.60	-11.90	370.20	0.00
369.70	-10.87	370.20	0.00
369.80	-9.73	370.20	0.00
369.90	-8.42	370.20	0.00
370.00	-6.87	370.20	0.00
370.10	-4.86	370.20	0.00
370.20	0.00	370.20	0.00
370.30	4.85	370.20	0.00
370.40	6.88	370.20	0.00
370.50	8.43	370.20	0.00
370.60	9.72	370.20	0.00
370.70	10.86	370.20	0.00
370.80	11.91	370.20	0.00
370.90	12.86	370.20	0.00
371.00	13.74	370.20	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

# Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(IT)	10 42	0.0 0.0	0.00
366.90	-19.42	370.30	0.00
367.00	-19.42	370.30	0.00
367.10	-19.42	370.30	0.00
367.20	-19.42	370.30	0.00
367.30	-19.42	370.30	0.00
367.40	-19.42	370.30	0.00
367.50	-19.42	370.30	0.00
367.60	-19.42	370.30	0.00
367.70	-19.42	370.30	0.00
367.80	-19.42	370.30	0.00
367.90	-19.42	370.30	0.00
308.00	-19.42	370.30	0.00
308.10	-19.42	370.30	0.00
300.20	-19.42	370.30	0.00
308.30	-19.42	270.20	0.00
368 50	-19.42	370.30	0.00
308.30	-19.42	270.20	0.00
368 70	-19.54	370.30	0.00
368.80	-19.10	370.30	0.00
368.00	-10.75	370.30	0.00
369.00	-17.52	370.30	0.00
369.10	-16.83	370.30	0.00
369.20	-16.12	370.30	0.00
369.30	-15 37	370.30	0.00
369.40	-14.58	370.30	0.00
369.50	-13.74	370.30	0.00
369.60	-12.85	370.30	0.00
369.70	-11.90	370.30	0.00
369.80	-10.87	370.30	0.00
369,90	-9.73	370.30	0.00
370.00	-8.42	370.30	0.00
370.10	-6.87	370.30	0.00
370.20	-4.86	370.30	0.00
370.30	0.00	370.30	0.00
370.40	4.87	370.30	0.00
370.50	6.89	370.30	0.00
370.60	8.40	370.30	0.00
370.70	9.71	370.30	0.00
370.80	10.87	370.30	0.00
370.90	11.90	370.30	0.00
371.00	12.85	370.30	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-19.94	370.40	0.00
367.00	-19.94	370.40	0.00
367.10	-19.94	370.40	0.00
367.20	-19.94	370.40	0.00
367.30	-19.94	370.40	0.00
367.40	-19.94	370.40	0.00
367.50	-19.94	370.40	0.00
367.60	-19.94	370.40	0.00
367.70	-19.94	370.40	0.00
367.80	-19.94	370.40	0.00
367.90	-19.94	370.40	0.00
368.00	-19.94	370.40	0.00
368.10	-19.94	370.40	0.00
368.20	-19.94	370.40	0.00
368.30	-19.94	370.40	0.00
368.40	-19.94	370.40	0.00
368.50	-19.94	370.40	0.00
368.60	-19.88	370.40	0.00
368.70	-19.68	370.40	0.00
368.80	-19.34	370.40	0.00
368.90	-18.82	370.40	0.00
369.00	-18.18	370.40	0.00
369.10	-17.52	370.40	0.00
369.20	-16.83	370.40	0.00
369.30	-16.12	370.40	0.00
369.40	-15.37	370.40	0.00
369.50	-14.58	370.40	0.00
369.60	-13.74	370.40	0.00
369.70	-12.85	370.40	0.00
369.80	-11.90	370.40	0.00
369.90	-10.87	370.40	0.00
370.00	-9.73	370.40	0.00
370.10	-8.42	370.40	0.00
370.20	-6.87	370.40	0.00
370.30	-4.86	370.40	0.00
370.40	0.00	370.40	0.00
370.50	4.88	370.40	0.00
370.60	6.88	370.40	0.00
370.70	8.42	370.40	0.00
370.80	9.72	370.40	0.00
370.90	10.87	370.40	0.00
371.00	11.90	370.40	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 10 years Storm Event: 10

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
	20.46	270 50	0.00
366.90	-20.46	370.50	0.00
367.00	-20.46	370.50	0.00
367.10	-20.46	370.50	0.00
367.20	-20.46	370.50	0.00
367.30	-20.46	370.50	0.00
367.40	-20.46	370.50	0.00
367.50	-20.46	370.50	0.00
367.60	-20.46	370.50	0.00
367.70	-20.46	370.50	0.00
367.80	-20.46	370.50	0.00
367.90	-20.40	370.50	0.00
300.00	-20.40	370.50	0.00
300.10	-20.40	370.50	0.00
300.20	-20.40	370.50	0.00
308.30	-20.40	270.50	0.00
368 50	-20.40	370.50	0.00
308.30	-20.40	270.50	0.00
368 70	-20.43	370.50	0.00
368.80	-20.25	370.50	0.00
368.00	-19.95	370.50	0.00
369.00	-19.77	370.50	0.00
369.10	-18 18	370.50	0.00
369.20	-17 52	370.50	0.00
369.20	-16.83	370.50	0.00
369.40	-16.12	370.50	0.00
369.50	-15.37	370.50	0.00
369.60	-14.58	370.50	0.00
369.70	-13.74	370.50	0.00
369.80	-12.85	370.50	0.00
369,90	-11.90	370.50	0.00
370.00	-10.87	370.50	0.00
370.10	-9.73	370.50	0.00
370.20	-8.42	370.50	0.00
370.30	-6.87	370.50	0.00
370.40	-4.86	370.50	0.00
370.50	0.00	370.50	0.00
370.60	4.86	370.50	0.00
370.70	6.87	370.50	0.00
370.80	8.43	370.50	0.00
370.90	9.71	370.50	0.00
371.00	10.86	370.50	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-20.96	370.60	0.00
367.00	-20.96	370.60	0.00
367.10	-20.96	370.60	0.00
367.20	-20.96	370.60	0.00
367.30	-20.96	370.60	0.00
367.40	-20.96	370.60	0.00
367.50	-20.96	370.60	0.00
367.60	-20.96	370.60	0.00
367.70	-20.96	370.60	0.00
367.80	-20.96	370.60	0.00
367.90	-20.96	370.60	0.00
368.00	-20.96	370.60	0.00
368.10	-20.96	370.60	0.00
368.20	-20.96	370.60	0.00
368.30	-20.96	370.60	0.00
368.40	-20.96	370.60	0.00
368.50	-20.96	370.60	0.00
368.60	-20.95	370.60	0.00
368.70	-20.80	370.60	0.00
368.80	-20.50	370.60	0.00
368.90	-20.04	370.60	0.00
369.00	-19.44	370.60	0.00
369.10	-18.82	370.60	0.00
369.20	-18.18	370.60	0.00
369.30	-17.52	370.60	0.00
369.40	-16.83	370.60	0.00
369.50	-16.12	370.60	0.00
369.60	-15.37	370.60	0.00
369.70	-14.58	370.60	0.00
369.80	-13.74	370.60	0.00
369.90	-12.85	370.60	0.00
370.00	-11.90	370.60	0.00
370.10	-10.87	370.60	0.00
370.20	-9.73	370.60	0.00
370.30	-8.42	370.60	0.00
370.40	-6.87	370.60	0.00
370.50	-4.86	370.60	0.00
370.60	0.00	370.60	0.00
370.70	4.88	370.60	0.00
370.80	6.87	370.60	0.00
370.90	8.43	370.60	0.00
371.00	9.73	370.60	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Contributing Structure
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# Return Event: 10 years Storm Event: 10

# Composite Outflow Summary

Water Surface	Flow (ft <sup>3</sup> /s)	Tailwater Elevation	Convergence Error
(ft)	(12 / 5)	(10)	(10)
366.90	-21.46	370.70	0.00
367.00	-21.46	370.70	0.00
367.10	-21.46	370.70	0.00
367.20	-21.46	370.70	0.00
367.30	-21.46	370.70	0.00
367.40	-21.46	370.70	0.00
367.50	-21.46	370.70	0.00
367.60	-21.46	370.70	0.00
367.70	-21.46	370.70	0.00
367.80	-21.46	370.70	0.00
367.90	-21.46	370.70	0.00
368.00	-21.46	370.70	0.00
368.10	-21.46	370.70	0.00
368.20	-21.46	370.70	0.00
368.30	-21.46	370.70	0.00
368.40	-21.46	370.70	0.00
368.50	-21.46	370.70	0.00
368.60	-21.46	370.70	0.00
368.70	-21.33	370.70	0.00
368.80	-21.06	370.70	0.00
368.90	-20.62	370.70	0.00
369.00	-20.04	370.70	0.00
369.10	-19.44	370.70	0.00
369.20	-18.82	370.70	0.00
369.30	-18.18	370.70	0.00
369.40	-17.52	370.70	0.00
369.50	-16.83	370.70	0.00
369.60	-16.12	370.70	0.00
369.70	-15.37	370.70	0.00
369.80	-14.58	370.70	0.00
369.90	-13.74	370.70	0.00
370.00	-12.85	370.70	0.00
3/0.10	-11.90	3/0./0	0.00
3/0.20	-10.87	3/0.70	0.00
3/0.30	-9.73	3/0.70	0.00
3/0.40	-8.42	3/0./0	0.00
3/0.50	-6.87	3/0./0	0.00
3/0.60	-4.86	3/0./0	0.00
3/0.70	0.00	3/0.70	0.00
3/0.80	4.87	3/0./0	0.00
3/0.90	6.86	3/0./0	0.00
3/1.00	ŏ.42	3/0./0	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 300 of 448

#### Composite Outflow Summary

Contributing Structures

Contributing Structure
Culvert - 1

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 10 years Storm Event: 10

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water SurfaceFlowElevation(ft³/s)		Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-21.95	370.80	0.00
367.00	-21.95	370.80	0.00
367.10	-21.95	370.80	0.00
367.20	-21.95	370.80	0.00
367.30	-21.95	370.80	0.00
367.40	-21.95	370.80	0.00
367.50	-21.95	370.80	0.00
367.60	-21.95	370.80	0.00
367.70	-21.95	370.80	0.00
367.80	-21.95	370.80	0.00
367.90	-21.95	370.80	0.00
368.00	-21.95	370.80	0.00
368.10	-21.95	370.80	0.00
368.20	-21.95	370.80	0.00
368.30	-21.95	370.80	0.00
368.40	-21.95	370.80	0.00
368.50	-21.95	370.80	0.00
368.60	-21.95	370.80	0.00
368.70	-21.86	370.80	0.00
368.80	-21.61	370.80	0.00
368.90	-21.18	370.80	0.00
369.00	-20.62	370.80	0.00
369.10	-20.04	370.80	0.00
369.20	-19.44	370.80	0.00
369.30	-18.82	370.80	0.00
369.40	-18.18	370.80	0.00
369.50	-17.52	370.80	0.00
369.60	-16.83	370.80	0.00
369.70	-16.12	370.80	0.00
369.80	-15.37	370.80	0.00
369.90	-14.58	370.80	0.00
370.00	-13.74	370.80	0.00
370.10	-12.85	370.80	0.00
370.20	-11.90	370.80	0.00
370.30	-10.87	370.80	0.00
370.40	-9.73	370.80	0.00
370.50	-8.42	370.80	0.00
370.60	-6.87	370.80	0.00
370.70	-4.86	370.80	0.00
370.80	0.00	370.80	0.00
370.90	4.86	370.80	0.00
371.00	6.88	370.80	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 302 of 448

#### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 10 years Storm Event: 10

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

	Water Surface Flow		Tailwater Elevation	Convergence Error
	Elevation (ft <sup>3</sup> /s)		(ft)	(ft)
_	(π)		0-0.00	0.00
	366.90	-22.47	370.90	0.00
	367.00	-22.47	3/0.90	0.00
	367.10	-22.47	370.90	0.00
	367.20	-22.47	370.90	0.00
	367.30	-22.47	370.90	0.00
	367.40	-22.47	370.90	0.00
	367.50	-22.47	370.90	0.00
	367.60	-22.47	370.90	0.00
	367.70	-22.47	370.90	0.00
	367.80	-22.47	370.90	0.00
	367.90	-22.47	370.90	0.00
	368.00	-22.47	370.90	0.00
	368.10	-22.47	370.90	0.00
	368.20	-22.47	370.90	0.00
	368.30	-22.47	370.90	0.00
	368.40	-22.47	370.90	0.00
	368.50	-22.47	370.90	0.00
	368.60	-22.47	370.90	0.00
	368.70	-22.38	370.90	0.00
	368.80	-22.14	370.90	0.00
	368.90	-21./3	370.90	0.00
	369.00	-21.18	370.90	0.00
	369.10	-20.62	370.90	0.00
	369.20	-20.04	370.90	0.00
	369.30	-19.44	370.90	0.00
	369.40	-18.82	370.90	0.00
	369.50	-18.18	370.90	0.00
	369.60	-17.52	370.90	0.00
	369.70	-16.83	370.90	0.00
	369.80	-16.12	370.90	0.00
	369.90	-15.37	370.90	0.00
	370.00	-14.58	370.90	0.00
	3/0.10	-13.74	370.90	0.00
	370.20	-12.85	370.90	0.00
1	3/0.30	-11.90	370.90	0.00
	370.40	-10.87	370.90	0.00
1	3/0.50	-9./3	370.90	0.00
1	3/0.60	-8.42	3/0.90	0.00
1	3/0./0	-6.8/	3/0.90	0.00
1	370.80	-4.86	3/0.90	0.00
1	3/0.90	0.00	3/0.90	0.00
1	3/1.00	4.86	370.90	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 304 of 448

#### Composite Outflow Summary

Contributing Structures

Contributing Structu
Culvert - 1

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 10 years Storm Event: 10

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Water Surface Flow		Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-22.94	371.00	0.00
367.00	-22.94	371.00	0.00
367.10	-22.94	371.00	0.00
367.20	-22.94	371.00	0.00
367.30	-22.94	371.00	0.00
367.40	-22.94	371.00	0.00
367.50	-22.94	371.00	0.00
367.60	-22.94	371.00	0.00
367.70	-22.94	371.00	0.00
367.80	-22.94	371.00	0.00
367.90	-22.94	371.00	0.00
368.00	-22.94	371.00	0.00
368.10	-22.94	371.00	0.00
368.20	-22.94	371.00	0.00
368.30	-22.94	371.00	0.00
368.40	-22.94	371.00	0.00
368.50	-22.94	371.00	0.00
368.60	-22.94	371.00	0.00
368.70	-22.88	371.00	0.00
368.80	-22.66	371.00	0.00
368.90	-22.27	371.00	0.00
369.00	-21.73	371.00	0.00
369.10	-21.18	371.00	0.00
369.20	-20.62	371.00	0.00
369.30	-20.04	371.00	0.00
369.40	-19.44	371.00	0.00
369.50	-18.82	371.00	0.00
369.60	-18.18	371.00	0.00
369.70	-17.52	371.00	0.00
369.80	-16.83	371.00	0.00
369.90	-16.12	371.00	0.00
370.00	-15.37	371.00	0.00
370.10	-14.58	371.00	0.00
370.20	-13.74	371.00	0.00
370.30	-12.85	371.00	0.00
370.40	-11.90	371.00	0.00
370.50	-10.87	371.00	0.00
370.60	-9.73	371.00	0.00
370.70	-8.42	371.00	0.00
370.80	-6.87	371.00	0.00
370.90	-4.86	371.00	0.00
371.00	0.00	371.00	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 306 of 448

#### Composite Outflow Summary

Contributing Structures

Contributing	Su	uctui
Culvert - 1		

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Subsection: Outlet Input Data Label: OCS-A Scenario: Proposed Conditions 100 Year Storm

Return Event:	100 y	ears
Storm E	vent:	100

Requested Pond Water Surface ElevationsMinimum (Headwater)366.90 ftIncrement (Headwater)0.50 ftMaximum (Headwater)371.00 ft

# **Outlet Connectivity**

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Culvert-Circular	Culvert - 1	Forward + Reverse	TW	366.90	371.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 308 of 448 Subsection: Outlet Input Data Label: OCS-A Scenario: Proposed Conditions 100 Year Storm

Return Event: 100 years Storm Event: 100

Structure ID: Culvert - 1 Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	24.0 in
Length	120.00 ft
Length (Computed Barrel)	120.00 ft
Slope (Computed)	0.003 ft/ft
Outlet Control Data	
Manning's n	0.013
Ке	0.200
Kb	0.012
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
К	0.0045
М	2.0000
С	0.0317
Y	0.6900
T1 ratio (HW/D)	1.094
T2 ratio (HW/D)	1.196
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation.

Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control,

interpolate between flows at T1 & T2...

T1 Elevation	369.09 ft	T1 Flow	15.55 ft <sup>3</sup> /s
T2 Elevation	369.29 ft	T2 Flow	17.77 ft <sup>3</sup> /s

# Return Event: 100 years Storm Event: 100

#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error	
Elevation	(ft³/s)	(ft)	(ft)	
(ft)				
366.90	0.00	366.50	0.00	
367.00	0.04	366.50	0.00	
367.10	0.17	366.50	0.00	
367.20	0.39	366.50	0.00	
367.30	0.68	366.50	0.00	
367.40	1.05	366.50	0.00	
367.50	1.50	366.50	0.00	
367.60	2.01	366.50	0.00	
367.70	2.59	366.50	0.00	
367.80	3.24	366.50	0.00	
367.90	3.93	366.50	0.00	
368.00	4.70	366.50	0.00	
368.10	5.50	366.50	0.00	
368.20	6.34	366.50	0.00	
368.30	7.21	366.50	0.00	
368.40	8.14	366.50	0.00	
368.50	9.06	366.50	0.00	
368.60	10.00	366.50	0.00	
368.70	10.95	366.50	0.00	
368.80	11.90	366.50	0.00	
368.90	12.82	366.50	0.00	
369.00	13.75	366.50	0.00	
369.10	14.62	366.50	0.00	
369.20	15.45	366.50	0.00	
369.30	16.24	366.50	0.00	
369.40	16.93	366.50	0.00	
369.50	17.51	366.50	0.00	
369.60	18.03	366.50	0.00	
369.70	18.54	366.50	0.00	
369.80	19.06	366.50	0.00	
369.90	19.57	366.50	0.00	
370.00	20.07	366.50	0.00	
370.10	20.58	366.50	0.00	
370.20	21.07	366.50	0.00	
370.30	21.56	366.50	0.00	
370.40	22.04	366.50	0.00	
370.50	22.51	366.50	0.00	
370.60	22.99	366.50	0.00	
370.70	23.46	366.50	0.00	
370.80	23.91	366.50	0.00	
370.90	24.36	366.50	0.00	
371.00	24.81	366.50	0.00	

Contributing Structures

None Contributing

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 310 of 448

#### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

#### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error	
Elevation	(ft³/s)	(ft)	(ft)	
(ft)				
366.90	0.00	366.60	0.00	
367.00	0.04	366.60	0.00	
367.10	0.17	366.60	0.00	
367.20	0.39	366.60	0.00	
367.30	0.68	366.60	0.00	
367.40	1.05	366.60	0.00	
367.50	1.50	366.60	0.00	
367.60	2.01	366.60	0.00	
367.70	2.59	366.60	0.00	
367.80	3.24	366.60	0.00	
367.90	3.93	366.60	0.00	
368.00	4.70	366.60	0.00	
368.10	5.50	366.60	0.00	
368.20	6.34	366.60	0.00	
368.30	7.21	366.60	0.00	
368.40	8.14	366.60	0.00	
368.50	9.06	366.60	0.00	
368.60	10.00	366.60	0.00	
368.70	10.95	366.60	0.00	
368.80	11.90	366.60	0.00	
368.90	12.82	366.60	0.00	
369.00	13.75	366.60	0.00	
369.10	14.62	366.60	0.00	
369.20	15.45	366.60	0.00	
369.30	16.24	366.60	0.00	
369.40	16.93	366.60	0.00	
369.50	17.51	366.60	0.00	
369.60	18.03	366.60	0.00	
369.70	18.54	366.60	0.00	
369.80	19.06	366.60	0.00	
369.90	19.57	366.60	0.00	
370.00	20.07	366.60	0.00	
3/0.10	20.58	366.60	0.00	
3/0.20	21.07	366.60	0.00	
370.30	21.56	366.60	0.00	
3/0.40	22.04	366.60	0.00	
370.50	22.51	366.60	0.00	
370.60	22.99	366.60	0.00	
370.70	23.46	366.60	0.00	
3/0.80	23.91	366.60	0.00	
370.90	24.36	366.60	0.00	
371.00	24.81	366.60	0.00	

Contributing Structures

None Contributing

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 312 of 448

#### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	0.00	366.70	0.00
367.00	0.04	366.70	0.00
367.10	0.17	366.70	0.00
367.20	0.39	366.70	0.00
367.30	0.68	366.70	0.00
367.40	1.05	366.70	0.00
367.50	1.50	366.70	0.00
367.60	2.01	366.70	0.00
367.70	2.59	366.70	0.00
367.80	3.24	366.70	0.00
367.90	3.93	366.70	0.00
368.00	4.70	366.70	0.00
368.10	5.50	366.70	0.00
368.20	6.34	366.70	0.00
368.30	7.21	366.70	0.00
368.40	8.14	366.70	0.00
368.50	9.06	366.70	0.00
368.60	10.00	366.70	0.00
368.70	10.95	366.70	0.00
368.80	11.90	366.70	0.00
368.90	12.82	366.70	0.00
369.00	13.75	366.70	0.00
369.10	14.62	366.70	0.00
369.20	15.45	366.70	0.00
369.30	16.24	366.70	0.00
369.40	16.93	366.70	0.00
369.50	17.51	366.70	0.00
369.60	18.03	366.70	0.00
369.70	18.54	366.70	0.00
369.80	19.06	366.70	0.00
369.90	19.57	366.70	0.00
370.00	20.07	366.70	0.00
3/0.10	20.58	366.70	0.00
3/0.20	21.07	366.70	0.00
370.30	21.56	366.70	0.00
3/0.40	22.04	366.70	0.00
370.50	22.51	366.70	0.00
370.60	22.99	366.70	0.00
370.70	23.46	366.70	0.00
3/0.80	23.91	366.70	0.00
370.90	24.36	366.70	0.00
371.00	24.81	366.70	0.00

Contributing Structures

None Contributing

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 314 of 448

#### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 100 years Storm Event: 100

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)		T	
366.90	0.00	366.80	0.00
367.00	0.04	366.80	0.00
367.10	0.17	366.80	0.00
367.20	0.39	366.80	0.00
367.30	0.68	366.80	0.00
367.40	1.05	366.80	0.00
367.50	1.50	366.80	0.00
367.60	2.01	366.80	0.00
367.70	2.59	366.80	0.00
367.80	3.24	366.80	0.00
367.90	3.93	366.80	0.00
368.00	4.70	366.80	0.00
368.10	5.50	366.80	0.00
368.20	6.34	366.80	0.00
368.30	7.21	366.80	0.00
368.40	8.14	366.80	0.00
368.50	9.06	366.80	0.00
368.60	10.00	366.80	0.00
368.70	10.95	366.80	0.00
368.80	11.90	366.80	0.00
368.90	12.82	366.80	0.00
369.00	13.75	366.80	0.00
369.10	14.62	366.80	0.00
369.20	15.45	366.80	0.00
369.30	16.24	366.80	0.00
369.40	16.93	366.80	0.00
369.50	17.51	366.80	0.00
369.60	18.03	366.80	0.00
369.70	18.54	366.80	0.00
369.80	19.06	366.80	0.00
369.90	19.57	366.80	0.00
3/0.00	20.07	366.80	0.00
3/0.10	20.58	366.80	0.00
3/0.20	21.07	366.80	0.00
370.30	21.56	366.80	0.00
3/0.40	22.04	366.80	0.00
3/0.50	22.51	366.80	0.00
370.60	22.99	366.80	0.00
370.70	23.46	366.80	0.00
3/0.80	23.91	366.80	0.00
3/0.90	24.36	366.80	0.00
3/1.00	24.81	366.80	0.00

Contributing Structures

None Contributing

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 316 of 448

#### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 100 years Storm Event: 100

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	0.00	366.90	0.00
367.00	0.04	366.90	0.00
367.10	0.17	366.90	0.00
367.20	0.38	366.90	0.00
367.30	0.68	366.90	0.00
367.40	1.06	366.90	0.00
367.50	1.50	366.90	0.00
367.60	2.01	366.90	0.00
367.70	2.59	366.90	0.00
367.80	3.24	366.90	0.00
367.90	3.93	366.90	0.00
368.00	4.70	366.90	0.00
368.10	5.50	366.90	0.00
368.20	6.34	366.90	0.00
368.30	7.21	366.90	0.00
368.40	8.14	366.90	0.00
368.50	9.06	366.90	0.00
368.60	10.00	366.90	0.00
368.70	10.95	366.90	0.00
368.80	11.90	366.90	0.00
368.90	12.82	366.90	0.00
369.00	13.75	366.90	0.00
369.10	14.62	366.90	0.00
369.20	15.45	366.90	0.00
369.30	16.24	366.90	0.00
369.40	16.93	366.90	0.00
369.50	17.51	366.90	0.00
369.60	18.03	366.90	0.00
369.70	18.54	366.90	0.00
369.80	19.06	366.90	0.00
369.90	19.57	366.90	0.00
370.00	20.07	366.90	0.00
370.10	20.58	366.90	0.00
370.20	21.07	366.90	0.00
370.30	21.56	366.90	0.00
370.40	22.04	366.90	0.00
370.50	22.51	366.90	0.00
370.60	22.99	366.90	0.00
370.70	23.46	366.90	0.00
370.80	23.91	366.90	0.00
370.90	24.36	366.90	0.00
371.00	24.81	366.90	0.00

Contributing Structures

None Contributing

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-0.03	367.00	0.00
367.00	0.00	367.00	0.00
367.10	0.17	367.00	0.00
367.20	0.39	367.00	0.00
367.30	0.68	367.00	0.00
367.40	1.05	367.00	0.00
367.50	1.50	367.00	0.00
367.60	2.02	367.00	0.00
367.70	2.59	367.00	0.00
367.80	3.24	367.00	0.00
367.90	3.93	367.00	0.00
368.00	4.70	367.00	0.00
368.10	5.50	367.00	0.00
368.20	6.34	367.00	0.00
368.30	7.21	367.00	0.00
368.40	8.14	367.00	0.00
368.50	9.06	367.00	0.00
368.60	10.00	367.00	0.00
368.70	10.95	367.00	0.00
368.80	11.90	367.00	0.00
368.90	12.82	367.00	0.00
369.00	13.75	367.00	0.00
369.10	14.62	367.00	0.00
369.20	15.45	367.00	0.00
369.30	16.24	367.00	0.00
369.40	16.93	367.00	0.00
369.50	17.51	367.00	0.00
369.60	18.03	367.00	0.00
369.70	18.54	367.00	0.00
369.80	19.06	367.00	0.00
369.90	19.57	367.00	0.00
370.00	20.07	367.00	0.00
370.10	20.58	367.00	0.00
370.20	21.07	367.00	0.00
370.30	21.56	367.00	0.00
370.40	22.04	367.00	0.00
370.50	22.51	367.00	0.00
370.60	22.99	367.00	0.00
370.70	23.46	367.00	0.00
370.80	23.91	367.00	0.00
370.90	24.36	367.00	0.00
371.00	24.81	367.00	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-0.14	367.10	0.00
367.00	-0.14	367.10	0.00
367.10	0.00	367.10	0.00
367.20	0.38	367.10	0.00
367.30	0.68	367.10	0.00
367.40	1.05	367.10	0.00
367.50	1.50	367.10	0.00
367.60	2.01	367.10	0.00
367.70	2.59	367.10	0.00
367.80	3.24	367.10	0.00
367.90	3.93	367.10	0.00
368.00	4.70	367.10	0.00
368.10	5.50	367.10	0.00
368.20	6.34	367.10	0.00
368.30	7.21	367.10	0.00
368.40	8.14	367.10	0.00
368.50	9.06	367.10	0.00
368.60	10.00	367.10	0.00
368.70	10.95	367.10	0.00
368.80	11.90	367.10	0.00
368.90	12.82	367.10	0.00
369.00	13.75	367.10	0.00
369.10	14.62	367.10	0.00
369.20	15.45	367.10	0.00
369.30	16.24	367.10	0.00
369.40	16.93	367.10	0.00
369.50	17.51	367.10	0.00
369.60	18.03	367.10	0.00
369.70	18.54	367.10	0.00
369.80	19.06	367.10	0.00
369.90	19.57	367.10	0.00
370.00	20.07	367.10	0.00
370.10	20.58	367.10	0.00
370.20	21.07	367.10	0.00
370.30	21.56	367.10	0.00
370.40	22.04	367.10	0.00
370.50	22.51	367.10	0.00
370.60	22.99	367.10	0.00
370.70	23.46	367.10	0.00
370.80	23.91	367.10	0.00
370.90	24.36	367.10	0.00
371.00	24.81	367.10	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-0.32	367.20	0.00
367.00	-0.32	367.20	0.00
367.10	-0.32	367.20	0.00
367.20	0.00	367.20	0.00
367.30	0.66	367.20	0.00
367.40	1.05	367.20	0.00
367.50	1.50	367.20	0.00
367.60	2.01	367.20	0.00
367.70	2.59	367.20	0.00
367.80	3.24	367.20	0.00
367.90	3.95	367.20	0.00
368.00	4.70	367.20	0.00
368.10	5.50	367.20	0.00
368.20	6.34	367.20	0.00
368.30	7.21	367.20	0.00
368.40	8.14	367.20	0.00
368.50	9.06	367.20	0.00
368.60	10.00	367.20	0.00
368.70	10.95	367.20	0.00
368.80	11.90	367.20	0.00
368.90	12.82	367.20	0.00
369.00	13.75	367.20	0.00
369.10	14.62	367.20	0.00
369.20	15.45	367.20	0.00
369.30	16.24	367.20	0.00
369.40	16.93	367.20	0.00
369.50	17.51	367.20	0.00
369.60	18.03	367.20	0.00
369.70	18.54	367.20	0.00
369.80	19.06	367.20	0.00
369.90	19.57	367.20	0.00
370.00	20.07	367.20	0.00
370.10	20.58	367.20	0.00
370.20	21.07	367.20	0.00
370.30	21.56	367.20	0.00
370.40	22.04	367.20	0.00
370.50	22.51	367.20	0.00
370.60	22.99	367.20	0.00
370.70	23.46	367.20	0.00
370.80	23.91	367.20	0.00
370.90	24.36	367.20	0.00
371.00	24.81	367.20	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
	0.50	267.20	0.00
366.90	-0.58	367.30	0.00
367.00	-0.58	307.30	0.00
367.10	-0.58	367.30	0.00
307.20	-0.50	307.30	0.00
367.30	0.00	307.30	0.00
307.40	0.98	307.30	0.00
307.50	1.49	307.30	0.00
267.00	2.01	267.20	0.00
367.70	2.59	367.30	0.00
307.80	3.24	307.30	0.00
307.90	3.94	267.20	0.00
300.00	4.00	267.20	0.00
300.10	5.50	267.20	0.00
300.20	0.34	267.30	0.00
300.30	7.21	267.30	0.00
269 50	0.14	267.30	0.00
300.30	9.00	267.30	0.00
368 70	10.00	367 30	0.00
368.80	10.95	367 30	0.00
368.00	11.90	367 30	0.00
369.00	12.02	367.30	0.00
369.10	14.62	367.30	0.00
369.20	15.45	367.30	0.00
369.30	16.74	367.30	0.00
369.40	16.93	367.30	0.00
369.50	17.51	367.30	0.00
369.60	18.03	367.30	0.00
369.70	18.54	367.30	0.00
369.80	19.06	367.30	0.00
369.90	19.57	367.30	0.00
370.00	20.07	367.30	0.00
370.10	20.58	367.30	0.00
370.20	21.07	367.30	0.00
370.30	21.56	367.30	0.00
370.40	22.04	367.30	0.00
370.50	22.51	367.30	0.00
370.60	22.99	367.30	0.00
370.70	23.46	367.30	0.00
370.80	23.91	367.30	0.00
370.90	24.36	367.30	0.00
371.00	24.81	367.30	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-0.90	367.40	0.00
367.00	-0.90	367.40	0.00
367.10	-0.90	367.40	0.00
367.20	-0.90	367.40	0.00
367.30	-0.83	367.40	0.00
367.40	0.00	367.40	0.00
367.50	1.33	367.40	0.00
367.60	1.97	367.40	0.00
367.70	2.58	367.40	0.00
367.80	3.24	367.40	0.00
367.90	3.94	367.40	0.00
368.00	4.69	367.40	0.00
368.10	5.50	367.40	0.00
368.20	6.34	367.40	0.00
368.30	7.21	367.40	0.00
368.40	8.14	367.40	0.00
368.50	9.06	367.40	0.00
368.60	10.00	367.40	0.00
368.70	10.95	367.40	0.00
368.80	11.90	367.40	0.00
368.90	12.82	367.40	0.00
369.00	13.75	367.40	0.00
369.10	14.62	367.40	0.00
369.20	15.45	367.40	0.00
369.30	16.24	367.40	0.00
369.40	16.93	367.40	0.00
369.50	17.51	367.40	0.00
369.60	18.03	367.40	0.00
369.70	18.54	367.40	0.00
369.80	19.06	367.40	0.00
369.90	19.57	367.40	0.00
370.00	20.07	367.40	0.00
370.10	20.58	367.40	0.00
370.20	21.07	367.40	0.00
370.30	21.56	367.40	0.00
370.40	22.04	367.40	0.00
370.50	22.51	367.40	0.00
370.60	22.99	367.40	0.00
370.70	23.46	367.40	0.00
370.80	23.91	367.40	0.00
370.90	24.36	367.40	0.00
371.00	24.81	367.40	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-1.29	367.50	0.00
367.00	-1.29	367.50	0.00
367.10	-1.29	367.50	0.00
367.20	-1.29	367.50	0.00
367.30	-1.29	367.50	0.00
367.40	-1.14	367.50	0.00
367.50	0.00	367.50	0.00
367.60	1.71	367.50	0.00
367.70	2.50	367.50	0.00
367.80	3.21	367.50	0.00
367.90	3.93	367.50	0.00
368.00	4.68	367.50	0.00
368.10	5.50	367.50	0.00
368.20	6.34	367.50	0.00
368.30	7.21	367.50	0.00
368.40	8.14	367.50	0.00
368.50	9.06	367.50	0.00
368.60	10.00	367.50	0.00
368.70	10.95	367.50	0.00
368.80	11.90	367.50	0.00
368.90	12.82	367.50	0.00
369.00	13.75	367.50	0.00
369.10	14.62	367.50	0.00
369.20	15.45	367.50	0.00
369.30	16.24	367.50	0.00
369.40	16.93	367.50	0.00
369.50	17.51	367.50	0.00
369.60	18.03	367.50	0.00
369.70	18.54	367.50	0.00
369.80	19.06	367.50	0.00
369.90	19.57	367.50	0.00
370.00	20.07	367.50	0.00
370.10	20.58	367.50	0.00
370.20	21.07	367.50	0.00
370.30	21.56	367.50	0.00
370.40	22.04	367.50	0.00
370.50	22.51	367.50	0.00
370.60	22.99	367.50	0.00
370.70	23.46	367.50	0.00
370.80	23.91	367.50	0.00
370.90	24.36	367.50	0.00
371.00	24.81	367.50	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-1.74	367.60	0.00
367.00	-1.74	367.60	0.00
367.10	-1.74	367.60	0.00
367.20	-1.74	367.60	0.00
367.30	-1.74	367.60	0.00
367.40	-1.73	367.60	0.00
367.50	-1.48	367.60	0.00
367.60	0.00	367.60	0.00
367.70	2.09	367.60	0.00
367.80	3.05	367.60	0.00
367.90	3.86	367.60	0.00
368.00	4.66	367.60	0.00
368.10	5.47	367.60	0.00
368.20	6.34	367.60	0.00
368.30	7.21	367.60	0.00
368.40	8.12	367.60	0.00
368.50	9.06	367.60	0.00
368.60	10.00	367.60	0.00
368.70	10.95	367.60	0.00
368.80	11.90	367.60	0.00
368.90	12.82	367.60	0.00
369.00	13.75	367.60	0.00
369.10	14.62	367.60	0.00
369.20	15.45	367.60	0.00
369.30	16.24	367.60	0.00
369.40	16.93	367.60	0.00
369.50	17.51	367.60	0.00
369.60	18.03	367.60	0.00
369.70	18.54	367.60	0.00
369.80	19.06	367.60	0.00
369.90	19.57	367.60	0.00
370.00	20.07	367.60	0.00
370.10	20.58	367.60	0.00
370.20	21.07	367.60	0.00
370.30	21.56	367.60	0.00
370.40	22.04	367.60	0.00
370.50	22.51	367.60	0.00
370.60	22.99	367.60	0.00
370.70	23.46	367.60	0.00
370.80	23.91	367.60	0.00
370.90	24.36	367.60	0.00
371.00	24 81	367.60	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow (ft³/s)	Tailwater Elevation	Convergence Error
(ft)	(10,70)	(10)	
366.90	-2.25	367.70	0.00
367.00	-2.25	367.70	0.00
367.10	-2.25	367.70	0.00
367.20	-2.25	367.70	0.00
367.30	-2.25	367.70	0.00
367.40	-2.25	367.70	0.00
367.50	-2.19	367.70	0.00
367.60	-1.81	367.70	0.00
367.70	0.00	367.70	0.00
367.80	2.48	367.70	0.00
367.90	3.59	367.70	0.00
368.00	4.54	367.70	0.00
368.10	5.43	367.70	0.00
368.20	6.30	367.70	0.00
368.30	7.20	367.70	0.00
368.40	8.12	367.70	0.00
368.50	9.04	367.70	0.00
368.60	9.99	367.70	0.00
368.70	10.95	367.70	0.00
368.80	11.90	367.70	0.00
368.90	12.82	367.70	0.00
369.00	13.75	367.70	0.00
369.10	14.62	367.70	0.00
369.20	15.45	367.70	0.00
369.30	16.24	367.70	0.00
369.40	16.93	367.70	0.00
369.50	17.51	367.70	0.00
369.60	18.03	367.70	0.00
369.70	18.54	367.70	0.00
369.80	19.06	367.70	0.00
369.90	19.57	367.70	0.00
370.00	20.07	367.70	0.00
370.10	20.58	367.70	0.00
370.20	21.07	367.70	0.00
370.30	21.56	367.70	0.00
370.40	22.04	367.70	0.00
370.50	22.51	367.70	0.00
370.60	22.99	367.70	0.00
370.70	23.46	367.70	0.00
370.80	23.91	367.70	0.00
370.90	24.36	367.70	0.00
371.00	24.81	367.70	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-2.81	367.80	0.00
367.00	-2.81	367.80	0.00
367.10	-2.81	367.80	0.00
367.20	-2.81	367.80	0.00
367.30	-2.81	367.80	0.00
367.40	-2.81	367.80	0.00
367.50	-2.81	367.80	0.00
367.60	-2.68	367.80	0.00
367.70	-2.17	367.80	0.00
367.80	0.00	367.80	0.00
367.90	2.88	367.80	0.00
368.00	4.17	367.80	0.00
368.10	5.22	367.80	0.00
368.20	6.18	367.80	0.00
368.30	7.12	367.80	0.00
368.40	8.07	367.80	0.00
368.50	9.02	367.80	0.00
368.60	9.99	367.80	0.00
368.70	10.93	367.80	0.00
368.80	11.90	367.80	0.00
368.90	12.82	367.80	0.00
369.00	13.75	367.80	0.00
369.10	14.62	367.80	0.00
369.20	15.45	367.80	0.00
369.30	16.24	367.80	0.00
369.40	16.93	367.80	0.00
369.50	17.51	367.80	0.00
369.60	18.03	367.80	0.00
369.70	18.54	367.80	0.00
369.80	19.06	367.80	0.00
369.90	19.57	367.80	0.00
370.00	20.07	367.80	0.00
370.10	20.58	367.80	0.00
370.20	21.07	367.80	0.00
370.30	21.56	367.80	0.00
370.40	22.04	367.80	0.00
370.50	22.51	367.80	0.00
370.60	22.99	367.80	0.00
370.70	23.46	367.80	0.00
370.80	23.91	367.80	0.00
370.90	24.36	367.80	0.00
371.00	24.81	367.80	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-3.43	367.90	0.00
367.00	-3.43	367.90	0.00
367.10	-3.43	367.90	0.00
367.20	-3.43	367.90	0.00
367.30	-3.43	367.90	0.00
367.40	-3.43	367.90	0.00
367.50	-3.43	367.90	0.00
367.60	-3.42	367.90	0.00
367.70	-3.18	367.90	0.00
367.80	-2.53	367.90	0.00
367.90	0.00	367.90	0.00
368.00	3.25	367.90	0.00
368.10	4.71	367.90	0.00
368.20	5.86	367.90	0.00
368.30	6.93	367.90	0.00
368.40	7.93	367.90	0.00
368.50	8.93	367.90	0.00
368.60	9.91	367.90	0.00
368.70	10.90	367.90	0.00
368.80	11.86	367.90	0.00
368.90	12.81	367.90	0.00
369.00	13.75	367.90	0.00
369.10	14.62	367.90	0.00
369.20	15.45	367.90	0.00
369.30	16.24	367.90	0.00
369.40	16.93	367.90	0.00
369.50	17.51	367.90	0.00
369.60	18.03	367.90	0.00
369.70	18.54	367.90	0.00
369.80	19.06	367.90	0.00
369.90	19.57	367.90	0.00
370.00	20.07	367.90	0.00
370.10	20.58	367.90	0.00
370.20	21.07	367.90	0.00
370.30	21.56	367.90	0.00
370.40	22.04	367.90	0.00
370.50	22.51	367.90	0.00
370.60	22.99	367.90	0.00
370.70	23.46	367.90	0.00
370.80	23.91	367.90	0.00
370.90	24.36	367.90	0.00
371.00	24 81	367.90	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-4.10	368.00	0.00
367.00	-4.10	368.00	0.00
367.10	-4.10	368.00	0.00
367.20	-4.10	368.00	0.00
367.30	-4.10	368.00	0.00
367.40	-4.10	368.00	0.00
367.50	-4.10	368.00	0.00
367.60	-4.10	368.00	0.00
367.70	-4.03	368.00	0.00
367.80	-3.70	368.00	0.00
367.90	-2.91	368.00	0.00
368.00	0.00	368.00	0.00
368.10	3.64	368.00	0.00
368.20	5.22	368.00	0.00
368.30	6.49	368.00	0.00
368.40	7.64	368.00	0.00
368.50	8.72	368.00	0.00
368.60	9.76	368.00	0.00
368.70	10.77	368.00	0.00
368.80	11.77	368.00	0.00
368.90	12.74	368.00	0.00
369.00	13.69	368.00	0.00
369.10	14.59	368.00	0.00
369.20	15.44	368.00	0.00
369.30	16.24	368.00	0.00
369.40	16.93	368.00	0.00
369.50	17.51	368.00	0.00
369.60	18.03	368.00	0.00
369.70	18.54	368.00	0.00
369.80	19.06	368.00	0.00
369.90	19.57	368.00	0.00
370.00	20.07	368.00	0.00
370.10	20.58	368.00	0.00
370.20	21.07	368.00	0.00
370.30	21.56	368.00	0.00
370.40	22.04	368.00	0.00
370.50	22.51	368.00	0.00
370.60	22.99	368.00	0.00
370.70	23.46	368.00	0.00
370.80	23.91	368.00	0.00
370.90	24.36	368.00	0.00
371.00	24.81	368.00	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-4.79	368.10	0.00
367.00	-4.79	368.10	0.00
367.10	-4.79	368.10	0.00
367.20	-4.79	368.10	0.00
367.30	-4.79	368.10	0.00
367.40	-4.79	368.10	0.00
367.50	-4.79	368.10	0.00
367.60	-4.79	368.10	0.00
367.70	-4.79	368.10	0.00
367.80	-4.65	368.10	0.00
367.90	-4.20	368.10	0.00
368.00	-3.24	368.10	0.00
368.10	0.00	368.10	0.00
368.20	4.00	368.10	0.00
368.30	5.70	368.10	0.00
368.40	7.09	368.10	0.00
368.50	8.33	368.10	0.00
368.60	9.45	368.10	0.00
368.70	10.54	368.10	0.00
368.80	11.57	368.10	0.00
368.90	12.58	368.10	0.00
369.00	13.55	368.10	0.00
369.10	14.48	368.10	0.00
369.20	15.35	368.10	0.00
369.30	16.16	368.10	0.00
369.40	16.88	368.10	0.00
369.50	17.48	368.10	0.00
369.60	18.01	368.10	0.00
369.70	18.54	368.10	0.00
369.80	19.06	368.10	0.00
369.90	19.57	368.10	0.00
370.00	20.07	368.10	0.00
370.10	20.58	368.10	0.00
370.20	21.07	368.10	0.00
370.30	21.56	368.10	0.00
370.40	22.04	368.10	0.00
370.50	22.51	368.10	0.00
370.60	22.99	368.10	0.00
370.70	23.46	368.10	0.00
370.80	23.91	368.10	0.00
370.90	24.36	368.10	0.00
371.00	24.81	368.10	0.00

Contributing Structures

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#### Composite Outflow Summary

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-5.53	368.20	0.00
367.00	-5.53	368.20	0.00
367.10	-5.53	368.20	0.00
367.20	-5.53	368.20	0.00
367.30	-5.53	368.20	0.00
367.40	-5.53	368.20	0.00
367.50	-5.53	368.20	0.00
367.60	-5.53	368.20	0.00
367.70	-5.53	368.20	0.00
367.80	-5.51	368.20	0.00
367.90	-5.25	368.20	0.00
368.00	-4.70	368.20	0.00
368.10	-3.60	368.20	0.00
368.20	0.00	368.20	0.00
368.30	4.28	368.20	0.00
368.40	6.17	368.20	0.00
368.50	7.63	368.20	0.00
368.60	8.93	368.20	0.00
368.70	10.11	368.20	0.00
368.80	11.23	368.20	0.00
368.90	12.29	368.20	0.00
369.00	13.29	368.20	0.00
369.10	14.25	368.20	0.00
369.20	15.14	368.20	0.00
369.30	15.96	368.20	0.00
369.40	16.69	368.20	0.00
369.50	17.28	368.20	0.00
369.60	17.85	368.20	0.00
369.70	18.41	368.20	0.00
369.80	18.96	368.20	0.00
369.90	19.50	368.20	0.00
370.00	20.03	368.20	0.00
370.10	20.55	368.20	0.00
370.20	21.06	368.20	0.00
370.30	21.55	368.20	0.00
370.40	22.04	368.20	0.00
370.50	22.52	368.20	0.00
370.60	22.99	368.20	0.00
370.70	23.46	368.20	0.00
370.80	23.91	368.20	0.00
370.90	24.36	368.20	0.00
371.00	24.81	368.20	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-6.29	368.30	0.00
367.00	-6.29	368.30	0.00
367.10	-6.29	368.30	0.00
367.20	-6.29	368.30	0.00
367.30	-6.29	368.30	0.00
367.40	-6.29	368.30	0.00
367.50	-6.29	368.30	0.00
367.60	-6.29	368.30	0.00
367.70	-6.29	368.30	0.00
367.80	-6.29	368.30	0.00
367.90	-6.20	368.30	0.00
368.00	-5.84	368.30	0.00
368.10	-5.17	368.30	0.00
368.20	-3.91	368.30	0.00
368.30	0.00	368.30	0.00
368.40	4.58	368.30	0.00
368.50	6.55	368.30	0.00
368.60	8.10	368.30	0.00
368.70	9.45	368.30	0.00
368.80	10.66	368.30	0.00
368.90	11.81	368.30	0.00
369.00	12.86	368.30	0.00
369.10	13.85	368.30	0.00
369.20	14.76	368.30	0.00
369.30	15.59	368.30	0.00
369.40	16.31	368.30	0.00
369.50	16.90	368.30	0.00
369.60	17.51	368.30	0.00
369.70	18.09	368.30	0.00
369.80	18.67	368.30	0.00
369.90	19.26	368.30	0.00
370.00	19.81	368.30	0.00
370.10	20.36	368.30	0.00
370.20	20.90	368.30	0.00
370.30	21.42	368.30	0.00
370.40	21.93	368.30	0.00
370.50	22.43	368.30	0.00
370.60	22.93	368.30	0.00
370.70	23.41	368.30	0.00
370.80	23.88	368.30	0.00
370.90	24.35	368.30	0.00
371.00	24 79	368-30	0.00

Contributing Structures

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#### Composite Outflow Summary

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(π)			0.00
366.90	-7.08	368.40	0.00
367.00	-7.08	368.40	0.00
367.10	-7.08	368.40	0.00
367.20	-7.08	368.40	0.00
367.30	-7.08	368.40	0.00
367.40	-7.08	368.40	0.00
367.50	-7.08	368.40	0.00
367.60	-7.08	368.40	0.00
367.70	-7.08	368.40	0.00
367.80	-7.08	368.40	0.00
367.90	-7.06	368.40	0.00
368.00	-6.87	368.40	0.00
368.10	-6.41	368.40	0.00
368.20	-5.63	368.40	0.00
368.30	-4.24	368.40	0.00
368.40	0.00	368.40	0.00
368.50	4.82	368.40	0.00
368.60	6.85	368.40	0.00
368.70	8.45	368.40	0.00
368.80	9.85	368.40	0.00
368.90	11.06	368.40	0.00
369.00	12.20	368.40	0.00
369.10	13.22	368.40	0.00
369.20	14.16	368.40	0.00
369.30	14.96	368.40	0.00
369.40	15.61	368.40	0.00
369.50	16.26	368.40	0.00
369.60	16.91	368.40	0.00
369.70	17.56	368.40	0.00
369.80	18.19	368.40	0.00
369.90	18.81	368.40	0.00
370.00	19.41	368.40	0.00
370.10	19.98	368.40	0.00
370.20	20.55	368.40	0.00
3/0.30	21.11	368.40	0.00
3/0.40	21.65	308.40	0.00
3/0.50	22.17	368.40	0.00
3/0.60	22.69	368.40	0.00
3/0./0	23.19	368.40	0.00
3/0.80	23.69	368.40	0.00
3/0.90	24.18	368.40	0.00
3/1.00	24.65	368.40	0.00

Contributing Structures

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-7.87	368.50	0.00
367.00	-7.87	368.50	0.00
367.10	-7.87	368.50	0.00
367.20	-7.87	368.50	0.00
367.30	-7.87	368.50	0.00
367.40	-7.87	368.50	0.00
367.50	-7.87	368.50	0.00
367.60	-7.87	368.50	0.00
367.70	-7.87	368.50	0.00
367.80	-7.87	368.50	0.00
367.90	-7.87	368.50	0.00
368.00	-7.80	368.50	0.00
368.10	-7.51	368.50	0.00
368.20	-6.94	368.50	0.00
368.30	-6.01	368.50	0.00
368.40	-4.48	368.50	0.00
368.50	0.00	368.50	0.00
368.60	4.94	368.50	0.00
368.70	7.00	368.50	0.00
368.80	8.64	368.50	0.00
368.90	10.00	368.50	0.00
369.00	11.18	368.50	0.00
369.10	12.21	368.50	0.00
369.20	13.03	368.50	0.00
369.30	13.74	368.50	0.00
369.40	14.58	368.50	0.00
369.50	15.36	368.50	0.00
369.60	16.12	368.50	0.00
369.70	16.84	368.50	0.00
369.80	17.53	368.50	0.00
369.90	18.18	368.50	0.00
370.00	18.82	368.50	0.00
370.10	19.44	368.50	0.00
370.20	20.04	368.50	0.00
370.30	20.62	368.50	0.00
370.40	21.18	368.50	0.00
370.50	21.74	368.50	0.00
370.60	22.27	368.50	0.00
370.70	22.79	368.50	0.00
370.80	23.30	368.50	0.00
370.90	23.80	368.50	0.00
371.00	24.30	368.50	0.00

Contributing Structures

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### Composite Outflow Summary

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-8.65	368.60	0.00
367.00	-8.65	368.60	0.00
367.10	-8.65	368.60	0.00
367.20	-8.65	368.60	0.00
367.30	-8.65	368.60	0.00
367.40	-8.65	368.60	0.00
367.50	-8.65	368.60	0.00
367.60	-8.65	368.60	0.00
367.70	-8.65	368.60	0.00
367.80	-8.65	368.60	0.00
367.90	-8.65	368.60	0.00
368.00	-8.64	368.60	0.00
368.10	-8.48	368.60	0.00
368.20	-8.08	368.60	0.00
368.30	-7.39	368.60	0.00
368.40	-6.34	368.60	0.00
368.50	-4.63	368.60	0.00
368.60	0.00	368.60	0.00
368.70	4.95	368.60	0.00
368.80	6.98	368.60	0.00
368.90	8.53	368.60	0.00
369.00	9.82	368.60	0.00
369.10	10.89	368.60	0.00
369.20	11.90	368.60	0.00
369.30	12.85	368.60	0.00
369.40	13.74	368.60	0.00
369.50	14.58	368.60	0.00
369.60	15.36	368.60	0.00
369.70	16.12	368.60	0.00
369.80	16.83	368.60	0.00
369.90	17.52	368.60	0.00
370.00	18.19	368.60	0.00
370.10	18.83	368.60	0.00
370.20	19.44	368.60	0.00
370.30	20.03	368.60	0.00
370.40	20.62	368.60	0.00
370.50	21.18	368.60	0.00
370.60	21.73	368.60	0.00
370.70	22.27	368.60	0.00
370.80	22.79	368.60	0.00
370.90	23.31	368.60	0.00
371.00	23.80	368.60	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface Elevation	Flow (ft <sup>3</sup> /s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-9.42	368.70	0.00
367.00	-9.42	368.70	0.00
367.10	-9.42	368.70	0.00
367.20	-9.42	368.70	0.00
367.30	-9.42	368.70	0.00
367.40	-9.42	368.70	0.00
367.50	-9.42	368.70	0.00
367.60	-9.42	368.70	0.00
367.70	-9.42	368.70	0.00
367.80	-9.42	368.70	0.00
367.90	-9.42	368.70	0.00
368.00	-9.42	368.70	0.00
368.10	-9.35	368.70	0.00
368.20	-9.06	368.70	0.00
368.30	-8.56	368.70	0.00
368.40	-7.75	368.70	0.00
368.50	-6.56	368.70	0.00
368.60	-4.77	368.70	0.00
368.70	0.00	368.70	0.00
368.80	4.89	368.70	0.00
368.90	6.93	368.70	0.00
369.00	8.45	368.70	0.00
369.10	9.71	368.70	0.00
369.20	10.86	368.70	0.00
369.30	11.91	368.70	0.00
369.40	12.86	368.70	0.00
369.50	13.74	368.70	0.00
369.60	14.57	368.70	0.00
369.70	15.37	368.70	0.00
369.80	16.12	368.70	0.00
369.90	16.84	368.70	0.00
370.00	17.52	368.70	0.00
370.10	18.19	368.70	0.00
370.20	18.82	368.70	0.00
370.30	19.43	368.70	0.00
370.40	20.04	368.70	0.00
370.50	20.62	368.70	0.00
370.60	21.18	368.70	0.00
370.70	21.73	368.70	0.00
370.80	22.26	368.70	0.00
370.90	22.80	368.70	0.00
371.00	23.31	368.70	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-10.16	368.80	0.00
367.00	-10.16	368.80	0.00
367.10	-10.16	368.80	0.00
367.20	-10.16	368.80	0.00
367.30	-10.16	368.80	0.00
367.40	-10.16	368.80	0.00
367.50	-10.16	368.80	0.00
367.60	-10.16	368.80	0.00
367.70	-10.16	368.80	0.00
367.80	-10.16	368.80	0.00
367.90	-10.16	368.80	0.00
368.00	-10.16	368.80	0.00
368.10	-10.13	368.80	0.00
368.20	-9.95	368.80	0.00
368.30	-9.56	368.80	0.00
368.40	-8.94	368.80	0.00
368.50	-8.01	368.80	0.00
368.60	-6.72	368.80	0.00
368.70	-4.82	368.80	0.00
368.80	0.00	368.80	0.00
368.90	4.85	368.80	0.00
369.00	6.87	368.80	0.00
369.10	8.43	368.80	0.00
369.20	9.72	368.80	0.00
369.30	10.86	368.80	0.00
369.40	11.91	368.80	0.00
369.50	12.86	368.80	0.00
369.60	13.74	368.80	0.00
369.70	14.58	368.80	0.00
369.80	15.37	368.80	0.00
369.90	16.12	368.80	0.00
370.00	16.83	368.80	0.00
370.10	17.52	368.80	0.00
370.20	18.19	368.80	0.00
370.30	18.82	368.80	0.00
370.40	19.44	368.80	0.00
370.50	20.04	368.80	0.00
370.60	20.62	368.80	0.00
370.70	21.18	368.80	0.00
370.80	21.73	368.80	0.00
370.90	22.27	368.80	0.00
371.00	22.79	368.80	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-10.87	368.90	0.00
367.00	-10.87	368.90	0.00
367.10	-10.87	368.90	0.00
367.20	-10.87	368.90	0.00
367.30	-10.87	368.90	0.00
367.40	-10.87	368.90	0.00
367.50	-10.87	368.90	0.00
367.60	-10.87	368.90	0.00
367.70	-10.87	368.90	0.00
367.80	-10.87	368.90	0.00
367.90	-10.87	368.90	0.00
368.00	-10.87	368.90	0.00
368.10	-10.87	368.90	0.00
368.20	-10.78	368.90	0.00
368.30	-10.49	368.90	0.00
368.40	-9.99	368.90	0.00
368.50	-9.25	368.90	0.00
368.60	-8.20	368.90	0.00
368.70	-6.82	368.90	0.00
368.80	-4.86	368.90	0.00
368.90	0.00	368.90	0.00
369.00	4.87	368.90	0.00
369.10	6.86	368.90	0.00
369.20	8.42	368.90	0.00
369.30	9.73	368.90	0.00
369.40	10.87	368.90	0.00
369.50	11.91	368.90	0.00
369.60	12.86	368.90	0.00
369.70	13.74	368.90	0.00
369.80	14.58	368.90	0.00
369.90	15.37	368.90	0.00
370.00	16.12	368.90	0.00
370.10	16.84	368.90	0.00
370.20	17.52	368.90	0.00
370.30	18.18	368.90	0.00
370.40	18.82	368.90	0.00
370.50	19.44	368.90	0.00
370.60	20.04	368.90	0.00
370.70	20.61	368.90	0.00
370.80	21.18	368.90	0.00
370.90	21.74	368.90	0.00
371.00	22.27	368.90	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-11.58	369.00	0.00
367.00	-11.58	369.00	0.00
367.10	-11.58	369.00	0.00
367.20	-11.58	369.00	0.00
367.30	-11.58	369.00	0.00
367.40	-11.58	369.00	0.00
367.50	-11.58	369.00	0.00
367.60	-11.58	369.00	0.00
367.70	-11.58	369.00	0.00
367.80	-11.58	369.00	0.00
367.90	-11.58	369.00	0.00
368.00	-11.58	369.00	0.00
368.10	-11.58	369.00	0.00
368.20	-11.54	369.00	0.00
368.30	-11.34	369.00	0.00
368.40	-10.94	369.00	0.00
368.50	-10.32	369.00	0.00
368.60	-9.47	369.00	0.00
368.70	-8.34	369.00	0.00
368.80	-6.87	369.00	0.00
368.90	-4.86	369.00	0.00
369.00	0.00	369.00	0.00
369.10	4.84	369.00	0.00
369.20	6.88	369.00	0.00
369.30	8.41	369.00	0.00
369.40	9.72	369.00	0.00
369.50	10.86	369.00	0.00
369.60	11.90	369.00	0.00
369.70	12.85	369.00	0.00
369.80	13.74	369.00	0.00
369.90	14.58	369.00	0.00
370.00	15.37	369.00	0.00
370.10	16.12	369.00	0.00
370.20	16.84	369.00	0.00
370.30	17.52	369.00	0.00
370.40	18.18	369.00	0.00
370.50	18.82	369.00	0.00
370.60	19.43	369.00	0.00
370.70	20.04	369.00	0.00
370.80	20.62	369.00	0.00
370.90	21.18	369.00	0.00
371.00	21.73	369.00	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-12.25	369.10	0.00
367.00	-12.25	369.10	0.00
367.10	-12.25	369.10	0.00
367.20	-12.25	369.10	0.00
367.30	-12.25	369.10	0.00
367.40	-12.25	369.10	0.00
367.50	-12.25	369.10	0.00
367.60	-12.25	369.10	0.00
367.70	-12.25	369.10	0.00
367.80	-12.25	369.10	0.00
367.90	-12.25	369.10	0.00
368.00	-12.25	369.10	0.00
368.10	-12.25	369.10	0.00
368.20	-12.25	369.10	0.00
368.30	-12.12	369.10	0.00
368.40	-11.80	369.10	0.00
368.50	-11.30	369.10	0.00
368.60	-10.56	369.10	0.00
368.70	-9.61	369.10	0.00
368.80	-8.39	369.10	0.00
368.90	-6.87	369.10	0.00
369.00	-4.86	369.10	0.00
369.10	0.00	369.10	0.00
369.20	4.85	369.10	0.00
369.30	6.87	369.10	0.00
369.40	8.42	369.10	0.00
369.50	9.72	369.10	0.00
369.60	10.86	369.10	0.00
369.70	11.90	369.10	0.00
369.80	12.86	369.10	0.00
369.90	13.74	369.10	0.00
370.00	14.58	369.10	0.00
370.10	15.37	369.10	0.00
370.20	16.12	369.10	0.00
370.30	16.84	369.10	0.00
370.40	17.53	369.10	0.00
370.50	18.18	369.10	0.00
370.60	18.82	369.10	0.00
370.70	19.43	369.10	0.00
370.80	20.03	369.10	0.00
370.90	20.62	369.10	0.00
371.00	21.18	369.10	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface	Flow (ft³/s)	Tailwater Elevation	Convergence Error
(ft)	(12 / 5)	(10)	(10)
366.90	-12.92	369.20	0.00
367.00	-12.92	369.20	0.00
367.10	-12.92	369.20	0.00
367.20	-12.92	369.20	0.00
367.30	-12.92	369.20	0.00
367,40	-12.92	369.20	0.00
367.50	-12.92	369.20	0.00
367.60	-12.92	369.20	0.00
367.70	-12.92	369.20	0.00
367.80	-12.92	369.20	0.00
367.90	-12.92	369.20	0.00
368.00	-12.92	369.20	0.00
368.10	-12.92	369.20	0.00
368.20	-12.92	369.20	0.00
368.30	-12.85	369.20	0.00
368.40	-12.61	369.20	0.00
368.50	-12.18	369.20	0.00
368.60	-11.56	369.20	0.00
368.70	-10.73	369.20	0.00
368.80	-9.70	369.20	0.00
368.90	-8.42	369.20	0.00
369.00	-6.87	369.20	0.00
369.10	-4.86	369.20	0.00
369.20	0.00	369.20	0.00
369.30	4.88	369.20	0.00
369.40	6.87	369.20	0.00
369.50	8.42	369.20	0.00
369.60	9.71	369.20	0.00
369.70	10.88	369.20	0.00
369.80	11.91	369.20	0.00
369.90	12.87	369.20	0.00
370.00	13.75	369.20	0.00
370.10	14.58	369.20	0.00
370.20	15.37	369.20	0.00
370.30	16.12	369.20	0.00
370.40	16.84	369.20	0.00
370.50	17.52	369.20	0.00
370.60	18.19	369.20	0.00
370.70	18.82	369.20	0.00
370.80	19.43	369.20	0.00
370.90	20.04	369.20	0.00
371.00	20.62	369.20	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-13.58	369.30	0.00
367.00	-13.58	369.30	0.00
367.10	-13.58	369.30	0.00
367.20	-13.58	369.30	0.00
367.30	-13.58	369.30	0.00
367.40	-13.58	369.30	0.00
367.50	-13.58	369.30	0.00
367.60	-13.58	369.30	0.00
367.70	-13.58	369.30	0.00
367.80	-13.58	369.30	0.00
367.90	-13.58	369.30	0.00
368.00	-13.58	369.30	0.00
368.10	-13.58	369.30	0.00
368.20	-13.58	369.30	0.00
368.30	-13.55	369.30	0.00
368.40	-13.38	369.30	0.00
368.50	-13.02	369.30	0.00
368.60	-12.48	369.30	0.00
368.70	-11.75	369.30	0.00
368.80	-10.82	369.30	0.00
368.90	-9.73	369.30	0.00
369.00	-8.42	369.30	0.00
369.10	-6.87	369.30	0.00
369.20	-4.86	369.30	0.00
369.30	0.00	369.30	0.00
369.40	4.86	369.30	0.00
369.50	6.86	369.30	0.00
369.60	8.41	369.30	0.00
369.70	9.72	369.30	0.00
369.80	10.87	369.30	0.00
369.90	11.90	369.30	0.00
370.00	12.86	369.30	0.00
370.10	13.75	369.30	0.00
370.20	14.58	369.30	0.00
370.30	15.36	369.30	0.00
370.40	16.12	369.30	0.00
370.50	16.83	369.30	0.00
370.60	17.52	369.30	0.00
370.70	18.19	369.30	0.00
370.80	18.82	369.30	0.00
370.90	19.44	369.30	0.00
371.00	20.04	369.30	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-14.22	369.40	0.00
367.00	-14.22	369.40	0.00
367.10	-14.22	369.40	0.00
367.20	-14.22	369.40	0.00
367.30	-14.22	369.40	0.00
367.40	-14.22	369.40	0.00
367.50	-14.22	369.40	0.00
367.60	-14.22	369.40	0.00
367.70	-14.22	369.40	0.00
367.80	-14.22	369.40	0.00
367.90	-14.22	369.40	0.00
368.00	-14.22	369.40	0.00
368.10	-14.22	369.40	0.00
368.20	-14.22	369.40	0.00
368.30	-14.22	369.40	0.00
368.40	-14.09	369.40	0.00
368.50	-13.80	369.40	0.00
368.60	-13.33	369.40	0.00
368.70	-12.68	369.40	0.00
368.80	-11.86	369.40	0.00
368.90	-10.87	369.40	0.00
369.00	-9.73	369.40	0.00
369.10	-8.42	369.40	0.00
369.20	-6.87	369.40	0.00
369.30	-4.86	369.40	0.00
369.40	0.00	369.40	0.00
369.50	4.88	369.40	0.00
369.60	6.86	369.40	0.00
369.70	8.43	369.40	0.00
369.80	9.72	369.40	0.00
369.90	10.87	369.40	0.00
370.00	11.90	369.40	0.00
370.10	12.86	369.40	0.00
370.20	13.74	369.40	0.00
370.30	14.58	369.40	0.00
370.40	15.37	369.40	0.00
370.50	16.12	369.40	0.00
370.60	16.83	369.40	0.00
370.70	17.51	369.40	0.00
370.80	18.19	369.40	0.00
370.90	18.82	369.40	0.00
371.00	19.44	369,40	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-14.84	369.50	0.00
367.00	-14.84	369.50	0.00
367.10	-14.84	369.50	0.00
367.20	-14.84	369.50	0.00
367.30	-14.84	369.50	0.00
367.40	-14.84	369.50	0.00
367.50	-14.84	369.50	0.00
367.60	-14.84	369.50	0.00
367.70	-14.84	369.50	0.00
367.80	-14.84	369.50	0.00
367.90	-14.84	369.50	0.00
368.00	-14.84	369.50	0.00
368.10	-14.84	369.50	0.00
368.20	-14.84	369.50	0.00
368.30	-14.84	369.50	0.00
368.40	-14.77	369.50	0.00
368.50	-14.53	369.50	0.00
368.60	-14.13	369.50	0.00
368.70	-13.55	369.50	0.00
368.80	-12.80	369.50	0.00
368.90	-11.90	369.50	0.00
369.00	-10.87	369.50	0.00
369.10	-9.73	369.50	0.00
369.20	-8.42	369.50	0.00
369.30	-6.87	369.50	0.00
369.40	-4.86	369.50	0.00
369.50	0.00	369.50	0.00
369.60	4.84	369.50	0.00
369.70	6.87	369.50	0.00
369.80	8.42	369.50	0.00
369.90	9.73	369.50	0.00
370.00	10.86	369.50	0.00
370.10	11.90	369.50	0.00
370.20	12.86	369.50	0.00
370.30	13.75	369.50	0.00
370.40	14.58	369.50	0.00
370.50	15.36	369.50	0.00
370.60	16.11	369.50	0.00
370.70	16.83	369.50	0.00
370.80	17.52	369.50	0.00
370.90	18.18	369.50	0.00
371.00	18.82	369.50	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-15.46	369.60	0.00
367.00	-15.46	369.60	0.00
367.10	-15.46	369.60	0.00
367.20	-15.46	369.60	0.00
367.30	-15.46	369.60	0.00
367.40	-15.46	369.60	0.00
367.50	-15.46	369.60	0.00
367.60	-15.46	369.60	0.00
367.70	-15.46	369.60	0.00
367.80	-15.46	369.60	0.00
367.90	-15.46	369.60	0.00
368.00	-15.46	369.60	0.00
368.10	-15.46	369.60	0.00
368.20	-15.46	369.60	0.00
368.30	-15.46	369.60	0.00
368.40	-15.43	369.60	0.00
368.50	-15.23	369.60	0.00
368.60	-14.88	369.60	0.00
368.70	-14.36	369.60	0.00
368.80	-13.69	369.60	0.00
368.90	-12.85	369.60	0.00
369.00	-11.90	369.60	0.00
369.10	-10.87	369.60	0.00
369.20	-9.73	369.60	0.00
369.30	-8.42	369.60	0.00
369.40	-6.87	369.60	0.00
369.50	-4.86	369.60	0.00
369.60	0.00	369.60	0.00
369.70	4.85	369.60	0.00
369.80	6.87	369.60	0.00
369.90	8.41	369.60	0.00
370.00	9.73	369.60	0.00
370.10	10.87	369.60	0.00
370.20	11.90	369.60	0.00
370.30	12.87	369.60	0.00
370.40	13.75	369.60	0.00
370.50	14.58	369.60	0.00
370.60	15.36	369.60	0.00
370.70	16.12	369.60	0.00
370.80	16.83	369.60	0.00
370.90	17.52	369.60	0.00
371.00	18.19	369.60	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-16.06	369.70	0.00
367.00	-16.06	369.70	0.00
367.10	-16.06	369.70	0.00
367.20	-16.06	369.70	0.00
367.30	-16.06	369.70	0.00
367.40	-16.06	369.70	0.00
367.50	-16.06	369.70	0.00
367.60	-16.06	369.70	0.00
367.70	-16.06	369.70	0.00
367.80	-16.06	369.70	0.00
367.90	-16.06	369.70	0.00
368.00	-16.06	369.70	0.00
368.10	-16.06	369.70	0.00
368.20	-16.06	369.70	0.00
368.30	-16.06	369.70	0.00
368.40	-16.05	369.70	0.00
368.50	-15.90	369.70	0.00
368.60	-15.59	369.70	0.00
368.70	-15.14	369.70	0.00
368.80	-14.52	369.70	0.00
368.90	-13.74	369.70	0.00
369.00	-12.85	369.70	0.00
369.10	-11.90	369.70	0.00
369.20	-10.87	369.70	0.00
369.30	-9.73	369.70	0.00
369.40	-8.42	369.70	0.00
369.50	-6.87	369.70	0.00
369.60	-4.86	369.70	0.00
369.70	0.00	369.70	0.00
369.80	4.85	369.70	0.00
369.90	6.87	369.70	0.00
370.00	8.41	369.70	0.00
370.10	9.73	369.70	0.00
370.20	10.87	369.70	0.00
370.30	11.91	369.70	0.00
370.40	12.86	369.70	0.00
370.50	13.75	369.70	0.00
370.60	14.58	369.70	0.00
370.70	15.37	369.70	0.00
370.80	16.12	369.70	0.00
370.90	16.84	369.70	0.00
371.00	17.52	369.70	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-16.64	369.80	0.00
367.00	-16.64	369.80	0.00
367.10	-16.64	369.80	0.00
367.20	-16.64	369.80	0.00
367.30	-16.64	369.80	0.00
367.40	-16.64	369.80	0.00
367.50	-16.64	369.80	0.00
367.60	-16.64	369.80	0.00
367.70	-16.64	369.80	0.00
367.80	-16.64	369.80	0.00
367.90	-16.64	369.80	0.00
368.00	-16.64	369.80	0.00
368.10	-16.64	369.80	0.00
368.20	-16.64	369.80	0.00
368.30	-16.64	369.80	0.00
368.40	-16.64	369.80	0.00
368.50	-16.55	369.80	0.00
368.60	-16.28	369.80	0.00
368.70	-15.85	369.80	0.00
368.80	-15.29	369.80	0.00
368.90	-14.58	369.80	0.00
369.00	-13.74	369.80	0.00
369.10	-12.85	369.80	0.00
369.20	-11.90	369.80	0.00
369.30	-10.87	369.80	0.00
369.40	-9.73	369.80	0.00
369.50	-8.42	369.80	0.00
369.60	-6.87	369.80	0.00
369.70	-4.86	369.80	0.00
369.80	0.00	369.80	0.00
369.90	4.87	369.80	0.00
370.00	6.88	369.80	0.00
370.10	8.42	369.80	0.00
370.20	9.73	369.80	0.00
370.30	10.86	369.80	0.00
370.40	11.90	369.80	0.00
370.50	12.86	369.80	0.00
370.60	13.75	369.80	0.00
370.70	14.58	369.80	0.00
370.80	15.37	369.80	0.00
370.90	16.11	369.80	0.00
371.00	16.84	369.80	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
	17.01	200.00	0.00
366.90	-17.21	369.90	0.00
367.00	-17.21	369.90	0.00
367.10	-17.21	369.90	0.00
367.20	-17.21	369.90	0.00
367.30	-17.21	369.90	0.00
367.40	-17.21	369.90	0.00
307.50	-17.21	369.90	0.00
367.60	-17.21	369.90	0.00
367.70	-17.21	369.90	0.00
367.80	-17.21	369.90	0.00
367.90	-17.21	369.90	0.00
368.00	-17.21	369.90	0.00
308.10	-17.21	369.90	0.00
308.20	-17.21	369.90	0.00
300.30	-17.21	309.90	0.00
300.40	-17.21	369.90	0.00
308.50	-17.15	369.90	0.00
308.00	-10.94	369.90	0.00
300.70	-10.57	309.90	0.00
300.00	-10.03	309.90	0.00
308.90	-15.37	369.90	0.00
369.00	-14.30	309.90	0.00
369.10	-13.74	309.90	0.00
309.20	-12.05	309.90	0.00
369.30	-11.90	309.90	0.00
369.50	-10.07	369.90	0.00
369.50	-9.75	309.90	0.00
369.00	-6.87	369.90	0.00
369.80	-4.86	369.90	0.00
369.90	0.00	369.90	0.00
370.00	4 84	369.90	0.00
370.00	6.89	369.90	0.00
370.10	8 41	369.90	0.00
370.20	9.73	369.90	0.00
370.30	10.87	369.90	0.00
370.10	11 90	369.90	0.00
370.50	12.86	369.90	0.00
370.00	13 75	369.90	0.00
370.80	14 57	369.90	0.00
370.00	15 36	369.90	0.00
371.00	16 11	369.90	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-17.79	370.00	0.00
367.00	-17.79	370.00	0.00
367.10	-17.79	370.00	0.00
367.20	-17.79	370.00	0.00
367.30	-17.79	370.00	0.00
367.40	-17.79	370.00	0.00
367.50	-17.79	370.00	0.00
367.60	-17.79	370.00	0.00
367.70	-17.79	370.00	0.00
367.80	-17.79	370.00	0.00
367.90	-17.79	370.00	0.00
368.00	-17.79	370.00	0.00
368.10	-17.79	370.00	0.00
368.20	-17.79	370.00	0.00
368.30	-17.79	370.00	0.00
368.40	-17.79	370.00	0.00
368.50	-17.75	370.00	0.00
368.60	-17.57	370.00	0.00
368.70	-17.24	370.00	0.00
368.80	-16.75	370.00	0.00
368.90	-16.12	370.00	0.00
369.00	-15.37	370.00	0.00
369.10	-14.58	370.00	0.00
369.20	-13.74	370.00	0.00
369.30	-12.85	370.00	0.00
369.40	-11.90	370.00	0.00
369.50	-10.87	370.00	0.00
369.60	-9.73	370.00	0.00
369.70	-8.42	370.00	0.00
369.80	-6.87	370.00	0.00
369.90	-4.86	370.00	0.00
370.00	0.00	370.00	0.00
370.10	4.84	370.00	0.00
370.20	6.88	370.00	0.00
370.30	8.43	370.00	0.00
370.40	9.72	370.00	0.00
370.50	10.87	370.00	0.00
370.60	11.90	370.00	0.00
370.70	12.86	370.00	0.00
370.80	13.74	370.00	0.00
370.90	14.58	370.00	0.00
371.00	15.37	370.00	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-18.33	370.10	0.00
367.00	-18.33	370.10	0.00
367.10	-18.33	370.10	0.00
367.20	-18.33	370.10	0.00
367.30	-18.33	370.10	0.00
367.40	-18.33	370.10	0.00
367.50	-18.33	370.10	0.00
367.60	-18.33	370.10	0.00
367.70	-18.33	370.10	0.00
367.80	-18.33	370.10	0.00
367.90	-18.33	370.10	0.00
368.00	-18.33	370.10	0.00
368.10	-18.33	370.10	0.00
368.20	-18.33	370.10	0.00
368.30	-18.33	370.10	0.00
368.40	-18.33	370.10	0.00
368.50	-18.32	370.10	0.00
368.60	-18.17	370.10	0.00
368.70	-17.88	370.10	0.00
368.80	-17.43	370.10	0.00
368.90	-16.83	370.10	0.00
369.00	-16.12	370.10	0.00
369.10	-15.37	370.10	0.00
369.20	-14.58	370.10	0.00
369.30	-13.74	370.10	0.00
369.40	-12.85	370.10	0.00
369.50	-11.90	370.10	0.00
369.60	-10.87	370.10	0.00
369.70	-9.73	370.10	0.00
369.80	-8.42	370.10	0.00
369.90	-6.87	370.10	0.00
370.00	-4.86	370.10	0.00
370.10	0.00	370.10	0.00
370.20	4.88	370.10	0.00
370.30	6.86	370.10	0.00
370.40	8.41	370.10	0.00
370.50	9.71	370.10	0.00
370.60	10.87	370.10	0.00
370.70	11.91	370.10	0.00
370.80	12.86	370.10	0.00
370.90	13.74	370.10	0.00
371.00	14.58	370.10	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

## Composite Outflow Summary

Water Surface Elevation	Flow (ft <sup>3</sup> /s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)	( - / - /	(-)	
366.90	-18.87	370.20	0.00
367.00	-18.87	370.20	0.00
367.10	-18.87	370.20	0.00
367.20	-18.87	370.20	0.00
367.30	-18.87	370.20	0.00
367.40	-18.87	370.20	0.00
367.50	-18.87	370.20	0.00
367.60	-18.87	370.20	0.00
367.70	-18.87	370.20	0.00
367.80	-18.87	370.20	0.00
367.90	-18.87	370.20	0.00
368.00	-18.87	370.20	0.00
368.10	-18.87	370.20	0.00
368.20	-18.87	370.20	0.00
368.30	-18.87	370.20	0.00
368.40	-18.87	370.20	0.00
368.50	-18.87	370.20	0.00
368.60	-18.76	370.20	0.00
368.70	-18.50	370.20	0.00
368.80	-18.08	370.20	0.00
368.90	-17.52	370.20	0.00
369.00	-16.83	370.20	0.00
369.10	-16.12	370.20	0.00
369.20	-15.37	370.20	0.00
369.30	-14.58	370.20	0.00
369.40	-13.74	370.20	0.00
369.50	-12.85	370.20	0.00
369.60	-11.90	370.20	0.00
369.70	-10.87	370.20	0.00
369.80	-9.73	370.20	0.00
369.90	-8.42	370.20	0.00
370.00	-6.87	370.20	0.00
370.10	-4.86	370.20	0.00
370.20	0.00	370.20	0.00
370.30	4.85	370.20	0.00
370.40	6.88	370.20	0.00
370.50	8.43	370.20	0.00
370.60	9.72	370.20	0.00
370.70	10.86	370.20	0.00
370.80	11.91	370.20	0.00
370.90	12.86	370.20	0.00
371.00	13.74	370.20	0.00

Contributing Structures

Culvert - 1

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### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(π)
(π)		0-0.00	
366.90	-19.42	370.30	0.00
367.00	-19.42	370.30	0.00
367.10	-19.42	3/0.30	0.00
367.20	-19.42	3/0.30	0.00
367.30	-19.42	3/0.30	0.00
367.40	-19.42	370.30	0.00
367.50	-19.42	370.30	0.00
367.60	-19.42	3/0.30	0.00
367.70	-19.42	370.30	0.00
367.80	-19.42	370.30	0.00
367.90	-19.42	370.30	0.00
368.00	-19.42	370.30	0.00
368.10	-19.42	370.30	0.00
368.20	-19.42	370.30	0.00
368.30	-19.42	370.30	0.00
368.40	-19.42	370.30	0.00
368.50	-19.42	370.30	0.00
368.60	-19.34	370.30	0.00
368.70	-19.10	370.30	0.00
368.80	-18.73	370.30	0.00
368.90	-18.18	370.30	0.00
369.00	-17.52	370.30	0.00
369.10	-16.83	370.30	0.00
369.20	-16.12	370.30	0.00
369.30	-15.37	370.30	0.00
369.40	-14.58	370.30	0.00
369.50	-13.74	370.30	0.00
369.60	-12.85	370.30	0.00
369.70	-11.90	370.30	0.00
369.80	-10.87	370.30	0.00
369.90	-9.73	370.30	0.00
370.00	-8.42	370.30	0.00
370.10	-6.87	370.30	0.00
370.20	-4.86	370.30	0.00
370.30	0.00	370.30	0.00
370.40	4.87	370.30	0.00
370.50	6.89	370.30	0.00
370.60	8.40	370.30	0.00
370.70	9.71	370.30	0.00
370.80	10.87	370.30	0.00
370.90	11.90	370.30	0.00
371.00	12.85	370.30	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-19.94	370.40	0.00
367.00	-19.94	370.40	0.00
367.10	-19.94	370.40	0.00
367.20	-19.94	370.40	0.00
367.30	-19.94	370.40	0.00
367.40	-19.94	370.40	0.00
367.50	-19.94	370.40	0.00
367.60	-19.94	370.40	0.00
367.70	-19.94	370.40	0.00
367.80	-19.94	370.40	0.00
367.90	-19.94	370.40	0.00
368.00	-19.94	370.40	0.00
368.10	-19.94	370.40	0.00
368.20	-19.94	370.40	0.00
368.30	-19.94	370.40	0.00
368.40	-19.94	370.40	0.00
368.50	-19.94	370.40	0.00
368.60	-19.88	370.40	0.00
368.70	-19.68	370.40	0.00
368.80	-19.34	370.40	0.00
368.90	-18.82	370.40	0.00
369.00	-18.18	370.40	0.00
369.10	-17.52	370.40	0.00
369.20	-16.83	370.40	0.00
369.30	-16.12	370.40	0.00
369.40	-15.37	370.40	0.00
369.50	-14.58	370.40	0.00
369.60	-13.74	370.40	0.00
369.70	-12.85	370.40	0.00
369.80	-11.90	370.40	0.00
369.90	-10.87	370.40	0.00
370.00	-9.73	370.40	0.00
370.10	-8.42	370.40	0.00
370.20	-6.87	370.40	0.00
370.30	-4.86	370.40	0.00
370.40	0.00	370.40	0.00
370.50	4.88	370.40	0.00
370.60	6.88	370.40	0.00
370.70	8.42	370.40	0.00
370.80	9.72	370.40	0.00
370.90	10.87	370.40	0.00
371.00	11.90	370.40	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-20.46	370.50	0.00
367.00	-20.46	370.50	0.00
367.10	-20.46	370.50	0.00
367.20	-20.46	370.50	0.00
367.30	-20.46	370.50	0.00
367.40	-20.46	370.50	0.00
367.50	-20.46	370.50	0.00
367.60	-20.46	370.50	0.00
367.70	-20.46	370.50	0.00
367.80	-20.46	370.50	0.00
367.90	-20.46	370.50	0.00
368.00	-20.46	370.50	0.00
368.10	-20.46	370.50	0.00
368.20	-20.46	370.50	0.00
368.30	-20.46	370.50	0.00
368.40	-20.46	370.50	0.00
368.50	-20.46	370.50	0.00
368.60	-20.43	370.50	0.00
368.70	-20.25	370.50	0.00
368.80	-19.93	370.50	0.00
368.90	-19.44	370.50	0.00
369.00	-18.82	370.50	0.00
369.10	-18.18	370.50	0.00
369.20	-17.52	370.50	0.00
369.30	-16.83	370.50	0.00
369.40	-16.12	370.50	0.00
369.50	-15.37	370.50	0.00
369.60	-14.58	370.50	0.00
369.70	-13.74	370.50	0.00
369.80	-12.85	370.50	0.00
369.90	-11.90	370.50	0.00
370.00	-10.87	370.50	0.00
370.10	-9.73	370.50	0.00
370.20	-8.42	370.50	0.00
370.30	-6.87	370.50	0.00
370.40	-4.86	370.50	0.00
370.50	0.00	370.50	0.00
370.60	4.86	370.50	0.00
370.70	6.87	370.50	0.00
370.80	8.43	370.50	0.00
370.90	9.71	370.50	0.00
371.00	10.86	370.50	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-20.96	370.60	0.00
367.00	-20.96	370.60	0.00
367.10	-20.96	370.60	0.00
367.20	-20.96	370.60	0.00
367.30	-20.96	370.60	0.00
367.40	-20.96	370.60	0.00
367.50	-20.96	370.60	0.00
367.60	-20.96	370.60	0.00
367.70	-20.96	370.60	0.00
367.80	-20.96	370.60	0.00
367.90	-20.96	370.60	0.00
368.00	-20.96	370.60	0.00
368.10	-20.96	370.60	0.00
368.20	-20.96	370.60	0.00
368.30	-20.96	370.60	0.00
368.40	-20.96	370.60	0.00
368.50	-20.96	370.60	0.00
368.60	-20.95	370.60	0.00
368.70	-20.80	370.60	0.00
368.80	-20.50	370.60	0.00
368.90	-20.04	370.60	0.00
369.00	-19.44	370.60	0.00
369.10	-18.82	370.60	0.00
369.20	-18.18	370.60	0.00
369.30	-17.52	370.60	0.00
369.40	-16.83	370.60	0.00
369.50	-16.12	370.60	0.00
369.60	-15.37	370.60	0.00
369.70	-14.58	370.60	0.00
369.80	-13.74	370.60	0.00
369.90	-12.85	370.60	0.00
370.00	-11.90	370.60	0.00
370.10	-10.87	370.60	0.00
370.20	-9.73	370.60	0.00
370.30	-8.42	370.60	0.00
370.40	-6.87	370.60	0.00
370.50	-4.86	370.60	0.00
370.60	0.00	370.60	0.00
370.70	4.88	370.60	0.00
370.80	6.87	370.60	0.00
370.90	8.43	370.60	0.00
371.00	9.73	370.60	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-21.46	370.70	0.00
367.00	-21.46	370.70	0.00
367.10	-21.46	370.70	0.00
367.20	-21.46	370.70	0.00
367.30	-21.46	370.70	0.00
367.40	-21.46	370.70	0.00
367.50	-21.46	370.70	0.00
367.60	-21.46	370.70	0.00
367.70	-21.46	370.70	0.00
367.80	-21.46	370.70	0.00
367.90	-21.46	370.70	0.00
368.00	-21.46	370.70	0.00
368.10	-21.46	370.70	0.00
368.20	-21.46	370.70	0.00
368.30	-21.46	370.70	0.00
368.40	-21.46	370.70	0.00
368.50	-21.46	370.70	0.00
368.60	-21.46	370.70	0.00
368.70	-21.33	370.70	0.00
368.80	-21.06	370.70	0.00
368.90	-20.62	370.70	0.00
369.00	-20.04	370.70	0.00
369.10	-19.44	370.70	0.00
369.20	-18.82	370.70	0.00
369.30	-18.18	370.70	0.00
369.40	-17.52	370.70	0.00
369.50	-16.83	370.70	0.00
369.60	-16.12	370.70	0.00
369.70	-15.37	370.70	0.00
369.80	-14.58	370.70	0.00
369.90	-13.74	370.70	0.00
370.00	-12.85	370.70	0.00
370.10	-11.90	370.70	0.00
370.20	-10.87	370.70	0.00
370.30	-9.73	370.70	0.00
370.40	-8.42	370.70	0.00
370.50	-6.87	370.70	0.00
370.60	-4.86	370.70	0.00
370.70	0.00	370.70	0.00
370.80	4.87	370.70	0.00
370.90	6.86	370.70	0.00
371.00	8.42	370.70	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
(ft)			
366.90	-21.95	370.80	0.00
367.00	-21.95	370.80	0.00
367.10	-21.95	370.80	0.00
367.20	-21.95	370.80	0.00
367.30	-21.95	370.80	0.00
367.40	-21.95	370.80	0.00
367.50	-21.95	370.80	0.00
367.60	-21.95	370.80	0.00
367.70	-21.95	370.80	0.00
367.80	-21.95	370.80	0.00
367.90	-21.95	370.80	0.00
368.00	-21.95	370.80	0.00
368.10	-21.95	370.80	0.00
368.20	-21.95	370.80	0.00
368.30	-21.95	370.80	0.00
368.40	-21.95	370.80	0.00
368.50	-21.95	370.80	0.00
368.60	-21.95	370.80	0.00
368.70	-21.86	370.80	0.00
368.80	-21.61	370.80	0.00
368.90	-21.18	370.80	0.00
369.00	-20.62	370.80	0.00
369.10	-20.04	370.80	0.00
369.20	-19.44	370.80	0.00
369.30	-18.82	370.80	0.00
369.40	-18.18	370.80	0.00
369.50	-17.52	370.80	0.00
369.60	-16.83	370.80	0.00
369.70	-16.12	370.80	0.00
369.80	-15.37	370.80	0.00
369.90	-14.58	370.80	0.00
370.00	-13.74	370.80	0.00
370.10	-12.85	370.80	0.00
370.20	-11.90	370.80	0.00
370.30	-10.87	370.80	0.00
370.40	-9.73	370.80	0.00
370.50	-8.42	370.80	0.00
370.60	-6.87	370.80	0.00
370.70	-4.86	370.80	0.00
370.80	0.00	370.80	0.00
370.90	4.86	370.80	0.00
371.00	6.88	370.80	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

Contributing Structures

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(ft)			
366.90	-22.47	370.90	0.00
367.00	-22.47	370.90	0.00
367.10	-22.47	370.90	0.00
367.20	-22.47	370.90	0.00
367.30	-22.47	370.90	0.00
367.40	-22.47	370.90	0.00
367.50	-22.47	370.90	0.00
367.60	-22.47	370.90	0.00
367.70	-22.47	370.90	0.00
367.80	-22.47	370.90	0.00
367.90	-22.47	370.90	0.00
368.00	-22.47	370.90	0.00
368.10	-22.47	370.90	0.00
368.20	-22.47	370.90	0.00
368.30	-22.47	370.90	0.00
368.40	-22.47	370.90	0.00
368.50	-22.47	370.90	0.00
368.60	-22.47	370.90	0.00
368.70	-22.38	370.90	0.00
368.80	-22.14	370.90	0.00
368.90	-21.73	370.90	0.00
369.00	-21.18	370.90	0.00
369.10	-20.62	370.90	0.00
369.20	-20.04	370.90	0.00
369.30	-19.44	370.90	0.00
369.40	-18.82	370.90	0.00
369.50	-18.18	370.90	0.00
369.60	-17.52	370.90	0.00
369.70	-16.83	370.90	0.00
369.80	-16.12	370.90	0.00
369.90	-15.37	370.90	0.00
370.00	-14.58	370.90	0.00
370.10	-13.74	370.90	0.00
370.20	-12.85	370.90	0.00
370.30	-11.90	370.90	0.00
370.40	-10.87	370.90	0.00
370.50	-9.73	370.90	0.00
370.60	-8.42	370.90	0.00
370.70	-6.87	370.90	0.00
370.80	-4.86	370.90	0.00
370.90	0.00	370.90	0.00
371.00	4.86	370.90	0.00

Contributing Structures

Culvert - 1

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#### Composite Outflow Summary

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

	Water Surface	Flow	Tailwater Elevation	Convergence Error
	Elevation	(ft³/s)	(ft)	(ft)
	(π)		074.00	
	366.90	-22.94	3/1.00	0.00
	367.00	-22.94	3/1.00	0.00
	367.10	-22.94	3/1.00	0.00
	367.20	-22.94	3/1.00	0.00
	367.30	-22.94	3/1.00	0.00
	367.40	-22.94	3/1.00	0.00
	367.50	-22.94	3/1.00	0.00
	367.60	-22.94	3/1.00	0.00
	367.70	-22.94	3/1.00	0.00
	367.80	-22.94	3/1.00	0.00
	367.90	-22.94	3/1.00	0.00
	368.00	-22.94	3/1.00	0.00
	368.10	-22.94	3/1.00	0.00
	368.20	-22.94	3/1.00	0.00
	368.30	-22.94	3/1.00	0.00
	368.40	-22.94	3/1.00	0.00
	368.50	-22.94	3/1.00	0.00
	368.60	-22.94	3/1.00	0.00
	368.70	-22.88	3/1.00	0.00
	368.80	-22.66	3/1.00	0.00
	368.90	-22.27	3/1.00	0.00
	369.00	-21./3	3/1.00	0.00
	369.10	-21.18	3/1.00	0.00
	369.20	-20.62	3/1.00	0.00
	369.30	-20.04	3/1.00	0.00
	369.40	-19.44	3/1.00	0.00
	369.50	-18.82	3/1.00	0.00
	369.60	-18.18	3/1.00	0.00
	369.70	-17.52	3/1.00	0.00
	369.80	-16.83	3/1.00	0.00
	369.90	-16.12	3/1.00	0.00
	3/0.00	-15.3/	3/1.00	0.00
	370.10	-14.58	3/1.00	0.00
	370.20	-13./4	3/1.00	0.00
1	3/0.30	-12.85	3/1.00	0.00
	3/0.40	-11.90	3/1.00	0.00
1	3/0.50	-10.8/	3/1.00	0.00
1	3/0.60	-9./3	3/1.00	0.00
1	3/0./0	-8.42	3/1.00	0.00
1	3/0.80	-6.8/	3/1.00	0.00
1	3/0.90	-4.86	3/1.00	0.00
1	371.00	0.00	3/1.00	0.00

Contributing Structures

Culvert - 1

Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 400 of 448

#### Composite Outflow Summary

Contributing Structures

Culvert - 1 Culvert - 1

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Return Event: 100 years Storm Event: 100

PondPack CONNECT Edition

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Requested Pond Water Surface Elevations		
Minimum (Headwater)	366.50 ft	
Increment (Headwater)	0.10 ft	
Maximum (Headwater)	371.00 ft	

# **Outlet Connectivity**

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir	Weir - 1	Forward	Culvert - 1	370.00	371.00
Culvert-Circular	Culvert - 1	Forward	TW	366.50	371.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 402 of 448

Return Event: 1 years Storm Event: 1 Subsection: Outlet Input Data Label: OCS-B Scenario: Proposed Conditions 1 Year Storm

Structure ID: Weir - 1 Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	370.00 ft
Weir Length	4.00 ft
Weir Coefficient	3.33 (ft^0.5)/s
Structure ID: Culvert - 1	
Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	15.0 in
Length	146.50 ft
Length (Computed Barrel)	146.52 ft
Slope (Computed)	0.017 ft/ft
Outlet Control Data	
Manning's n	0.013
Ke	0.200
Kb	0.023
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
К	0.0045
М	2.0000
С	0.0317
Υ	0.6900
T1 ratio (HW/D)	1.087
T2 ratio (HW/D)	1.189
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation. Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	367.86 ft	T1 Flow	4.80 ft <sup>3</sup> /s
T2 Elevation	367.99 ft	T2 Flow	5.49 ft <sup>3</sup> /s

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Structure ID: TW Structure Type: TW Setup, DS	Channel
Tailwater Type	Free Outfall
Convergence Tolerances	
Maximum Iterations	30
Tailwater Tolerance (Minimum)	0.01 ft
Tailwater Tolerance (Maximum)	0.50 ft
Headwater Tolerance (Minimum)	0.01 ft
Headwater Tolerance (Maximum)	0.50 ft
Flow Tolerance (Minimum)	0.001 ft <sup>3</sup> /s
Flow Tolerance (Maximum)	10.000 ft <sup>3</sup> /s

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### Return Event: 1 years Storm Event: 1

### Composite Outflow Summary

Elevation (ft)         (ft)         (ft)         (ft)           366.50         0.00         (N/A)         0.00           366.60         0.00         (N/A)         0.00           366.70         0.00         (N/A)         0.00           366.80         0.00         (N/A)         0.00           367.10         0.00         (N/A)         0.00           367.20         0.00         (N/A)         0.00           367.30         0.00         (N/A)         0.00           367.40         0.00         (N/A)         0.00           367.50         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.80         0.00         (N/A)         0.00           367.80         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.60         0.00	Water Surface	Flow	Tailwater Elevation	Convergence Error
366.50         0.00 $(N/A)$ 0.00 $366.60$ 0.00 $(N/A)$ 0.00 $366.70$ 0.00 $(N/A)$ 0.00 $366.80$ 0.00 $(N/A)$ 0.00 $366.90$ 0.00 $(N/A)$ 0.00 $367.10$ 0.00 $(N/A)$ 0.00 $367.70$ 0.00 $(N/A)$ 0.00 $367.80$ 0.00 $(N/A)$ 0.00 $368.10$ 0.00 $(N/A)$ 0.00 $368.20$ 0.00 $(N/A)$ 0.00 $368.40$ 0.00 $(N/A)$ 0.00 $368.50$ 0.00 $(N/A)$ 0.00	Elevation	(ft³/s)	(ft)	(ft)
366.50         0.00         (N/A)         0.00           366.60         0.00         (N/A)         0.00           366.70         0.00         (N/A)         0.00           366.90         0.00         (N/A)         0.00           366.90         0.00         (N/A)         0.00           367.00         0.00         (N/A)         0.00           367.10         0.00         (N/A)         0.00           367.30         0.00         (N/A)         0.00           367.30         0.00         (N/A)         0.00           367.50         0.00         (N/A)         0.00           367.60         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.80         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.50         0.00         <	(π)	0.00	(1)(4)	0.00
366.70         0.00         (N/A)         0.00           366.70         0.00         (N/A)         0.00           366.80         0.00         (N/A)         0.00           367.00         0.00         (N/A)         0.00           367.10         0.00         (N/A)         0.00           367.20         0.00         (N/A)         0.00           367.30         0.00         (N/A)         0.00           367.40         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.80         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.60         0.00         (N/A)         0.00           368.70         0.00         <	366.50	0.00	(N/A)	0.00
306.70         0.00         (V/A)         0.00           366.80         0.00         (V/A)         0.00           367.00         0.00         (V/A)         0.00           367.10         0.00         (V/A)         0.00           367.20         0.00         (V/A)         0.00           367.30         0.00         (V/A)         0.00           367.40         0.00         (V/A)         0.00           367.50         0.00         (V/A)         0.00           367.70         0.00         (V/A)         0.00           367.70         0.00         (V/A)         0.00           367.70         0.00         (V/A)         0.00           367.70         0.00         (V/A)         0.00           367.90         0.00         (V/A)         0.00           368.70         0.00         (V/A)         0.00           368.70         0.00         (V/A)         0.00           368.80         0.00         (V/A)         0.00           368.90         0.00         (V/A)         0.00           368.90         0.00         (V/A)         0.00           369.90         0.00         <	300.00	0.00	(N/A) (N/A)	0.00
306.00         (V/A)         0.00           366.90         0.00         (V/A)         0.00           367.00         0.00         (V/A)         0.00           367.10         0.00         (V/A)         0.00           367.20         0.00         (V/A)         0.00           367.30         0.00         (V/A)         0.00           367.50         0.00         (V/A)         0.00           367.70         0.00         (V/A)         0.00           367.70         0.00         (V/A)         0.00           367.70         0.00         (V/A)         0.00           367.80         0.00         (V/A)         0.00           367.90         0.00         (V/A)         0.00           368.10         0.00         (V/A)         0.00           368.10         0.00         (V/A)         0.00           368.20         0.00         (V/A)         0.00           368.30         0.00         (V/A)         0.00           368.40         0.00         (V/A)         0.00           368.50         0.00         (V/A)         0.00           368.60         0.00         (V/A)	300.70	0.00	(N/A)	0.00
366.30         0.00         (V/A)         0.00           367.00         0.00         (V/A)         0.00           367.10         0.00         (V/A)         0.00           367.30         0.00         (V/A)         0.00           367.30         0.00         (V/A)         0.00           367.40         0.00         (V/A)         0.00           367.50         0.00         (V/A)         0.00           367.70         0.00         (V/A)         0.00           367.70         0.00         (V/A)         0.00           367.80         0.00         (V/A)         0.00           367.90         0.00         (V/A)         0.00           368.10         0.00         (V/A)         0.00           368.20         0.00         (V/A)         0.00           368.30         0.00         (V/A)         0.00           368.40         0.00         (V/A)         0.00           368.50         0.00         (V/A)         0.00           368.60         0.00         (V/A)         0.00           368.70         0.00         (V/A)         0.00           368.80         0.00         <	300.80	0.00	(N/A)	0.00
367.10 $0.00$ $(V/A)$ $0.00$ $367.10$ $0.00$ $(V/A)$ $0.00$ $367.30$ $0.00$ $(V/A)$ $0.00$ $367.40$ $0.00$ $(V/A)$ $0.00$ $367.50$ $0.00$ $(V/A)$ $0.00$ $367.50$ $0.00$ $(V/A)$ $0.00$ $367.50$ $0.00$ $(V/A)$ $0.00$ $367.80$ $0.00$ $(V/A)$ $0.00$ $367.80$ $0.00$ $(V/A)$ $0.00$ $367.80$ $0.00$ $(V/A)$ $0.00$ $368.00$ $0.00$ $(V/A)$ $0.00$ $368.00$ $0.00$ $(V/A)$ $0.00$ $368.30$ $0.00$ $(V/A)$ $0.00$ $368.50$ $0.00$ $(V/A)$ $0.00$ $368.70$ $0.00$ $(V/A)$ $0.00$ $368.70$ $0.00$ $(V/A)$ $0.00$ $368.70$ $0.00$ $(V/A)$ $0.00$ $368.90$	300.90	0.00	(N/A)	0.00
367.10         0.00         (N/A)         0.00           367.20         0.00         (N/A)         0.00           367.30         0.00         (N/A)         0.00           367.40         0.00         (N/A)         0.00           367.50         0.00         (N/A)         0.00           367.60         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.80         0.00         (N/A)         0.00           367.90         0.00         (N/A)         0.00           368.00         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.10         0.00         <	367.00	0.00	(N/A)	0.00
367.20         0.00         (N/A)         0.00           367.30         0.00         (N/A)         0.00           367.40         0.00         (N/A)         0.00           367.50         0.00         (N/A)         0.00           367.60         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.80         0.00         (N/A)         0.00           367.90         0.00         (N/A)         0.00           368.00         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.50         0.00         <	367.10	0.00	(N/A)	0.00
367.30 $0.00$ $(N/A)$ $0.00$ $367.40$ $0.00$ $(N/A)$ $0.00$ $367.50$ $0.00$ $(N/A)$ $0.00$ $367.70$ $0.00$ $(N/A)$ $0.00$ $367.80$ $0.00$ $(N/A)$ $0.00$ $367.80$ $0.00$ $(N/A)$ $0.00$ $367.80$ $0.00$ $(N/A)$ $0.00$ $368.10$ $0.00$ $(N/A)$ $0.00$ $368.20$ $0.00$ $(N/A)$ $0.00$ $368.30$ $0.00$ $(N/A)$ $0.00$ $368.50$ $0.00$ $(N/A)$ $0.00$ $368.50$ $0.00$ $(N/A)$ $0.00$ $368.60$ $0.00$ $(N/A)$ $0.00$ $368.80$ $0.00$ $(N/A)$ $0.00$ $368.90$ $0.00$ $(N/A)$ $0.00$ $369.30$ $0.00$ $(N/A)$ $0.00$ $369.30$ $0.00$ $(N/A)$ $0.00$ $369.70$	367.20	0.00	(N/A)	0.00
367.40         0.00         (N/A)         0.00           367.50         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.80         0.00         (N/A)         0.00           367.90         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.60         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.50         0.00         <	367.30	0.00	(N/A)	0.00
367.50         0.00         (N/A)         0.00           367.60         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.80         0.00         (N/A)         0.00           367.90         0.00         (N/A)         0.00           368.00         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.60         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           369.90         0.00         <	367.40	0.00	(N/A)	0.00
367.60         0.00         (N/A)         0.00           367.70         0.00         (N/A)         0.00           367.80         0.00         (N/A)         0.00           367.90         0.00         (N/A)         0.00           368.00         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         <	367.50	0.00	(N/A)	0.00
367.70         0.00         (N/A)         0.00           367.80         0.00         (N/A)         0.00           367.90         0.00         (N/A)         0.00           368.00         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.60         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.50         0.00         <	367.60	0.00	(N/A)	0.00
367.80         0.00         (N/A)         0.00           367.90         0.00         (N/A)         0.00           368.00         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.50         0.00         <	367.70	0.00	(N/A)	0.00
36/.90         0.00         (N/A)         0.00           368.00         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.50         0.00         <	367.80	0.00	(N/A)	0.00
368.00         0.00         (N/A)         0.00           368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.60         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         <	367.90	0.00	(N/A)	0.00
368.10         0.00         (N/A)         0.00           368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.60         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.80         0.00         <	368.00	0.00	(N/A)	0.00
368.20         0.00         (N/A)         0.00           368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           370.00         0.00         <	368.10	0.00	(N/A)	0.00
368.30         0.00         (N/A)         0.00           368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.60         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.10         0.42         <	368.20	0.00	(N/A)	0.00
368.40         0.00         (N/A)         0.00           368.50         0.00         (N/A)         0.00           368.60         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.10         0.42         <	368.30	0.00	(N/A)	0.00
368.50         0.00         (N/A)         0.00           368.60         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.30         2.19         <	368.40	0.00	(N/A)	0.00
368.60         0.00         (N/A)         0.00           368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         <	368.50	0.00	(N/A)	0.00
368.70         0.00         (N/A)         0.00           368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         <	368.60	0.00	(N/A)	0.00
368.80         0.00         (N/A)         0.00           368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         (N/A)         0.00           370.50         4.71         <	368.70	0.00	(N/A)	0.00
368.90         0.00         (N/A)         0.00           369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.20         1.19         (N/A)         0.00           370.40         3.37         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.60         6.19         (N/A)         0.00           370.70         7.80         <	368.80	0.00	(N/A)	0.00
369.00         0.00         (N/A)         0.00           369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.60         6.19         (N/A)         0.00           370.70         7.80         <	368.90	0.00	(N/A)	0.00
369.10         0.00         (N/A)         0.00           369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.60         6.19         (N/A)         0.00           370.70         7.80         (N/A)         0.00           370.80         9.53         <	369.00	0.00	(N/A)	0.00
369.20         0.00         (N/A)         0.00           369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.20         1.19         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.60         6.19         (N/A)         0.00           370.70         7.80         (N/A)         0.00           370.80         9.53         (N/A)         0.00	369.10	0.00	(N/A)	0.00
369.30         0.00         (N/A)         0.00           369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.20         1.19         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.60         6.19         (N/A)         0.00           370.70         7.80         (N/A)         0.00           370.80         9.53         (N/A)         0.00	369.20	0.00	(N/A)	0.00
369.40         0.00         (N/A)         0.00           369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.20         1.19         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.60         6.19         (N/A)         0.00           370.70         7.80         (N/A)         0.00           370.70         9.53         (N/A)         0.00	369.30	0.00	(N/A)	0.00
369.50         0.00         (N/A)         0.00           369.60         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.20         1.19         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.60         6.19         (N/A)         0.00           370.70         7.80         (N/A)         0.00           370.80         9.53         (N/A)         0.00	369.40	0.00	(N/A)	0.00
369.60         0.00         (N/A)         0.00           369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.20         1.19         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.60         6.19         (N/A)         0.00           370.70         7.80         (N/A)         0.00           370.80         9.53         (N/A)         0.00	369.50	0.00	(N/A)	0.00
369.70         0.00         (N/A)         0.00           369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.20         1.19         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.70         7.80         (N/A)         0.00           370.70         9.53         (N/A)         0.00	369.60	0.00	(N/A)	0.00
369.80         0.00         (N/A)         0.00           369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.20         1.19         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.60         6.19         (N/A)         0.00           370.70         7.80         (N/A)         0.00           370.80         9.53         (N/A)         0.00	369.70	0.00	(N/A)	0.00
369.90         0.00         (N/A)         0.00           370.00         0.00         (N/A)         0.00           370.10         0.42         (N/A)         0.00           370.20         1.19         (N/A)         0.00           370.30         2.19         (N/A)         0.00           370.40         3.37         (N/A)         0.00           370.50         4.71         (N/A)         0.00           370.60         6.19         (N/A)         0.00           370.70         7.80         (N/A)         0.00           370.80         9.53         (N/A)         0.00	369.80	0.00	(N/A)	0.00
370.00       0.00       (N/A)       0.00         370.10       0.42       (N/A)       0.00         370.20       1.19       (N/A)       0.00         370.30       2.19       (N/A)       0.00         370.40       3.37       (N/A)       0.00         370.50       4.71       (N/A)       0.00         370.60       6.19       (N/A)       0.00         370.70       7.80       (N/A)       0.00         370.80       9.53       (N/A)       0.00	369.90	0.00	(N/A)	0.00
370.10       0.42       (N/A)       0.00         370.20       1.19       (N/A)       0.00         370.30       2.19       (N/A)       0.00         370.40       3.37       (N/A)       0.00         370.50       4.71       (N/A)       0.00         370.60       6.19       (N/A)       0.00         370.70       7.80       (N/A)       0.00         370.80       9.53       (N/A)       0.00	370.00	0.00	(N/A)	0.00
370.20       1.19       (N/A)       0.00         370.30       2.19       (N/A)       0.00         370.40       3.37       (N/A)       0.00         370.50       4.71       (N/A)       0.00         370.60       6.19       (N/A)       0.00         370.70       7.80       (N/A)       0.00         370.80       9.53       (N/A)       0.00	370.10	0.42	(N/A)	0.00
370.30       2.19       (N/A)       0.00         370.40       3.37       (N/A)       0.00         370.50       4.71       (N/A)       0.00         370.60       6.19       (N/A)       0.00         370.70       7.80       (N/A)       0.00         370.80       9.53       (N/A)       0.00	370.20	1.19	(N/A)	0.00
370.40       3.37       (N/A)       0.00         370.50       4.71       (N/A)       0.00         370.60       6.19       (N/A)       0.00         370.70       7.80       (N/A)       0.00         370.80       9.53       (N/A)       0.00	370.30	2.19	(N/A)	0.00
370.50       4.71       (N/A)       0.00         370.60       6.19       (N/A)       0.00         370.70       7.80       (N/A)       0.00         370.80       9.53       (N/A)       0.00	370.40	3.37	(N/A)	0.00
370.60         6.19         (N/A)         0.00           370.70         7.80         (N/A)         0.00           370.80         9.53         (N/A)         0.00	370.50	4.71	(N/A)	0.00
370.70         7.80         (N/A)         0.00           370.80         9.53         (N/A)         0.00	370.60	6.19	(N/A)	0.00
370.80 9.53 (N/A) 0.00	370.70	7.80	(N/A)	0.00
	370.80	9.53	(N/A)	0.00

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Bentley Systems, Inc. Haestad Methods Solution Center

27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666

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### Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft³/s)		Tailwater Elevation (ft)	Convergence Error (ft)
370.90		10.36	(N/A)	0.00
371.00		10.62	(N/A)	0.00
(no O: Weir - 1 Culvert -	T			
1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert -				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert -				
(no Q: Weir - 1,Culvert -				
(no Q: Weir - 1,Culvert -				
(no Q: Weir - 1,Culvert -				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert - 1)				

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#### Composite Outflow Summary

**Contributing Structures** (no Q: Weir - 1,Culvert -1) Weir - 1,Culvert - 1 Weir - 1,Culvert - 1

Subsection: Outlet Input Data Label: OCS-B Scenario: Proposed Conditions 10 Year Storm Return Event: 10 years Storm Event: 10

Requested Pond Water Surface ElevationsMinimum (Headwater)366.50 ftIncrement (Headwater)0.10 ftMaximum (Headwater)371.00 ft

### **Outlet Connectivity**

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir	Weir - 1	Forward	Culvert - 1	370.00	371.00
Culvert-Circular	Culvert - 1	Forward	TW	366.50	371.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

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Structure ID: Weir - 1 Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	370.00 ft
Weir Length	4.00 ft
Weir Coefficient	3.33 (ft^0.5)/s
Structure ID: Culvert - 1 Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	15.0 in
Length	146.50 ft
Length (Computed Barrel)	146.52 ft
Slope (Computed)	0.017 ft/ft
Outlet Control Data	
Manning's n	0.013
Ке	0.200
Kb	0.023
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
К	0.0045
Μ	2.0000
С	0.0317
Υ	0.6900
T1 ratio (HW/D)	1.087
T2 ratio (HW/D)	1.189
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation. Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	367.86 ft	T1 Flow	4.80 ft <sup>3</sup> /s
T2 Elevation	367.99 ft	T2 Flow	5.49 ft <sup>3</sup> /s

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# Return Event: 10 years Storm Event: 10

Structure ID: TW Structure Type: TW Setup, DS Channel			
Tailwater Type Free Outfall			
Convergence Tolerances			
Maximum Iterations	30		
Tailwater Tolerance (Minimum)	0.01 ft		
Tailwater Tolerance (Maximum)	0.50 ft		
Headwater Tolerance (Minimum)	0.01 ft		
Headwater Tolerance (Maximum)	0.50 ft		
Flow Tolerance (Minimum)	0.001 ft <sup>3</sup> /s		
Flow Tolerance (Maximum)	10.000 ft <sup>3</sup> /s		

Scenario: Proposed Conditions 10 Year Storm

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# Return Event: 10 years Storm Event: 10

### Composite Outflow Summary

Water Surface	Flow	Tailwater Elevation	Convergence Error
Elevation	(ft³/s)	(ft)	(ft)
(π)	0.00	(1)(4)	0.00
366.50	0.00	(N/A)	0.00
366.60	0.00	(N/A)	0.00
366.70	0.00	(N/A)	0.00
366.80	0.00	(N/A)	0.00
366.90	0.00	(N/A)	0.00
367.00	0.00	(N/A)	0.00
367.10	0.00	(N/A)	0.00
367.20	0.00	(N/A)	0.00
367.30	0.00	(N/A)	0.00
367.40	0.00	(N/A)	0.00
367.50	0.00	(N/A)	0.00
367.60	0.00	(N/A)	0.00
367.70	0.00	(N/A)	0.00
367.80	0.00	(N/A)	0.00
367.90	0.00	(N/A)	0.00
368.00	0.00	(N/A)	0.00
368.10	0.00	(N/A)	0.00
368.20	0.00	(N/A)	0.00
368.30	0.00	(N/A)	0.00
368.40	0.00	(N/A)	0.00
368.50	0.00	(N/A)	0.00
368.60	0.00	(N/A)	0.00
368.70	0.00	(N/A)	0.00
368.80	0.00	(N/A)	0.00
368.90	0.00	(N/A)	0.00
369.00	0.00	(N/A)	0.00
369.10	0.00	(N/A)	0.00
369.20	0.00	(N/A)	0.00
369.30	0.00	(N/A)	0.00
369.40	0.00	(N/A)	0.00
369.50	0.00	(N/A)	0.00
369.60	0.00	(N/A)	0.00
369.70	0.00	(N/A)	0.00
369.80	0.00	(N/A)	0.00
369.90	0.00	(N/A)	0.00
370.00	0.00	(N/A)	0.00
370.10	0.42	(N/A)	0.00
370.20	1.19	(N/A)	0.00
370.30	2.19	(N/A)	0.00
370,40	3,37	(N/A)	0.00
370.50	4.71	(N/A)	0.00
370.60	6.19	(Ν/Δ)	0.00
370.00	7 20	(N/A)	0.00
370.70	9.50	(Ν/Δ)	0.00
570.00	0.50	("/")   ("/")	0.00

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# Composite Outflow Summary

Water Surface	Flow		Tailwater Elevation	Convergence Error
Elevation	(ft³/s)		(ft)	(ft)
(11)		10.36	(N/A)	0.00
371.00		10.62	(N/A) (N/A)	0.00
Contributing Structures				
(no Q: Weir - 1,Culvert -	Ī			
1)				
(no Q: Weir - 1,Culvert -				
(no Q: Weir - 1,Culvert -				
1)				
(no Q: Weir - 1,Culvert -				
(no O: Weir - 1 Culvert -				
1)				
(no Q: Weir - 1,Culvert -				
1) (no O: Weir - 1 Culvert -				
1)				
(no Q: Weir - 1,Culvert -				
1) (no O: Wair 1 Culvart				
(10 Q. Weil - 1,Culvert - 1)				
(no Q: Weir - 1,Culvert -				
1)				
(no Q: weir - 1,Cuivert - 1)				
, (no Q: Weir - 1,Culvert -				
1)				
(no Q: Weir - 1,Culvert -				
(no Q: Weir - 1,Culvert -				
1)				
(no Q: Weir - 1,Culvert -				
(no Q: Weir - 1,Culvert -				
1)				
(no Q: Weir - 1,Culvert -				
(no Q: Weir - 1,Culvert -				
1)				
(no Q: Weir - 1,Culvert -				
(no O: Weir - 1.Culvert -				
1)				
(no Q: Weir - 1,Culvert -				
1)				

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#### Composite Outflow Summary

**Contributing Structures** (no Q: Weir - 1,Culvert -1) Weir - 1,Culvert - 1 Weir - 1,Culvert - 1

PondPack CONNECT Edition [10.02.00.01] Page 413 of 448 Subsection: Outlet Input Data Label: OCS-B Scenario: Proposed Conditions 100 Year Storm

Storm Event:	100

Return Event: 100 years

Requested Pond Water Surface Elevations				
Minimum (Headwater) 366.50 ft				
0.10 ft				
Maximum (Headwater) 371.00 ft				

# **Outlet Connectivity**

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir	Weir - 1	Forward	Culvert - 1	370.00	371.00
Culvert-Circular	Culvert - 1	Forward	TW	366.50	371.00
Tailwater Settings	Tailwater			(N/A)	(N/A)

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Structure ID: Weir - 1 Structure Type: Rectangular Weir	
Number of Openings	1
Elevation	370.00 ft
Weir Length	4.00 ft
Weir Coefficient	3.33 (ft^0.5)/s
Structure ID: Culvert - 1 Structure Type: Culvert-Circular	
Number of Barrels	1
Diameter	15.0 in
Length	146.50 ft
Length (Computed Barrel)	146.52 ft
Slope (Computed)	0.017 ft/ft
Outlet Control Data	
Manning's n	0.013
Ке	0.200
Kb	0.023
Kr	0.000
Convergence Tolerance	0.00 ft
Inlet Control Data	
Equation Form	Form 1
К	0.0045
Μ	2.0000
С	0.0317
Y	0.6900
T1 ratio (HW/D)	1.087
T2 ratio (HW/D)	1.189
Slope Correction Factor	-0.500

Use unsubmerged inlet control 0 equation below T1 elevation. Use submerged inlet control 0 equation above T2 elevation

In transition zone between unsubmerged and submerged inlet control, interpolate between flows at T1 & T2...

T1 Elevation	367.86 ft	T1 Flow	4.80 ft <sup>3</sup> /s
T2 Elevation	367.99 ft	T2 Flow	5.49 ft <sup>3</sup> /s

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Structure ID: TW Structure Type: TW Setup, DS Channel			
Tailwater Type	Free Outfall		
Convergence Tolerances			
Maximum Iterations	30		
Tailwater Tolerance (Minimum)	0.01 ft		
Tailwater Tolerance (Maximum)	0.50 ft		
Headwater Tolerance (Minimum)	0.01 ft		
Headwater Tolerance (Maximum)	0.50 ft		
Flow Tolerance (Minimum)	0.001 ft <sup>3</sup> /s		
Flow Tolerance (Maximum)	10.000 ft <sup>3</sup> /s		

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface Elevation	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)	
(ft)				
366.50	0.00	(N/A)	0.00	
366.60	0.00	(N/A)	0.00	
366.70	0.00	(N/A)	0.00	
366.80	0.00	(N/A)	0.00	
366.90	0.00	(N/A)	0.00	
367.00	0.00	(N/A)	0.00	
367.10	0.00	(N/A)	0.00	
367.20	0.00	(N/A)	0.00	
367.30	0.00	(N/A)	0.00	
367.40	0.00	(N/A)	0.00	
367.50	0.00	(N/A)	0.00	
367.60	0.00	(N/A)	0.00	
367.70	0.00	(N/A)	0.00	
367.80	0.00	(N/A)	0.00	
367.90	0.00	(N/A)	0.00	
368.00	0.00	(N/A)	0.00	
368.10	0.00	(N/A)	0.00	
368.20	0.00	(N/A)	0.00	
368.30	0.00	(N/A)	0.00	
368.40	0.00	(N/A)	0.00	
368.50	0.00	(N/A)	0.00	
368.60	0.00	(N/A)	0.00	
368.70	0.00	(N/A)	0.00	
368.80	0.00	(N/A)	0.00	
368.90	0.00	(N/A)	0.00	
369.00	0.00	(N/A)	0.00	
369.10	0.00	(N/A)	0.00	
369.20	0.00	(N/A)	0.00	
369.30	0.00	(N/A)	0.00	
369.40	0.00	(N/A)	0.00	
369.50	0.00	(N/A)	0.00	
369.60	0.00	(N/A)	0.00	
369.70	0.00	(N/A)	0.00	
369.80	0.00	(N/A)	0.00	
369.90	0.00	(N/A)	0.00	
370.00	0.00	(N/A)	0.00	
370.10	0.42	(N/A)	0.00	
370.20	1.19	(N/A)	0.00	
370.30	2.19	(N/A)	0.00	
370.40	3.37	(N/A)	0.00	
370.50	4.71	(N/A)	0.00	
370.60	6.19	(N/A)	0.00	
370.70	7.80	(N/A)	0.00	
370.80	9.53	(N/A)	0.00	
I.				

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# Return Event: 100 years Storm Event: 100

### Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
370.90	10.36	(N/A)	0.00
371.00	10.62	(N/A)	0.00
(no Q: Weir - 1,Culvert -			
1)			
(no Q: weir - 1,Cuivert - 1)			
(no Q: Weir - 1,Culvert -			
1) (no O: Weir - 1.Culvert -			
1)			
(no Q: Weir - 1,Culvert -			
(no Q: Weir - 1,Culvert -			
1) (no Ou Wair 1 Culvert			
1)			
(no Q: Weir - 1,Culvert -			
1) (no O: Weir - 1,Culvert -			
1)			
(no Q: Weir - 1,Culvert - 1)			
, (no Q: Weir - 1,Culvert -			
1) (no O: Weir - 1 Culvert -			
1)			
(no Q: Weir - 1,Culvert -			
(no Q: Weir - 1,Culvert -			
1)			
(no Q: weir - 1,Cuivert - 1)			
(no Q: Weir - 1,Culvert -			
1) (no O: Weir - 1.Culvert -			
1)			
(no Q: Weir - 1,Culvert - 1)			
(no Q: Weir - 1,Culvert -			
1) (no.0: Weir - 1 Culvert -			
1)			
(no Q: Weir - 1,Culvert -			
±)			

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#### Composite Outflow Summary

**Contributing Structures** (no Q: Weir - 1,Culvert -1) Weir - 1,Culvert - 1 Weir - 1,Culvert - 1

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# Subsection: Outlet Input Data Label: Porous Pavement Scenario: Proposed Conditions 1 Year Storm

Return Event: 1 yea	rs
Storm Event:	1

Requested Pond Water Surface Elevations		
Minimum (Headwater)	364.71 ft	
Increment (Headwater)	0.50 ft	
Maximum (Headwater)	366.21 ft	

# **Outlet Connectivity**

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir	Weir - 1	Forward	TW	368.20	368.21
Tailwater Settings	Tailwater			(N/A)	(N/A)

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# Subsection: Outlet Input Data Label: Porous Pavement Scenario: Proposed Conditions 1 Year Storm

Structure ID: Weir - 1 Structure Type: Rectangular Weir			
Number of Openings	1		
Elevation	368.20 ft		
Weir Length	100.00 ft		
Weir Coefficient	3.33 (ft^0.5)/s		
Structure ID: TW Structure Type: TW Setup, DS Channel			
Tailwater Type	Free Outfall		
Convergence Tolerances			
Maximum Iterations	30		
Tailwater Tolerance (Minimum)	0.01 ft		
Tailwater Tolerance (Maximum)	0.50 ft		
Headwater Tolerance (Minimum)	0.01 ft		
Headwater Tolerance (Maximum)	0.50 ft		
Flow Tolerance (Minimum)	0.001 ft <sup>3</sup> /s		
Flow Tolerance (Maximum)	10.000 ft <sup>3</sup> /s		

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Subsection: Composite Rating Curve Label: Porous Pavement Scenario: Proposed Conditions 1 Year Storm

#### Return Event: 1 years Storm Event: 1

#### Composite Outflow Summary

None Contributing

Weir - 1 Weir - 1

Water Surface Elevation (ft)	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
366.71	0.00	(N/A)	0.00
367.21	0.00	(N/A)	0.00
367.71	0.00	(N/A)	0.00
368.20	0.00	(N/A)	0.00
368.21	0.33	(N/A)	0.00
Contributing Structures			
None Contributing			
None Contributing			

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#### Subsection: Outlet Input Data Label: Porous Pavement Scenario: Proposed Conditions 10 Year Storm

Return Event:	10 ye	ars
Storm E	vent:	10

Requested Pond Water Surface Elevations		
Minimum (Headwater)	364.71 ft	
Increment (Headwater)	0.50 ft	
Maximum (Headwater)	366.21 ft	

#### **Outlet Connectivity**

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir	Weir - 1	Forward	TW	368.20	368.21
Tailwater Settings	Tailwater			(N/A)	(N/A)

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#### Subsection: Outlet Input Data Label: Porous Pavement Scenario: Proposed Conditions 10 Year Storm

Structure ID: Weir - 1			
Structure Type: Rectangular Weir			
Number of Openings	1		
Elevation	368.20 ft		
Weir Length	100.00 ft		
Weir Coefficient	3.33 (ft^0.5)/s		
Structure ID: TW			
Structure Type: TW Setup, DS	Channel		
Tailwater Type	Free Outfall		
Convergence Tolerances			
Maximum Iterations	30		
Tailwater Tolerance (Minimum)	0.01 ft		
Tailwater Tolerance (Maximum)	0.50 ft		
Headwater Tolerance (Minimum)	0.01 ft		
Headwater Tolerance (Maximum)	0.50 ft		
Flow Tolerance (Minimum)	0.001 ft <sup>3</sup> /s		
Flow Tolerance (Maximum)	10.000 ft <sup>3</sup> /s		

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#### Subsection: Composite Rating Curve Label: Porous Pavement Scenario: Proposed Conditions 10 Year Storm

#### Return Event: 10 years Storm Event: 10

#### Composite Outflow Summary

None Contributing

Weir - 1 Weir - 1

Water Surface Elevation (ft)	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
366.71	0.00	(N/A)	0.00
367.21	0.00	(N/A)	0.00
367.71	0.00	(N/A)	0.00
368.20	0.00	(N/A)	0.00
368.21	0.33	(N/A)	0.00
Contributing Structures			
None Contributing			
None Contributing			

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#### Subsection: Outlet Input Data Label: Porous Pavement Scenario: Proposed Conditions 100 Year Storm

Return Event:	100 y	ears
Storm E	vent:	100

Requested Pond Water Surface Elevations		
Minimum (Headwater) 364.71 ft		
Increment (Headwater)	0.50 ft	
Maximum (Headwater) 366.21 ft		

#### **Outlet Connectivity**

Structure Type	Outlet ID	Direction	Outfall	E1 (ft)	E2 (ft)
Rectangular Weir	Weir - 1	Forward	TW	368.20	368.21
Tailwater Settings	Tailwater			(N/A)	(N/A)

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#### Subsection: Outlet Input Data Label: Porous Pavement Scenario: Proposed Conditions 100 Year Storm

Structure ID: Weir - 1 Structure Type: Rectangular Weir		
Number of Openings	1	
Elevation	368.20 ft	
Weir Length	100.00 ft	
Weir Coefficient	3.33 (ft^0.5)/s	
Structure ID: TW Structure Type: TW Setup, D	S Channel	
Tailwater Type Free Outfall		
Convergence Tolerances		
Maximum Iterations	30	
Tailwater Tolerance (Minimum)	0.01 ft	
Tailwater Tolerance (Maximum)	0.50 ft	
Headwater Tolerance (Minimum)	0.01 ft	
Headwater Tolerance (Maximum)	0.50 ft	
Flow Tolerance (Minimum)	0.001 ft <sup>3</sup> /s	
Flow Tolerance (Maximum)	10.000 ft <sup>3</sup> /s	

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#### Subsection: Composite Rating Curve Label: Porous Pavement Scenario: Proposed Conditions 100 Year Storm

#### Return Event: 100 years Storm Event: 100

#### Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
366.71	0.00	(N/A)	0.00
367.21	0.00	(N/A)	0.00
367.71	0.00	(N/A)	0.00
368.20	0.00	(N/A)	0.00
368.21	0.33	(N/A)	0.00
Contributing Structures			
None Contributing			
None Contributing			

None Contributing Weir - 1

Weir - 1

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#### **Average Infiltration Rating Table**

Elevation (Water	Area (Total)	How (Infiltration)	
Surrace)	(π²)	(It''/S)	
	ን በዩን 1	0.00	
367.00	2,007.1	0.00	
367.00	2,143.0	0.30	
367.20	2,200.5	0.31	
367.20	2,230.5	0.31	
367.30	2,317.7	0.32	
367.10	2,377.5	0.34	
367.60	2,498.7	0.35	
367.70	2,560.5	0.36	
367.80	2,623.1	0.36	
367.90	2,686.4	0.37	
368.00	2,750.5	0.38	
368.10	2,812.9	0.39	
368.20	2,876.1	0.40	
368.30	2,939.9	0.41	
368.40	3,004.4	0.42	
368.50	3,069.6	0.43	
368.60	3,135.6	0.44	
368.70	3,202.2	0.44	
368.80	3,269.5	0.45	
368.90	3,337.5	0.46	
369.00	3,406.3	0.47	
369.10	3,475.7	0.48	
369.20	3,545.8	0.49	
369.30	3,616.7	0.50	
369.40	3,688.2	0.51	
369.50	3,760.4	0.52	
369.60	3,833.4	0.53	
369.70	3,907.0	0.54	
369.80	3,981.3	0.55	
369.90	4,056.3	0.56	
3/0.00	4,132.1	0.57	
370.10	4,206.6	0.58	
370.20	4,281.9	0.59	
3/0.30	4,357.8	0.61	
3/0.40	4,434.3	0.62	
3/0.50	4,511.0	0.63	
3/0.60	4,589.4	0.64	
3/0./0	4,008.0 2 7 4 7 1	0.65	
370.80	4,/4/.2 1 700 1	0.00	
370.90	4,027.1 4 007 7	0.07	
5/1.00	4,907.7	0.08	

Return Event: 1 years Storm Event: 1

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#### Subsection: Interconnected Pond Routing Summary Label: INFILTRATION BASIN A Scenario: Proposed Conditions 1 Year Storm

Infiltration								
Infiltration Metho (Computed)	od	Average Infiltration Rate			-			
Infiltration Rate (	(Average)	6.0000 in/h		'n				
					-	<u> </u>		
Initial Conditions				(	Calculatio	n Tolerances		
Elevation (Startin Surface Compute	ng Water ed)	366.90	ft	F	low Tolera	ance (Minimum)	0.000	ft³/s
Volume (Starting	)	0.000	ft³	Ν	laximum I	terations	35	
Infiltration (Start	ing ICPM)	0.00	ft³/s	I	CPM Time	Step	0.050	hours
Outflow (Starting	)	0.00	ft³/s	C	utput Inc	rement	0.050	hours
Pond Inflow Infiltration Pond Outflow	F Time to Pea (hours) 12.1 12.5 12.1	Time to Peak (hours) 12.55 orward Flow k Flow (fi 100 550 150 Total Volum	Max Elevati (ft) 50 36 Peaks (Peak) t <sup>3</sup> /s) 4.10 0.38 2.67 me In	imum St ion 7.98 F Time t (hor	orage Volume (ft <sup>3</sup> ) 2,611.000 Reverse Flo o Peak urs) 0.000 0.000 0.000 0.000 Total Volu	ow Peaks Flow (Peak) (ft <sup>3</sup> /s) 0.00 0.00 0.00		
	Volume (ft³)	Dire	ection	Volu (ft	ime ³)	Direction		
Pond Inflow	14,470.0	000	Forward	(**	0.000	Reverse		
Infiltration	0.0	000	Reverse	9,	135.000	Forward		
Pond Outflow	0.0	000	Reverse	5,	294.000	Forward		
Mass Balance (f	t³)							
Volume (Initial IC	CPM)		0.000 ft <sup>3</sup>		-			
Volume (Total In	ICPM)	14,	470.000 ft <sup>3</sup>					
Volume (Total Ou	ut ICPM)	14,	429.000 ft <sup>3</sup>					
Volume (Ending)			37.000 ft <sup>3</sup>					
Elevation (Ending	<b>]</b> )		366.92 ft					
Difference			5.000 ft <sup>3</sup>					
Percent of Inflow (Interconnected Balance)	v Volume Pond Mass		0.0 %					

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#### Subsection: Interconnected Pond Routing Summary Label: INFILTRATION BASIN A Scenario: Proposed Conditions 10 Year Storm

Infiltration								
Infiltration Metho (Computed)	bd	Average Infiltration Rate			-			
Infiltration Rate	(Average)		6.0000 in,	/h	-			
Initial Conditions	3			C	Calculatio	n Tolerances		
Elevation (Startin Surface Compute	ng Water ed)	366.90	ft	F	low Tolera	ance (Minimum)	0.000	ft³/s
Volume (Starting	I)	0.000	ft³	Ν	laximum 1	Iterations	35	
Infiltration (Start	ing ICPM)	0.00	ft³/s	Ι	CPM Time	Step	0.050	hours
Outflow (Starting	3)	0.00	ft³/s	C	utput Inc	rement	0.050	hours
			Max	kimum St	orage			
		Time t	o Elevat	ion	Volume			
		Peak (bourg	(ft) ۱		(ft³)			
		(11001)	9) 050 26	0.41	7 460 000			
		12.	50 50	. 17.0	,100.000			
	F	orward Flo	w Peaks	F	Reverse Flo	ow Peaks		
	Time to Pea	ık Flo	w (Peak)	Time t	o Peak	Flow (Peak)		
	(hours)		(ft³/s)	(ho	urs)	(ft³/s)		
Pond Inflow	12.	100	8.85		0.000	0.00		
Infiltration	12.	950	0.51		0.000	0.00		
Pond Outflow	12.	100	4.27		0.000	0.00		
		Total Volu	ıme In		Total Volu	ime Out		
	Volume	Lorden Activ	irection	Volu	ime	Direction		
	(ft³)			(ft	3)			
Pond Inflow	32,534.	000	Forward		0.000	Reverse		
Infiltration	0.0	000	Reverse	19,	234.000	Forward		
Pond Outflow	0.	000	Reverse	13,	251.000	Forward		
Mass Balance (f	ˈť³)				_			
Volume (Initial I	CPM)		0.000 ft <sup>s</sup>	3				
Volume (Total In ICPM) 32,		2,534.000 ft <sup>3</sup>	3					
Volume (Total O	ut ICPM)	3	2,485.000 ft <sup>3</sup>	3				
Volume (Ending)	1		71.000 ft <sup>3</sup>	3				
Elevation (Ending	g)		366.93 ft					
Difference			-22.000 ft <sup>3</sup>	3				
Percent of Inflow	v Volume							
(Interconnected Balance)	Pond Mass		0.1 %	1				

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#### Subsection: Interconnected Pond Routing Summary Label: INFILTRATION BASIN A Scenario: Proposed Conditions 100 Year Storm

Infiltration							
Infiltration Method (Computed)	1	Average Infiltration Rate					
Infiltration Rate (A	Average)		5.0000 in/h	1			
Initial Conditions				Calculatio	n Tolerances		
Elevation (Starting	y Water	366.90	ft	Flow Tolera	ance (Minimum)	0.000	ft³/s
Volume (Starting)	.)	0.000	ft³	Maximum I	terations	35	
Infiltration (Startin	ng ICPM)	0.00	ft³/s	ICPM Time	Step	0.050	hours
Outflow (Starting)	5 - 7	0.00	ft³/s	Output Inc	rement	0.050	hours
			Maxii	num Storage			
		Time to	Elevatio	on Volume			
		(hours)	(11)	$(\Pi^3)$			
		12 350	370	73 12 791 000			
			010				
	Fo	orward Flow F	Peaks	Reverse Flo	ow Peaks		
	Time to Peal (hours)	k Flow ( (ft³	Peak) /s)	Time to Peak (hours)	Flow (Peak) (ft³/s)		
Pond Inflow	12.1	.00	17.02	0.000	0.00		
Infiltration	12.3	50	0.65	0.000	0.00		
Pond Outflow	12.0	50	7.78	0.000	0.00		
		Total Volume	In	Total Volu	me Out		
	Volume	Direc	tion	Volume	Direction		
	(ft³)			(ft³)			
Pond Inflow	65,439.0	00	Forward	0.000	Reverse		
Infiltration	0.0	00	Reverse	27,770.000	Forward		
Pond Outflow	0.0	00	Reverse	36,630.000	Forward		
	)						
Volume (Initial IC	PM)		0.000 ft <sup>3</sup>				
volume (Total In .		65,4	39.000 ft <sup>3</sup>				
	t iCPM)	64,4					
volume (Ending)		1,0					
Elevation (Ending)	)		507.32 ft				
Difference			30.000 ft <sup>3</sup>				
Percent of Inflow (Interconnected P Balance)	Volume ond Mass		0.0 %				

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#### **Average Infiltration Rating Table**

Elevation (Water Surface) (ft)	Area (Total) (ft²)	Flow (Infiltration) (ft³/s)
366.50	1,290.2	0.00
366.60	1,339.5	0.19
366.70	1,389.8	0.19
366.80	1,441.0	0.20
366.90	1,493.1	0.21
367.00	1,546.1	0.21
367.10	1,600.1	0.22
367.20	1,654.9	0.23
367.30	1,710.8	0.24
367.40	1,767.5	0.25
367.50	1,825.1	0.25
367.60	1,883.7	0.26
367.70	1,943.2	0.27
367.80	2,003.7	0.28
367.90	2,065.0	0.29
368.00	2,127.3	0.30
368.10	2,186.6	0.30
368.20	2,246.8	0.31
368.30	2,307.8	0.32
368.40	2,369.6	0.33
368.50	2,432.2	0.34
368.60	2,495.6	0.35
368.70	2,559.9	0.36
368.80	2,625.0	0.36
368.90	2,690.8	0.37
369.00	2,757.5	0.38
369.10	2,825.1	0.39
369.20	2,893.4	0.40
369.30	2,962.5	0.41
369.40	3,032.5	0.42
369.50	3,103.3	0.43
369.60	3,174.9	0.44
369.70	3,247.3	0.45
369.80	3,320.5	0.46
369.90	3,394.6	0.47
370.00	3,469.4	0.48
370.10	3,541.7	0.49
370.20	3,614.7	0.50
370.30	3,688.4	0.51
370.40	3,762.9	0.52
370.50	3,838.1	0.53
370.60	3,914.0	0.54
370.70	3,990.7	0.55
370.80	4,068.2	0.57

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#### **Average Infiltration Rating Table**

Elevation (Water Surface) (ft)	Area (Total) (ft²)	Flow (Infiltration) (ft <sup>3</sup> /s)
370.90	4,146.4	0.58
371.00	4,225.3	0.59

Return Event: 1 years Storm Event: 1

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#### Subsection: Interconnected Pond Routing Summary Label: INFILTRATION BASIN B Scenario: Proposed Conditions 1 Year Storm

Infiltration							
Infiltration Method (Computed)		A Infiltratio	verage n Rate				
Infiltration Rate (A	verage)		6.0000 in/	h			
Initial Conditions				Calculatio	n Tolerances		
Elevation (Starting	Water			Flow Tolera	ance (Minimum)	0.000	ft³/s
Surface Computed	)	366.50	ft				- / -
Volume (Starting)		0.000	ft³	Maximum 1	iterations	35	
Infiltration (Starting	g ICPM)	0.00	ft³/s	ICPM Time	Step	0.050	hours
Outflow (Starting)		0.00	ft³/s	Output Inc	rement	0.050	hours
			Maxi	imum Storage			
		Time to	Elevati	on Volume			
		Peak (hours)	(ft)	(ft³)			
		12 500	367	7 98 2 498 000			
		121000		2,1901000			
	F	orward Flow I	Peaks	Reverse Fl	ow Peaks		
-	Time to Pea	k Flow (	(Peak)	Time to Peak	Flow (Peak)		
	(hours)	(ft <sup>3</sup>	/s)	(hours)	(ft³/s)		
Pond Inflow	12.1	150	2.67	0.000	0.00		
Infiltration	12.	500	0.29	0.000	0.00		
Pond Outflow	0.0	000	0.00	0.000	0.00		
		Total Volum	In	Total Val	imo Out		
	Volume		tion	Volume	Direction		
	(ft <sup>3</sup> )	Dire		(ft <sup>3</sup> )	Direction		
Pond Inflow	5,294.0	000	Forward	0.000	Reverse		
Infiltration	0.0	000	Reverse	5,287.000	Forward		
Pond Outflow	0.0	000	Reverse	0.000	Forward		
Mass Balance (ft <sup>3</sup> )							
Volume (Initial ICP	M)		0.000 ft <sup>3</sup>				
Volume (Total In I	CPM)	5,2	94.000 ft <sup>3</sup>				
Volume (Total Out	ICPM)	5,2	87.000 ft <sup>3</sup>				
Volume (Ending)			6.000 ft <sup>3</sup>				
Elevation (Ending)			366.50 ft				
Difference			1.000 ft <sup>3</sup>				
Percent of Inflow V (Interconnected Pc Balance)	olume ond Mass		0.0 %				

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#### Subsection: Interconnected Pond Routing Summary Label: INFILTRATION BASIN B Scenario: Proposed Conditions 10 Year Storm

Infiltration								
Infiltration Metho (Computed)	od	A Infiltratio	verage on Rate					
Infiltration Rate (	(Average)		6.0000 in/l	n				
Initial Conditions	;			Calc	ulatio	n Tolerances		
Elevation (Startin	ng Water	266 50		Flow	Tolera	ance (Minimum)	0.000	ft³/s
Surface Compute	ed)	366.50	π					
Volume (Starting	)	0.000	ft <sup>3</sup>	Maxir	num I	terations	35	
Infiltration (Start	ing ICPM)	0.00	ft³/s	ICPM Outer	lime	Step	0.050	hours
	)	0.00	113/5	Outp		rement	0.050	nours
			Maxi	mum Storag	е			
		Time to	Elevatio	on Volu	me			
		(hours)	(11)	(It	-)			
		12.950	) 369	).40 6 <i>.</i> 41	3.000			
				,				
	F	orward Flow	Peaks	Reve	rse Flo	ow Peaks		
	Time to Pea	k Flow	(Peak)	Time to Pe	ak	Flow (Peak)		
	(hours)	(ft <sup>:</sup>	<sup>3</sup> /s)	(hours)		(ft <sup>3</sup> /s)		
Pond Inflow	12.	100	4.27	0	.000	0.00		
Infiltration	12.9	950	0.42	0	.000	0.00		
Pond Outflow	0.0	000	0.00	0	.000	0.00		
		Tatal Values	. T.e	Tab				
	Volume		e In ction	Volume	ai voiu	Direction		
	(ft <sup>3</sup> )	Dire	cuon	(ft <sup>3</sup> )		Direction		
Pond Inflow	13,251.0	000	Forward	0	.000	Reverse		
Infiltration	0.0	000	Reverse	13,238	.000	Forward		
Pond Outflow	0.0	000	Reverse	0	.000	Forward		
Mass Balance (f	t³)							
Volume (Initial IC	CPM)		0.000 ft <sup>3</sup>					
Volume (Total In	ICPM)	13,2	51.000 ft <sup>3</sup>					
Volume (Total Out ICPM) 13		13,2	38.000 ft <sup>3</sup>					
Volume (Ending)			12.000 ft <sup>3</sup>					
Elevation (Ending	g)		366.51 ft					
Difference			1.000 ft <sup>3</sup>					
Percent of Inflow (Interconnected   Balance)	v Volume Pond Mass		0.0 %					

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#### Subsection: Interconnected Pond Routing Summary Label: INFILTRATION BASIN B Scenario: Proposed Conditions 100 Year Storm

Infiltration							
Infiltration Metho (Computed)	d	Average Infiltration Rate					
Infiltration Rate (	Average)		6.0000 in/	h			
Initial Conditions				Calculation	n Tolerances		
Elevation (Startin Surface Computer	g Water d)	366.50	ft	Flow Tolera	Flow Tolerance (Minimum)		ft³/s
Volume (Starting)	)	0.000	ft³	Maximum I	terations	35	
Infiltration (Starti	ng ICPM)	0.00	ft³/s	ICPM Time	Step	0.050	hours
Outflow (Starting	)	0.00	ft³/s	Output Incr	rement	0.050	hours
		Time to Peak (hours)	Max Elevati (ft)	imum Storage on Volume (ft³)			
		12.35	370	0.57 10,259.000			
	F	orward Flow	Peaks	Reverse Flo	w Peaks		
	Time to Pea (hours)	ak Flow (ft	(Peak) ³/s)	Time to Peak (hours)	Flow (Peak) (ft³/s)		
Pond Inflow	12.	050	7.78	0.000	0.00		
Infiltration	12.	350	0.54	0.000	0.00		
Pond Outflow	12.	350	5.70	0.000	0.00		
		Total Volum	e In	Total Volu	me Out		
	Volume (ft³)	Dire	ction	Volume (ft³)	Direction		
Pond Inflow	36,630.	000	Forward	0.000	Reverse		
Infiltration	0.	000	Reverse	20,034.000	Forward		
Pond Outflow	0.	000	Reverse	15,230.000	Forward		
Mass Balance (ft	3)						
Volume (Initial IC	PM)		0.000 ft <sup>3</sup>				
Volume (Total In	ICPM)	36,6	30.000 ft <sup>3</sup>				
Volume (Total Ou	t ICPM)	35,2	64.000 ft <sup>3</sup>				
Volume (Ending)		1,3	45.000 ft <sup>3</sup>				
Elevation (Ending	)		367.30 ft				
Difference			21.000 ft <sup>3</sup>				
Percent of Inflow (Interconnected F Balance)	Volume Pond Mass		0.1 %				

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Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	3.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	366.71 ft
Volume (Initial)	0.000 ft <sup>3</sup>
Flow (Initial Outlet)	0.00 ft <sup>3</sup> /s
Flow (Initial Infiltration)	0.00 ft <sup>3</sup> /s
Flow (Initial, Total)	0.00 ft <sup>3</sup> /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ft <sup>3</sup> )	Area (ft²)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
366.71	0.00	0.000	15,097.870	0.00	0.00	0.00
367.21	0.00	3,019.574	15,097.870	1.05	1.05	34.60
367.71	0.00	6,039.148	15,097.870	1.05	1.05	68.15
368.20	0.00	8,998.330	15,097.870	1.05	1.05	101.03
368.21	0.33	9,058.722	15,097.870	1.05	1.38	102.03

Subsection: Elevation-Volume-Flow Table (Pond) Label: Porous Pavement Scenario: Proposed Conditions 10 Year Storm

Return Event:	10	ye	ars
Storm E	ven	t:	10

Infiltration	
Infiltration Method (Computed) Infiltration Rate (Average)	Average Infiltration Rate 3.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	366.71 ft
Volume (Initial)	0.000 ft <sup>3</sup>
Flow (Initial Outlet)	0.00 ft <sup>3</sup> /s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft <sup>3</sup> /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ft <sup>3</sup> )	Area (ft²)	Infiltration (ft³/s)	Flow (Total) (ft <sup>3</sup> /s)	2S/t + O (ft³/s)
366.71	0.00	0.000	15,097.870	0.00	0.00	0.00
367.21	0.00	3,019.574	15,097.870	1.05	1.05	34.60
367.71	0.00	6,039.148	15,097.870	1.05	1.05	68.15
368.20	0.00	8,998.330	15,097.870	1.05	1.05	101.03
368.21	0.33	9,058.722	15,097.870	1.05	1.38	102.03

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Infiltration	
Infiltration Method (Computed)	Average Infiltration Rate
Infiltration Rate (Average)	3.0000 in/h
Initial Conditions	
Elevation (Water Surface, Initial)	366.71 ft
Volume (Initial)	0.000 ft <sup>3</sup>
Flow (Initial Outlet)	0.00 ft <sup>3</sup> /s
Flow (Initial Infiltration)	0.00 ft <sup>3</sup> /s
Flow (Initial, Total)	0.00 ft <sup>3</sup> /s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ft <sup>3</sup> )	Area (ft²)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
366.71	0.00	0.000	15,097.870	0.00	0.00	0.00
367.21	0.00	3,019.574	15,097.870	1.05	1.05	34.60
367.71	0.00	6,039.148	15,097.870	1.05	1.05	68.15
368.20	0.00	8,998.330	15,097.870	1.05	1.05	101.03
368.21	0.33	9,058.722	15,097.870	1.05	1.38	102.03

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Peak Discharge	0.00 ft³/s
Time to Peak	8.000 hours
Hydrograph Volume	0.000 ft <sup>3</sup>

## HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row. Time Flow Flow Flow Flow

(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
0.000	0.00	0.00	(N/A)	(N/A)	(N/A)

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Peak Discharge	0.00 ft <sup>3</sup> /s
Time to Peak	8.000 hours
Hydrograph Volume	0.000 ft <sup>3</sup>
75-1	

# HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row. Time Flow Flow Flow Flow

(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
0.000	0.00	0.00	(N/A)	(N/A)	(N/A)

EDA-PDA.ppc 1/22/2021 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 PondPack CONNECT Edition [10.02.00.01] Page 442 of 448 Subsection: Pond Routed Hydrograph (total out) Label: Porous Pavement (OUT) Scenario: Proposed Conditions 100 Year Storm Return Event: 100 years Storm Event: 100

Peak Discharge	0.00 ft <sup>3</sup> /s
Time to Peak	8.000 hours
Hydrograph Volume	0.000 ft <sup>3</sup>

# HYDROGRAPH ORDINATES (ft³/s) Output Time Increment = 0.050 hours Time on left represents time for first value in each row. Time Flow Flow Flow Flow

(hours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)
0.000	0.00	0.00	(N/A)	(N/A)	(N/A)

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### **APPENDIX B**

## NYSDEC STORMWATER SIZING CALCULATIONS

INFILTRATION WORKSHEET		JMC Project:	19124
		Design Point:	<b>DL-1</b>
Infiltration Basins A and	B	Drainage Area:	PDA-1A
Site Data for Drainage Area to be Treated by Practice	2		
DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	Р	1.5	In
Impervious Area	Ι	1.72	Ac
Area	А	2.31	Ac
Percent Impervious	%I	74.46	%
Runoff Coefficient [0.05 + 0.009 x %I]	R <sub>V</sub>	0.72	CF
<b>TOTAL VOLUME Required</b> $[WQ_V = (P \times R_V \times A) / 12]$	WQv	9,049	CF
Minimum Infiltration Basin Area			
DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ <sub>V</sub>	9,049	CF
Depth of the Basin	d <sub>b</sub>	3.50	Ft
<b>Required Bottom Area of Infiltration Basin</b> $[A_p = WQ_V / d_b]$	A <sub>p</sub>	2,585	SF
Proposed Infiltration Basin			
DESCRIPTION	SYMBOL	VALUE	UNITS
Provided Bottom Area of Infiltration Basin		3,313.62	SF
Total Area of Infiltration Basin Provided	A <sub>p</sub>	9,133.01	SF
Runoff Reduction			
DESCRIPTION	SYMBOL	VALUE	UNITS
90% Runoff Reduction capacity	RR <sub>V</sub>	8,144	CF

### **INFILTRATION WORKSHEET**

JMC Project:	19124
Design Point:	<b>DL-1</b>
Drainage Area:	PDA-1B

### Porous Pavement #1

Site Data for Drainage Area to be Treated by Practice							
DESCRIPTION	SYMBOL	VALUE	UNITS				
Design Storm [90% Rainfall Event Number]	Р	1.5	In				
Impervious Area	Ι	0.47	Ac				
Area	А	0.57	Ac				
Percent Impervious	%I	82.91	%				
Runoff Coefficient [0.05 + 0.009 x %I]	R <sub>V</sub>	0.80	CF				
<b>TOTAL VOLUME Required</b> $[WQ_V = (P \times R_V \times A) / 12]$	WQ <sub>V</sub>	2,466	CF				

Minimum Porous Pavement Area								
DESCRIPTION	SYMBOL	VALUE	UNITS					
Water Quality Volume	$WQ_V$	2,466	CF					
Porosity	п	0.40	Ft / Day					
Trench Depth	d <sub>t</sub>	1.50	Ft					
<b>Surface Area Required</b> $[A_R = WQ_v / (n \times d_t)]$	A <sub>R</sub>	4,110	SF					

Proposed Porous Pavement								
DESCRIPTION	SYMBOL	VALUE	UNITS					
Surface Area of Porous Pavement Provided $[A_p]$	A <sub>p</sub>	15,098	SF					
Actual Volume Provided	WQ <sub>VP</sub>	9,049	CF					

Runoff Reduction							
DESCRIPTION	SYMBOL	VALUE	UNITS				
100% Runoff Reduction capacity	RR <sub>V</sub>	2,466	CF				

PROPRIETARY PRACTICE WORKSHEET			19124	
		-	Design Point:	DL-1
Continuous Deflective Separation	on Unit	I	PDA-1A	
		Rainfall Dist	ribution Type:	III
		Α	В	С
Coefficients for the equation unit peak	C <sub>0</sub>	-1.774	0.3301	2.4577
$[\mathbf{R} = \mathbf{I}_{\mathbf{a}} / \mathbf{P}]$	C <sub>1</sub>	1.8622	-0.7397	-0.4627
$[C_i = A x R^2 + B x R + C]$	<b>C</b> <sub>2</sub>	-0.0648	0.2276	-0.1932
Site Data for Drainage Area to be Treated by Pra-	ctice			
DESCRIPTION		SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]		Р	1.5	In
Impervious Area		Ι	1.72	Ac
Area		А	2.31	Ac
Percent Impervious		%I	74.46	%
Runoff Coefficient [0.05 + 0.009 x %I]		R <sub>V</sub>	0.72	CF
<b>TOTAL VOLUME Required</b> $[WQ_V = (P \times R_V \times A) / 12]$		WQ <sub>V</sub>	9,049	CF
Water Quality Peak Flow Calculation				
DESCRIPTION		SYMBOL	VALUE	UNITS
Water Quality Volume		WQ <sub>V</sub>	9,049	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm I	Depth]	Р	1.5	In
Time of Concentration		t <sub>c</sub>	0.0833	Hr
Runoff Volume $[Q = WQ_V / (A \times 3630)]$		Q	1.08	In
Curve Number [CN = $1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 \text{ QF}))$	) <sup>1/2</sup> ]	CN	95.83	
Curve Number		CN	96	
Initial Abstraction $[I_a = 200 / CN - 2]$		I <sub>a</sub>	0.09	In
Ratio $[\mathbf{R} = \mathbf{I}_a / \mathbf{P}]$		R	0.06	
$C_0 = A x R^2 + B x R + C$		C <sub>0</sub>	2.47	
$C_1 = A x R2 + B x R + C$		C <sub>1</sub>	-0.50	
$C_2 = A x R2 + B x R + C$		C <sub>2</sub>	-0.18	
Unit Peak Discharge		$q_u$	630.80	cfs/mi <sup>2</sup> /in
<b>Peak Discharge</b> $[Q_p = q_u \ge A \ge Q / 640]$		Q <sub>p</sub>	2.46	cfs
Proposed Device				
DESCRIPTION		SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided		Q <sub>p</sub>	3.38	cfs
Water Quality Volume Provided [WQ <sub>V</sub> = 640 x 3600 x Q <sub>P</sub> / $q_t$	]	WQ <sub>V</sub>	12,346	CF
Model Designation			Hydro Inter First Defense	rnational e FD-6HC
Quantity			1	

Deal Descar Defe			
Peak Bypass Rate	GVMDOL	VALUE	LINUTS
DESCRIPTION	SIMBOL	VALUE	UNITS
Peak Bypass Rate (100 Year Storm - See Hydrologic Calculations)	Q <sub>p100</sub>	17.02	cts
Provided Bypass Rate (First Detense FD-6HC)	$Q_{bp}$	32.00	cfs

PROPRIETARY PRACTICE WORKSHEET		JMC Project:	19124
	_	Design Point:	DL-1
Continuous Deflective Separation Unit	I	Drainage Area:	PDA-1C
	Rainfall Dist	ribution Type:	III
	А	В	С
Coefficients for the equation unit peak $C_0$	-1.774	0.3301	2.4577
$[\mathbf{R} = \mathbf{I}_a / \mathbf{P}] \qquad \mathbf{C}_1$	1.8622	-0.7397	-0.4627
$[C_i = A x R^2 + B x R + C] \qquad C_2$	-0.0648	0.2276	-0.1932
Site Data for Drainage Area to be Treated by Practice			
DESCRIPTION	SYMBOL	VALUE	UNITS
Design Storm [90% Rainfall Event Number]	Р	1.5	In
Impervious Area	Ι	0.51	Ac
Area	А	0.80	Ac
Percent Impervious	%I	64.44	%
Runoff Coefficient [0.05 + 0.009 x %I]	R <sub>V</sub>	0.63	CF
<b>TOTAL VOLUME Required</b> $[WQ_V = (P \times R_V \times A) / 12]$	WQ <sub>V</sub>	2,737	CF
Water Quality Peak Flow Calculation			
DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Volume	WQ <sub>V</sub>	2,737	CF
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	Р	1.5	In
Time of Concentration	t <sub>c</sub>	0.0833	Hr
Runoff Volume $[Q = WQ_V / (A \times 3630)]$	Q	0.94	In
Curve Number [CN = $1000 / (10 + 5P + 10Q - 10 \times (Q^2 + 1.25 QP)^{\frac{1}{2}}]$	CN	94.11	
Curve Number	CN	94	
Initial Abstraction $[I_a = 200 / CN - 2]$	Ia	0.13	In
Ratio $[R = I_a / P]$	R	0.08	
$C_0 = A x R^2 + B x R + C$	C <sub>0</sub>	2.47	
$C_1 = A x R2 + B x R + C$	C <sub>1</sub>	-0.51	
$C_2 = A x R2 + B x R + C$	C <sub>2</sub>	-0.17	
Unit Peak Discharge	q <sub>u</sub>	662.83	cfs/mi <sup>2</sup> /in
<b>Peak Discharge</b> $[Q_p = q_u \ge A \ge Q / 640]$	Qp	0.78	cfs
Proposed Device			
DESCRIPTION	SYMBOL	VALUE	UNITS
Water Quality Peak Flow Provided	Q <sub>p</sub>	0.84	cfs
Water Quality Volume Provided $[WQ_V = 640 \times 3600 \times Q_P / q_u]$	WQ <sub>V</sub>	2,920	CF
Model Designation		Hydro Inter First Defense	rnational e FD-3HC
Quantity	1	1	

Peak Bypass Rate			
DESCRIPTION	SYMBOL	VALUE	UNITS
Peak Bypass Rate (100 Year Storm - See Hydrologic Calculations)	Q <sub>p100</sub>	5.88	cfs
Provided Bypass Rate (First Defense FD-3HC)	Q <sub>bp</sub>	15.00	cfs

<b>RUNOFF REDUCTION VOLUME WORKSHEET</b>		JMC Project:	19124	
		Design Point:	DL-1	
Proposed Warehouse	Drainage Area:	PDA-1A, PDA-11	B, PDA-1C	
Total Water Quality Treatment Volume				
DESCRIPTION	SYMBOL	VALUE	UNITS	
Initial Water Quality Volume	WQ <sub>V</sub>	14,252	CF	
Adjusted Water Quality Volume	WQ <sub>V</sub>	3,641	CF	
Minimum Runoff Reduction Volume				
DESCRIPTION	SYMBOL	VALUE	UNITS	
Design Storm [90% Rainfall Event Number] or [1-yr Storm Depth]	Р	1.5	In	
Total Area of <i>new</i> Impervious Cover (B Soils)	Aic	2.19	Ac	
Hydrologic Soil Group (HSG) Specific Reduction Factor	S	0.40		
Total Area of <i>new</i> Impervious Cover (D Soils)	Aic	0.15	Ac	
Hydrologic Soil Group (HSG) Specific Reduction Factor	S	0.20		
Runoff Coefficient [0.05 + 0.009 x %I]	R <sub>V</sub>	0.95	CF	
Impervious Cover targeted for Runoff Reduction [S x Aic]	Ai	0.91	Ac	
TOTAL VOLUME Required [RR <sub>V</sub> = (P x R <sub>V</sub> x Ai) / 12]	RR <sub>V</sub>	4,687	CF	
Runoff Reduction Techniques (Volume)				
GREEN INFRASTRUCTURE PRACTICE / SMP	SYMBOL	VALUE	UNITS	
Infiltration Basin	RR <sub>V</sub>	8,144	CF	
Porous Pavement	$RR_V$	2,466	CF	
TOTAL	RR <sub>V</sub>	10,610	CF	

Runoff Reduction	
Is Total RR $_V > Adjusted WQ_V$ ?	YES
Is Total RR $_V > Minimum RR _V$ ?	YES

10,610

## **APPENDIX C**

## SOIL TESTING DATA

DESIGN	DATA S	SHEET - STOR	MWATER INF	LTRATION	SYSTEM		JOB NO.			19124
Owner	A&R I	Real Estate Ho	oldings, LLC	Address	100 Busines	s Park Drive				
Located a	at (Stree	et) <u>Business Pa</u>	ark Drive				Sec.108.03	Block 1	Lot	51
Municipal	lit <u>y</u> Arm	(Indicate nea	arest cross st.)		Watershed	Inland Long	g Island Sound	Basin		
SOIL INF	ILTRAT	ION TEST DA	TA							
Presoak I	Date:		11/15/2019		Run Date:		11/15/2019	9	_	
<b></b>									_	
Hole #	<u>e #</u>	C				INFIL	IRATION Water	Soil	_	
							Level	Rate		
	_			Elapse	Depth	То	Drop			
Hole Number	Run No	Start	Stop	l ime Min	From	surface water	In Inches	In/Hr Drop		
PT-1	1	1:00 PM	2:00 PM	60	30"	24"	18"	18"		
	2	2:05 PM	3:05 PM	60	30"	24"	6"	6"		
	3	3:05 PM	4:05 PM	60	30"	24"	6"	6"	_	
	4								_	
PT-2	1	1:05 PM	2:05 PM	60	42"	24"	24"	24"	_	
	2	2:10 PM	2:50 PM	40	42"	24"	24"	36"	_	
	3	3:06 PM	3:46 PM	40	42"	24"	24"	36"	_	
	4	3:46 PM	4:26 PM	40	42"	24"	24"	36"	_	
PT-3	1	1:10 PM	2:10 PM	60	36"	24"	19"	19"	_	
	2	2:15 PM	3:15 PM	60	36"	24"	13"	13"	_	
	3	3:15 PM	4:15 PM	60	36"	24"	9"	9"	_	
	4	4:16 PM	5:16 PM	60	36"	24"	7"	7"	_	
PT-4	1									
	2								_	
	3								_	
	4									

#### Notes:

Perc test done by:

RAR

 Tests to be repeated at same depth until approximately equal soil rates are obtained at each infiltration test hole. All data to be submitted for review.

2) Depth measurements to be made from top of hole. DO NOT REPORT INCREMENTS OF LESS THAN ONE INCH.

#### DESCRIPTION OF SOILS ENCOUNTERED IN TEST HOLES

DEPTH	HOLE NO.	DH-1	HOLE NO.	DH-2	_HOLE NO.	DH-3	HOLE NO.	DH-4
G.L.	0"-6"	1	0"-6"		0"-6"	1	0"-6"	1
6"	lopsoil	ļ	lopsoil	ł	lopsoil	ł	lopsoil	ļ
12"					6"-18"			
18"					Light Brown Sandy Loam	ł		
24"								
30"								
36"			0" 70"					
42"	Light Brown		Dark Sandy		Dark Sandy	Į	Light Brown	
48"	Sandy Loam		Loam		Loam		Sandy Loam	
60"					42"-72"			
66"					Gray Sand			
72"				<u> </u>		<u> </u>		
78"								
84"								
90"								<u> </u>
96"	-	<u> </u>						
WAS GROUNDV INDICATE LEVE INDICATE LEVE DEEP TESTS M/	VATER ENCOUNTER L AT WHICH GROUN L AT WHICH WATER ADE BY <u>Pe</u>	RED? ND WATER R RISES AF cord	Yes IS ENCOUNTEREE TER BEING ENCOU	) JNTERED D		2 @ 6', DH-3 9 3.5', DH-3 TESTS	8 @ 6', DH-4 @ 7.5 @ 5.5', DH-4 @ 6 11/15/2	5' 2019
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# **APPENDIX D**

# HYDRO INTERNATIONAL FIRST DEFENSE OPERATION AND MAINTENANCE MANUAL





# **Operation and Maintenance Manual**

# First Defense® and First Defense® High Capacity

Vortex Separator for Stormwater Treatment

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- 3 FIRST DEFENSE<sup>®</sup> BY HYDRO INTERNATIONAL
  - INTRODUCTION
  - OPERATION
  - POLLUTANT CAPTURE AND RETENTION
- 4 MODEL SIZES & CONFIGURATIONS
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#### 5 MAINTENANCE

- OVERVIEW
- MAINTENANCE EQUIPMENT CONSIDERATIONS
- DETERMINING YOUR MAINTENANCE SCHEDULE
- 6 MAINTENANCE PROCEDURES
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- 8 FIRST DEFENSE® INSTALLATION LOG
- 9 FIRST DEFENSE® INSPECTION AND MAINTENANCE LOG

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**DISCLAIMER:** Information and data contained in this manual is exclusively for the purpose of assisting in the operation and maintenance of Hydro International plc's First Defense<sup>®</sup>. No warranty is given nor can liability be accepted for use of this information for any other purpose. Hydro International plc has a policy of continuous product development and reserves the right to amend specifications without notice.

## Hydro Maintenance Services

Hydro International has been engineering stormwater treatment systems for over 30 years. We understand the mechanics of removing pollutants from stormwater and how to keep systems running at an optimal level.

## NOBODY KNOWS OUR SYSTEMS BETTER THAN WE DO



## AVOID SERVICE NEGLIGENCE

Sanitation services providers not intimately familiar with stormwater treatment systems are at risk of the following:

- Inadvertently breaking parts or failing to clean/replace system components appropriately.
- Charging you for more frequent maintenance because they lacked the tools to service your system properly in the first place.
- Billing you for replacement parts that might have been covered under your Hydro warranty plan
- Charging for maintenance that may not yet have been required.

## LEAVE THE DIRTY WORK TO US

Trash, sediment and polluted water is stored inside treatment systems until they are removed by our team with a vactor truck. Sometimes teams must physically enter the system chambers in order to prepare the system for maintenance and install any replacement parts. Services include but are not limited to:

- · Solids removal
- · Removal of liquid pollutants
- Replacement media installation (when applicable)



## **BETTER TOOLS, BETTER RESULTS**

Not all vactor trucks are created equal. Appropriate tools and suction power are needed to service stormwater systems appropriately. Companies who don't specialize in stormwater treatment won't have the tools to properly clean systems or install new parts.



## SERVICE WARRANTY

Make sure you're not paying for service that is covered under your warranty plan. Only Hydro International's service teams can identify tune-ups that should be on us, not you.

## TREATMENT SYSTEMS SERVICED BY HYDRO:

- Stormwwater filters
- Stormwater separators
- Baffle boxes
- Biofilters/biorention systems
- Storage structures
- Catch basins
- Stormwater ponds
- Permeable pavement



## SAVE TIME & MONEY: CALL HYDRO FOR A QUOTE

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# I. First Defense® by Hydro International

## Introduction

The First Defense<sup>®</sup> is an enhanced vortex separator that combines an effective and economical stormwater treatment chamber with an integral peak flow bypass. It efficiently removes total suspended solids (TSS), trash and hydrocarbons from stormwater runoff without washing out previously captured pollutants. The First Defense<sup>®</sup> is available in several model configurations (refer to *Section II. Model Sizes & Configurations*, page 4) to accommodate a wide range of pipe sizes, peak flows and depth constraints.

### Operation

The First Defense® operates on simple fluid hydraulics. It is selfactivating, has no moving parts, no external power requirement and is fabricated with durable non-corrosive components. No manual procedures are required to operate the unit and maintenance is limited to monitoring accumulations of stored pollutants and periodic clean-outs. The First Defense® has been designed to allow for easy and safe access for inspection, monitoring and clean-out procedures. Neither entry into the unit nor removal of the internal components is necessary for maintenance, thus safety concerns related to confined-spaceentry are avoided.

### Pollutant Capture and Retention

The internal components of the First Defense<sup>®</sup> have been designed to optimize pollutant capture. Sediment is captured and retained in the base of the unit, while oil and floatables are stored on the water surface in the inner volume (Fig.1).

The pollutant storage volumes are isolated from the built-in bypass chamber to prevent washout during high-flow storm events. The sump of the First Defense<sup>®</sup> retains a standing water level between storm events. This ensures a quiescent flow regime at the onset of a storm, preventing resuspension and washout of pollutants captured during previous events.

Accessories such as oil absorbent pads are available for enhanced oil removal and storage. Due to the separation of the oil and floatable storage volume from the outlet, the potential for washout of stored pollutants between clean-outs is minimized.

### Applications

- Stormwater treatment at the point of entry into the drainage line
- Sites constrained by space, topography or drainage profiles with limited slope and depth of cover
- Retrofit installations where stormwater treatment is placed on or tied into an existing storm drain line
- · Pretreatment for filters, infiltration and storage

### **Advantages**

- · Inlet options include surface grate or multiple inlet pipes
- Integral high capacity bypass conveys large peak flows without the need for "offline" arrangements using separate junction manholes
- Proven to prevent pollutant washout at up to 500% of its treatment flow
- Long flow path through the device ensures a long residence time within the treatment chamber, enhancing pollutant settling
- Delivered to site pre-assembled and ready for installation



Fig.1 Pollutant storage volumes in the First Defense®.

# II. Model Sizes & Configurations

The First Defense<sup>®</sup> inlet and internal bypass arrangements are available in several model sizes and configurations. The components of the First Defense<sup>®</sup>-4HC and First Defense<sup>®</sup>-6HC have modified geometries as to allow greater design flexibility needed to accommodate various site constraints.

All First Defense<sup>®</sup> models include the internal components that are designed to remove and retain total suspended solids (TSS), gross solids, floatable trash and hydrocarbons (Fig.2a - 2b). First Defense<sup>®</sup> model parameters and design criteria are shown in Table 1.

### First Defense® Components

- 1. Built-In Bypass
- 4. Floatables Draw-off Port
- 2. Inlet Pipe
- 5. Outlet Pipe
- 3. Inlet Chute
- 6. Floatables Storage
- 7. Sediment Storage
- 8. Inlet Grate or Cover





Fig.2a) First Defense<sup>®</sup>-4 and First Defense<sup>®</sup>-6; b) First Defense<sup>®</sup>-4HC and First Defense<sup>®</sup>-6HC, with higher capacity dual internal bypass and larger maximum pipe diameter.

First Defense <sup>®</sup> High Capacity	Diameter	Typical TSS Flow	S Treatment Rates	Peak Online	Maximum Pipe	Oil Storage	Typical Sediment	Minimum Distance from	Standard Distance from Outlet
Model Number		NJDEP Certified	106µm	Flow Rate	Diameter <sup>1</sup>	Capacity	Storage Capacity <sup>2</sup>	Outlet Invert to Top of Rim <sup>3</sup>	Invert to Sump Floor
	(ft / m)	(cfs / L/s)	(cfs / L/s)	(cfs / L/s)	(in / mm)	(gal / L)	(yd³ / m³)	(ft / m)	(ft / m)
FD-3HC	3 / 0.9	0.84 / 23.7	1.60 / 45.3	15 / 424	18 / 457	125 / 473	0.4 / 0.3	2.0 - 3.5 / 0.6 - 1.0	3.71 / 1.13
FD-4HC	4 / 1.2	1.50 / 42.4	1.88 / 50.9	18 / 510	24 / 600	191 / 723	0.7 / 0.5	2.3 - 3.9 / 0.7 - 1.2	4.97 / 1.5
FD-5HC	5 / 1.5	2.34 / 66.2	2.94 / 82.1	20 / 566	24 / 609	300 / 1135	1.1 / .84	2.5 - 4.5 / 0.7 - 1.3	5.19 / 1.5
FD-6HC	6 / 1.8	3.38 / 95.7	4.73 / 133.9	32 / 906	30 / 750	496 / 1,878	1.6 / 1.2	3.0 - 5.1 / 0.9 - 1.6	5.97 / 1.8
FD-8HC	8 / 2.4	6.00 / 169.9	7.52 / 212.9	50 / 1,415	48 / 1219	1120 / 4239	2.8 / 2.1	3.0 - 6.0 / 0.9 -1.8	7.40 / 2.2

<sup>1</sup>Contact Hydro International when larger pipe sizes are required.

<sup>2</sup>Contact Hydro International when custom sediment storage capacity is required.

<sup>3</sup>Minimum distance for models depends on pipe diameter.

**Hydro International** (Stormwater), 94 Hutchins Drive, Portland ME 04102 Tel: (207) 756-6200 Fax: (207) 756-6212 Web: www.hydro-int.com

## III. Maintenance

### Overview

The First Defense<sup>®</sup> protects the environment by removing a wide range of pollutants from stormwater runoff. Periodic removal of these captured pollutants is essential to the continuous, long-term functioning of the First Defense<sup>®</sup>. The First Defense<sup>®</sup> will capture and retain sediment and oil until the sediment and oil storage volumes are full to capacity. When sediment and oil storage capacities are reached, the First Defense<sup>®</sup> will no longer be able to store removed sediment and oil. Maximum pollutant storage capacities are provided in Table 1.

The First Defense<sup>®</sup> allows for easy and safe inspection, monitoring and clean-out procedures. A commercially or municipally owned sump-vac is used to remove captured sediment and floatables. Access ports are located in the top of the manhole.

Maintenance events may include Inspection, Oil & Floatables Removal, and Sediment Removal. Maintenance events do not require entry into the First Defense<sup>®</sup>, nor do they require the internal components of the First Defense<sup>®</sup> to be removed. In the case of inspection and floatables removal, a vactor truck is not required. However, a vactor truck is required if the maintenance event is to include oil removal and/or sediment removal.

### Maintenance Equipment Considerations

The internal components of the First Defense<sup>®</sup>-HC have a centrally located circular shaft through which the sediment storage sump can be accessed with a sump vac hose. The open diameter of this access shaft is 15 inches in diameter (Fig.3). Therefore, the nozzle fitting of any vactor hose used for maintenance should be less than 15 inches in diameter.



Fig.3 The central opening to the sump of the First Defense®-HC is 15 inches in diameter.

#### **Determining Your Maintenance Schedule**

The frequency of clean out is determined in the field after installation. During the first year of operation, the unit should be inspected every six months to determine the rate of sediment and floatables accumulation. A simple probe such as a Sludge-Judge<sup>®</sup> can be used to determine the level of accumulated solids stored in the sump. This information can be recorded in the maintenance log (see page 9) to establish a routine maintenance schedule.

The vactor procedure, including both sediment and oil / flotables removal, for a 6-ft First Defense® typically takes less than 30 minutes and removes a combined water/oil volume of about 765 gallons.

### First Defense® Operation and Maintenance Manual

#### Page | 6

#### **Inspection Procedures**

- Set up any necessary safety equipment around the access port or grate of the First Defense<sup>®</sup> as stipulated by local ordinances. Safety equipment should notify passing pedestrian and road traffic that work is being done.
- 2. Remove the grate or lid to the manhole.
- Without entering the vessel, look down into the chamber to inspect the inside. Make note of any irregularities. Fig.4 shows the standing water level that should be observed.
- Without entering the vessel, use the pole with the skimmer net to remove floatables and loose debris from the components and water surface.
- 5. Using a sediment probe such as a Sludge Judge<sup>®</sup>, measure the depth of sediment that has collected in the sump of the vessel.
- 6. On the Maintenance Log (see page 9), record the date, unit location, estimated volume of floatables and gross debris removed, and the depth of sediment measured. Also note any apparent irregularities such as damaged components or blockages.
- 7. Securely replace the grate or lid.
- 8. Take down safety equipment.
- Notify Hydro International of any irregularities noted during inspection.

### Floatables and Sediment Clean Out

Floatables clean out is typically done in conjunction with sediment removal. A commercially or municipally owned sumpvac is used to remove captured sediment and floatables (Fig.5).

Floatables and loose debris can also be netted with a skimmer and pole. The access port located at the top of the manhole provides unobstructed access for a vactor hose and skimmer pole to be lowered to the base of the sump.

#### Scheduling

- Floatables and sump clean out are typically conducted once a year during any season.
- Floatables and sump clean out should occur as soon as possible following a spill in the contributing drainage area.



Fig.4 Floatables are removed with a vactor hose (First Defense model FD-4, shown).

#### Recommended Equipment

- Safety Equipment (traffic cones, etc)
- · Crow bar or other tool to remove grate or lid
- Pole with skimmer or net (if only floatables are being removed)
- Sediment probe (such as a Sludge Judge<sup>®</sup>)
- · Vactor truck (flexible hose recommended)
- First Defense<sup>®</sup> Maintenance Log

**Hydro International** (Stormwater), 94 Hutchins Drive, Portland ME 04102 Tel: (207) 756-6200 Fax: (207) 756-6212 Web: www.hydro-int.com

## First Defense<sup>®</sup> Operation and Maintenance Manual

### Floatables and sediment Clean Out Procedures

- Set up any necessary safety equipment around the access port or grate of the First Defense<sup>®</sup> as stipulated by local ordinances. Safety equipment should notify passing pedestrian and road traffic that work is being done.
- 2. Remove the grate or lid to the manhole.
- **3.** Without entering the vessel, look down into the chamber to inspect the inside. Make note of any irregularities.
- Remove oil and floatables stored on the surface of the water with the vactor hose (Fig.5) or with the skimmer or net (not pictured).
- Using a sediment probe such as a Sludge Judge<sup>®</sup>, measure the depth of sediment that has collected in the sump of the vessel and record it in the Maintenance Log (page 9).
- Once all floatables have been removed, drop the vactor hose to the base of the sump. Vactor out the sediment and gross debris off the sump floor (Fig.5).
- 7. Retract the vactor hose from the vessel.
- 8. On the Maintenance Log provided by Hydro International, record the date, unit location, estimated volume of floatables and gross debris removed, and the depth of sediment measured. Also note any apparent irregularities such as damaged components, blockages, or irregularly high or low water levels.



Fig.5 Sediment is removed with a vactor hose (First Defense model FD-4, shown).

9. Securely replace the grate or lid.

## Maintenance at a Glance

Inspection	- Regularly during first year of installation
	- Every 6 months after the first year of installation
Oil and Floatables	- Once per year, with sediment removal
Removal	- Following a spill in the drainage area
Sediment Removal	- Once per year or as needed
	- Following a spill in the drainage area
NOTE: For most clear	n outs the entire volume of liquid does not need to be removed from the manhole. Only remove the
first few inches of oils	and floatables from the water surface to reduce the total volume of liquid removed during a clean out.



# First Defense® Installation Log

HYDRO INTERNATIONAL REFERENCE NUMBER:	
SITE NAME:	
SITE LOCATION:	
OWNER:	CONTRACTOR:
CONTACT NAME:	CONTACT NAME:
COMPANY NAME:	COMPANY NAME:
ADDRESS:	ADDRESS:
TELEPHONE:	TELEPHONE:
FAX:	FAX:

INSTALLATION DATE: / /

MODEL SIZE (CIRCLE ONE):	FD-4	FD-4HC	FD-6	FD-6HC
INLET (CIRCLE ALL THAT APPLY):	GRATED INL	ET (CATCH BASIN)	INLET PIPE (F	LOW THROUGH)



## First Defense<sup>®</sup> Inspection and Maintenance Log

Date	Initials	Depth of Floatables and Oils	Sediment Depth Measured	Volume of Sediment Removed	Site Activity and Comments



# DO IT RIGHT THE FIRST TIME

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# CALL 1 (888) 382-7808 TO SCHEDULE AN INSPECTION

# **Stormwater Solutions**

94 Hutchins Drive Portland, ME 04102

Tel: (207) 756-6200 Fax: (207) 756-6212 stormwaterinquiry@hydro-int.com

www.hydro-int.com

# **APPENDIX E**

# TEMPORARY EROSION AND SEDIMENT CONTROL INSPECTION AND MAINTENANCE CHECKLIST PERMANENT STORMWATER PRACTICE OPERATION, MAINTENANCE AND MANAGEMENT INSPECTION CHECKLISTS

JMC Project 19124 Proposed Warehouse 100 Business Park Drive Town of North Castle, NY

## **Temporary Erosion and Sediment Control Inspection and Maintenance Checklist**

Erosion and Sediment Control Measure	Inspection/Maintenance Intervals	Inspection/Maintenance Requirements
Stabilized Construction Access	Daily	<ul> <li>Periodic top dressing with additional aggregate as required</li> <li>Clean sediment in public right-of- ways immediately</li> </ul>
Silt Fence	Weekly + After Each Rain	<ul> <li>Remove &amp; redistribute sediment when bulges develop in the silt fence.</li> </ul>
Inlet Protection	Weekly + After Each Rain	<ul> <li>Remove sediment as necessary and replace filter fabric, crushed stone etc.</li> <li>Any broken and damaged components should be replaced.</li> <li>Check all materials for proper anchorage and secure as necessary.</li> </ul>

JMC Project 19124 Proposed Warehouse 100 Business Park Drive Town of North Castle, NY

Permanent Stormwater Management Practice Inspection and Maintenance
Checklist

Stormwater Management Practice	Inspection/Maintenance Intervals	Inspection/Maintenance Requirements
Stormwater Management Basin	Annually + After Major Storms	<ul> <li>Check adequacy of vegetation and ground cover; for evidence of embankment erosion, animal burrows, unauthorized plantings and cracking, bulging or sliding of dam, clear/properly functioning drains, seeps/leaks on downstream face, failure of slope protection or riprap. Repair/remove as necessary.</li> <li>Confirm emergency spillway is clear of obstructions and debris.</li> <li>Confirm all inlets and outlet structures/pipes are operating properly.</li> </ul>
Drain Inlets	Monthly	<ul> <li>Check for blockage and/or erosion at top of each inlet. Repair/remove as necessary.</li> <li>Check for sediment and debris collected within sumps and clean out as necessary.</li> </ul>

JMC Project 19124 Proposed Warehouse 100 Business Park Drive Town of North Castle, NY

## Permanent Stormwater Management Practice Inspection and Maintenance Checklist (Cont'd)

Stormwater Management Practice	Inspection/Maintenance Intervals	Inspection/Maintenance Requirements
Porous Pavement	Monthly and As Needed	<ul> <li>Ensure that paving area is clean of debris</li> <li>Ensure that paving dewaters between storms</li> <li>Ensure that the area is clean of sediments</li> </ul>
		<ul> <li>Mow upland and adjacent areas, and seed bare areas</li> </ul>
	Quarterly	• Vacuum sweep frequently to keep surface free of sediments
	Annually	<ul> <li>Inspect the surface for deterioration or spalling</li> </ul>
Hydro International First Defense Hydrodynamic Separator	(See Maintenance Guidelines in Appendix D)	<ul> <li>See Maintenance Guidelines Appendix D</li> </ul>

The owner/operator responsible for inspection and maintenance as outlined above:

A&R Real Estate Holdings, LLC Mr. Robert Troccoli 100 Business Park Drive Armonk, NY 10504 Phone: (718) 655-5450 Email: Rob@Jantile.com

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# **APPENDIX F**

# **CONTRACTOR'S CERTIFICATION**



Site Planning Civil Engineering Landscape Architecture Land Surveying Transportation Engineering Environmental Studies Entitlements Construction Services 3D Visualization Laser Scanning

JMC Project 19124 Proposed Warehouse 100 Business Park Drive Town of North Castle, NY

## **CONTRACTOR'S CERTIFICATION**

"I hereby certify under penalty of law that I understand and agree to comply with the terms and conditions of the SWPPP and agree to implement any corrective actions identified by the qualified inspector during a site inspection. I also understand that the owner or operator must comply with the terms and conditions of the most current version of the New York State Pollutant Discharge Elimination System ("SPDES") general permit for stormwater discharges from construction activities and that it is unlawful for any person to cause or contribute to a violation of water quality standards. Furthermore, I am aware that there are significant penalties for submitting false information, that I do not believe to be true, including the possibility of fine and imprisonment for knowing violations"

Company Name:	
Address:	
Telephone Number:	
Name and Title:	
Signature:	Date:
Permit Identification No.:	
Name and Title of Trained Contractor:	

### p:\2019\19124\drainage\reports\2020-03-23\_pd (swppp)\appendices\nys contractors certification.docx

JMC Planning Engineering Landscape Architecture & Land Surveying, PLLC | JMC Site Development Consultants, LLC

# **APPENDIX G**

DRAWINGS



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Michael Nowicki
Paul J. Dumont, EIT; Paul R. Sysak, RLA, ASLA
Fwd: 3-5538-00238_00001 (SD) PERMIT JURISDICTION REVIEW
Friday, May 29, 2020 9:13:26 AM

-----Original Message-----From: Michael Nowicki <ecolsol@aol.com> To: Christina.Pacella@dec.ny.gov Sent: Tue, Apr 28, 2020 9:38 am Subject: Re: 3-5538-00238\_00001 (SD) PERMIT JURISDICTION REVIEW

thanks Christina

Mike

-----Original Message-----From: Pacella, Christina (DEC) <Christina.Pacella@dec.ny.gov> To: 'Michael Nowicki' <ecolsol@aol.com> Cc: dec. sm. DEP. R3 <DEP.R3@dec.ny.gov>; Fisher, Joshua M (DEC) <Joshua.Fisher@dec.ny.gov>; townclerk@northcastleny.com <townclerk@northcastleny.com> Sent: Tue, Apr 28, 2020 9:36 am Subject: 3-5538-00238\_00001 (SD) PERMIT JURISDICTION REVIEW

Michael Nowicki Ecological Solutions, LLC Via email (ecolsol@aol.com)

RE: Proposed Warehouse - 100 Business Park Drive Town of North Castle, Westchester County DEC Facility ID# 3-5538-00238/00001 Permit Jurisdiction Screening

Dear Mr. Nowicki:

The New York State Department of Environmental Conservation (DEC or Department) has reviewed your inquiry received by this office on March 27, 2020. The project involves the construction of a 74,850-square-foot (SF), one-store warehouse building at the above-referenced address. Based upon our review of your inquiry and submitted materials, the Department offers the following comments:

### **PROTECTION OF WATERS**

The following stream is located within or near the site you indicated:

Name	Class	DEC Water Number	Index	Status
Byram River	C(T)	LIS 13		Protected

A Protection of Waters permit is required to physically disturb the bed or banks (up to 50 feet from stream) of any streams identified above as "protected." A time restriction may be required for protection of cold-water trout fisheries (waters classified under Article 15 of the Environmental Conservation Law (ECL) with a "T" or "TS" designation), beginning October 1 and ending April 30.

<u>If a permit is not required, please note</u>, however, you are still responsible for ensuring that work shall not pollute any stream or waterbody. Care shall be taken to stabilize any disturbed areas promptly after construction, and all necessary precautions shall be taken to prevent contamination of the stream or waterbody by silt, sediment, fuels, solvents, lubricants, or any other pollutant associated with the project.

## FRESHWATER WETLANDS

The project site is not within a New York State protected Freshwater Wetland.

## WATER QUALITY CERTIFICATION

If the United States Army Corps of Engineers (ACOE) requires a permit for work completed in or impacting a federal wetland or waters of the U.S., you will need a Section 401 Water Quality Certification from the Department. Please contact the ACOE at (917) 790-8411 for a determination.

### STATE-LISTED SPECIES

The DEC has reviewed the State's Natural Heritage records. No records of sensitive resources were identified by this review.

The absence of data does not necessarily mean that rare or state-listed species, natural communities, or other significant habitats do not exist on or adjacent to the proposed site. Rather, our files currently do not contain information which indicates their presence. For most sites, comprehensive field surveys have not been conducted. We cannot provide a definitive statement on the presence or absence of all rare or state-listed species or significant natural communities. Depending on the nature of the project and the conditions at the project site, further information from on-site surveys or other sources may be required to fully assess impacts on biological resources.

### STATE POLLUTION DISCHARGE ELIMINATION SYSTEM (SPDES) CONSTRUCTION

Since project activities will disturb over one acre of land, the project sponsor must obtain coverage under the current SPDES General Permit for Stormwater Discharge from Construction Activity (GP-0-20-001) and develop a Stormwater Pollution Prevention Plan (SWPPP) that conforms to requirements of the General Permit.

As this site is within a Municipal Separate Storm Sewer System (MS4) community, the municipality is responsible for review and acceptance of the SWPPP, and the MS-4 Acceptance Form must be submitted to the Department. For information on stormwater and the general permits, see the DEC website at <u>http://www.dec.ny.gov/chemical/8468.html</u>.

## CULTURAL RESOURCES

We have reviewed the statewide inventory of archaeological resources maintained by the New York State Museum and the New York State Office of Parks, Recreation, and Historic Preservation. These records indicate that the project is located within an area considered to be sensitive with regard to archaeological resources. The project sponsor should submit project materials to the New York State Historic Preservation Office's online Cultural Resource Information System (CRIS) to initiate the review process. Information on submitting to the system and access to it are available at <a href="http://www.nysparks.com/shpo/">http://www.nysparks.com/shpo/</a>.

## FEMA FLOODPLAIN

The project site is located within a Federal Emergency Management Agency (FEMA) Floodplain. The municipality will determine if any additional jurisdictions are applicable to the proposal.

### AIR RESOURCES

If the project activities include the installation of a stationary or portable combustion system that exceeds one of the following thresholds, then an air facility registration may be required:

- A maximum rated heat input capacity less than 10 million British Thermal Units (Btu) per hour burning fuels other than coal or wood; or
- A maximum rated heat input capacity of less than 1 million Btu/hr burning coal or wood.

For more information, please contact the DEC Division of Air Resources at (845) 256-3185.

## <u>OTHER</u>

Other permits from this Department or other agencies may be required for projects conducted on this property now or in the future. Also, regulations applicable to the location subject to this determination occasionally are revised and the project sponsor should, therefore, verify the need for permits if your project is delayed or postponed. This determination regarding the need for permits will remain effective for a maximum of one year. More information about DEC permits may be found on our website, www.dec.ny.gov, under "Regulatory" then "Permits and Licenses." Application forms may be downloaded at <a href="http://www.dec.ny.gov/permits/6081.html">http://www.dec.ny.gov/permits/6081.html</a>.

Please contact this office if you have questions regarding the above information. Thank you.

Sincerely, Christina Pacella Division of Environmental Permits Region 3, Telephone No. (845) 256-2250

Ecc: Joshua Fisher, NYSDEC Bureau of Ecosystem Health

Town of North Castle Town Clerk

### **Christina Pacella**

Environmental Engineering Technician, Division of Environmental Permits **New York State Department of Environmental Conservation** 21 South Putt Corners Road, New Paltz, NY 12561 P: (845) 256-2250 | F: (845) 255-4659 | <u>christina.pacella@dec.ny.gov</u> www.dec.ny.gov



August 10, 2020

**Regulatory Branch** 

# SUBJECT: Permit Application Number NAN-2020-00781-WRY by A&R Real Estate Holdings LLC

A&R Real Estate Holdings LLC 100 Business Park Drive Armonk, NY 10504

Dear Mr. Casola:

On August 3, 2020, the New York District, U.S. Army Corps of Engineers (Corps), received a request for Department of the Army authorization for the construction of a warehouse and attending features along the Byram River, in the Town of North Castle, Westchester County, New York.

The submittal entitled, "Proposed Warehouse, 100 Business Park Drive, Town of North Castle, New York" by JMC Planning, Engineering, Landscape Architecture, & Land Surveying, PLLC dated January 13, 2020 indicates the construction of a 74,850 square foot warehouse and attending features would occur above Ordinary High Water. The proposed activity would not involve the discharge of fill material into the Byram River or within the delineated adjacent wetland boundary.

Our review indicates that since the proposed work does not appear to include dredging or construction activities in or over any navigable waters of the United States, the placement of any dredged or fill material in any waters of the United States (including coastal or inland wetlands) or the accomplishment of any work affecting the course, location, condition or capacity of such areas, a Department of the Army permit, in accordance with 33 CFR 320-330, will not be required provided the proposed work is executed in accordance with the referenced material.

Care should be taken so that any fill or construction materials, including debris, do not enter the waterway to become a drift or pollution hazard. A No Permit Required (NPR) determination by the Corps:

- Does not obviate the requirement to obtain any other Federal, State, or local permits which may be necessary for your project;
- Does not constitute a federal evaluation of possible impacts to species protected under the Endangered Species Act. Projects that have the potential to impact federally listed species should contact the U.S. Fish and Wildlife Service and/or NOAA Fisheries Service; and,

 Does not constitute a federal evaluation of possible impacts to historic resources protected under Section 106 of the Natural Historic Preservation Act. Projects that have the potential to impact historic sites should contact the State Historic Preservation Officer in New York.

This NPR determination neither addresses nor includes any consideration for geographic jurisdiction on aquatic resources and shall not be interpreted as such.

In order for us to better serve you, please complete our Customer Service Survey located at <u>http://www.nan.usace.army.mil/Missions/Regulatory/CustomerSurvey.aspx</u>.

If any questions should arise concerning this matter, please contact Alexandra Ryan, of my staff, at (917) 790-8518.

Sincerely,

Rosita Miranda Chief, Western Section

Enclosures

cc: NYSDEC - Region 3 Town of North Castle



LJA

Leonard Jackson Associates Consulting Engineers

26 Firemens Memorial Drive . Pomona, New York 10970 . (845) 354-4382 . FAX (845) 354-4401

October 13, 2020

Town of North Castle Town Board 15 Bedford Road Armonk, NY 10504

Re: Warehouse Project 100 Business Park Drive North Castle, NY Hydrologic and Hydraulic Analyses of the Byram River – Reach 2 LJA # 19063

Dear Members of the Town Board:

Hydrologic and Hydraulic analyses were prepared to define the affects of the construction of the subject warehouse project on Base (100-year) Discharges and Flood Elevations on the Byram River and to address compliance with local Floodplain regulations which incorporate the regulations promulgated by the Federal Emergency Management Agency (FEMA).

The results of these analyses are numerically summarized in the attached tables. These results support the following conclusions.

Conclusions

- 1. The proposed project complies with Town of North Castle Floodplain regulations which include the regulations promulgated by the Federal Emergency Management Agency (FEMA).
- 2. The Hydraulic affect of the project is to yield minimal Base (100-year) flood elevation increases on the Byram River that only affect the project site. These increases are anticipated and comply with Floodplain regulations. No offsite structures are affected by these increases.
- 3. The Hydrologic affect of the project is to yield a deminimus increase in the Base (100-year) discharge rate on the Byram River that has no measurable affect on flood elevations.
- 4. The provision of compensating flood storage on the site to mitigate against flood storage displacement yields no measurable benefit on the Byram River. A waiver of this requirement from the Town Board has been requested.

#### Hydrologic Analysis Summary

The existing Base (100-year) Discharge rate on the Byram River in the reach of the proposed warehouse project is 996.0 cubic feet per second. Downstream of the confluence with the Wampus River it is 2576 cfs.

The displacement of flood storage resulting from the project, if not mitigated, will increase this Base (100-year) Discharge rate to 998.0 cfs; an increase of 2.0 cfs. The increase has been added downstream of the confluence to be 2580 cfs.

Leonard Jackson PE PLLC dba Leonard Jackson Associates

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# Leonard Jackson Associates

This increase in discharge rate can theoretically raise Base (100-year) flood elevations on the Byram River downstream of the project site in the range of 0.00 ft to 0.01ft (0 inches to 1/8 inch) during a Base (100-year) flood event. This theoretical increase in flood elevation range is academic. Our analysis procedures and theory are not accurate to 0.01 foot, nor can a flood elevation actually be measured to 0.01 foot. As a matter of perspective, a branch falling from a tree yields a ripple that exceeds 0.01foot rise in 100-year flood elevation. The number 0.01 feet is therefore meaningless other than to suggest that any increase in discharge rate must result in some theoretical increase in Base (100-year) flood elevation even though it cannot be measured and has no actual affect on flooding.

This theoretical increase in 100-year flood elevation could be mitigated by the provision of compensating flood storage. The cost of this mitigation however is great whereas it yields no measurable benefit. The Byram River system which conveys runoff from a twenty nine square mile drainage area at its mouth in Connecticut is simply too large to be hydrologically affected by the proposed warehouse project.

#### Hydraulic Analysis Summary

The hydraulic affects of the proposed warehouse project on Base (100-year) Flood Elevations on the Byram River were analyzed. Post project flood elevations were compared to existing condition Base (100-year) Flood Elevations.

Analyses were prepared for the project with and without the provision of compensating flood storage.

The results of these analyses show that whether or not compensating flood storage is provided, the proposed project will yield hydraulic increases in Base (100-year) flood elevations on the project site ranging from 0.2 to 0.4 feet and will affect no offsite structures.

These rises on the project site are permitted and conform to the FEMA regulations adopted within the Town of North Castle Floodplain Ordinance which permit 100-year Flood Elevation rises of up to one foot. The Floodplain Ordinance specifies that any proposed work within the "Fringe" segment of a "Floodplain" is permitted to yield a rise of up to one foot. Any proposed work within the "Floodway" segment of a "Floodplain" is permitted zero rise.

As no project work is proposed within the "Floodway"; by inspection the project yields zero rise for work within a Floodway, in compliance with the law.

The entire proposed project construction is located within the "Fringe" segment of the Floodplain where the resultant flood elevation increases on the site of 0.2 to 0.4 feet are less than half of the one foot rise permitted, hence the project complies with Floodplain regulations while not adversely affecting any offsite structures.

Leonard Jackson PE PLLC dba Leonard Jackson Associates

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# **Leonard Jackson Associates**

The following supporting summary tables are attached:

- Table 1. Byram River Reach 2, 100 year Flood Elevations Existing versus Post Project

   Conditions with compensating Flood Storage provided.
- Table 2. Byram River Reach 2, 100 year Flood Elevations Existing versus Post ProjectConditions with No Compensating Flood Storage provided.
- Table 3.
   Byram River Reach 2, 100 year Flood Elevations Post Project conditions with and without Compensating Flood Storage provided.

Also attached is our Byram River Reach 2 Hydrologic and Hydraulic analyses report detailing and documenting the methodology and the results of these analyses.

Very/truly yours, LEONARD JACKSON ASSOCIATES Leonard Jackson, P.E.

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LJ/k

Leonard Jackson PE PLLC dba Leonard Jackson Associates

Table 1

## Byram River Reach 2 100 Year Flood Elevations Existing Versus Post-Project Conditions With Compensating Flood Storage

	Section	Existing	Post-Project	Δ
	Number		with Comp Storage	
		Q = 996&2576 CFS	Q = 996&2576 CFS	_
1	6350	372.77	372.77	0
	6283	372.36	372.36	0
	5968	371.95	371.95	0
	5656	371.28	371.27	-0.01
1	5380	371.18	371.18	0
	5156	371.07	371.07	0
	5064	370.4	370.33	-0.07
	4996	370.24	370.13	-0.11
roject	4959	370.13	370.54	0.41
Site	4788	370.23	370.53	0.3
	4731	370.24	370.49	0.25
	4699	370.15	370.51	0.36
	4500	370.18	370.51	0.33
	4441	370.18	370.51	0.33
	4393	370.18	370.48	0.3
÷	4268	370.1	370.29	0.19
	4080	369.05	369.05	0
	3575	369.23	369.23	0
	3290	369.2	369.2	0
	2949	369.24	369.24	0
	2600	369.23	369.23	0
	2383	369.22	369.22	0
	2079	369.21	369.21	0
	1851	369.19	369.19	0
	1418	369.11	369.11	0
	1121	369.09	369.09	0
	661	369.05	369.05	0
	425	369.03	369.03	0
	50	367.45	367.45	0

## Table 2

## Byram River Reach 2 100 Year Flood Elevations Existing Versus Post-Project Conditions No Compensating Flood Storage

	Section	Existing	Post-Project	Δ
	Number		no Comp Storage	
		Q = 996&2576 CFS	Q = 998 & 2580 CFS	
	6350	372.77	372.77	0
	6283	372.36	372.36	0
	5968	371.95	371.96	0.01
	5656	371.28	371.3	0.02
4	5380	371.18	371.21	0.03
	5156	371.07	371.11	0.04
	5064	370.4	370.54	0.14
	4996	370.24	370.44	0.2
Project	4959	370.13	370.5	0.37
Site	4788	370.23	370.55	0.32
	4731	370.24	370.52	0.28
	4699	370.15	370.48	0.33
	4500	370.18	370.49	0.31
	4441	370.18	370.5	0.32
	4393	370.18	370.48	0.3
+	4268	370.1	370.3	0.2
	4080	369.05	369.05	0
	3575	369.23	369.24	0.01
	3290	369.2	369.21	0.01
	2949	369.24	369.24	0
	2600	369.23	369.23	0
	2383	369.22	369.23	0.01
	2079	369.21	369.22	0.01
	1851	369.19	369.19	0
	1418	369.11	369.12	0.01
	1121	369.09	369.09	0
	661	369.05	369.05	0
	425	369.03	369.03	0
	50	367.45	367.45	0

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## Byram River Reach 2 100 Year Flood Elevations Post-Project Conditions With and Without Compensating Flood Storage

	Section	Post-Project	Post-Project	Δ
	Number	without Comp Storage	with Comp Storage	
		Q = 998&2580 CFS	Q = 996&2576 CFS	
	6350	372.77	372.77	0
	6283	372.36	372.36	0
	5968	371.96	371.95	-0.01
	5656	371.3	371.27	-0.03
1	5380	371.21	371.18	-0.03
	5156	371.11	371.07	-0.04
	5064	370.54	370.33	-0.21
	4996	370.44	370.13	-0.31
Project	4959	370.5	370.54	0.04
Site	4788	370.55	370.53	-0.02
	4731	370.52	370.49	-0.03
	4699	370.48	370.51	0.03
	4500	370.49	370.51	0.02
	4441	370.5	370.51	0.01
	4393	370.48	370.48	0
•	4268	370.3	370.29	-0.01
	4080	369.05	369.05	0
	3575	369.24	369.23	-0.01
	3290	369.21	369.2	-0.01
	2949	369.24	369.24	0
	2600	369.23	369.23	0
	2383	369.23	369.22	-0.01
	2079	369.22	369.21	-0.01
	1851	369.19	369.19	0
	1418	369.12	369.11	-0.01
	1121	369.09	369.09	0
	661	369.05	369.05	0
	425	369.03	369.03	0
	50	367.45	367.45	0


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# Summary Letter & Results

LJA

Leonard Jackson Associates Consulting Engineers

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October 13, 2020

Town of North Castle Town Board 15 Bedford Road Armonk, NY 10504

Re: Warehouse Project 100 Business Park Drive North Castle, NY Hydrologic and Hydraulic Analyses of the Byram River – Reach 2 LJA # 19063

Dear Members of the Town Board:

Hydrologic and Hydraulic analyses were prepared to define the affects of the construction of the subject warehouse project on Base (100-year) Discharges and Flood Elevations on the Byram River and to address compliance with local Floodplain regulations which incorporate the regulations promulgated by the Federal Emergency Management Agency (FEMA).

The results of these analyses are numerically summarized in the attached tables. These results support the following conclusions.

Conclusions

- 1. The proposed project complies with Town of North Castle Floodplain regulations which include the regulations promulgated by the Federal Emergency Management Agency (FEMA).
- 2. The Hydraulic affect of the project is to yield minimal Base (100-year) flood elevation increases on the Byram River that only affect the project site. These increases are anticipated and comply with Floodplain regulations. No offsite structures are affected by these increases.
- 3. The Hydrologic affect of the project is to yield a deminimus increase in the Base (100-year) discharge rate on the Byram River that has no measurable affect on flood elevations.
- 4. The provision of compensating flood storage on the site to mitigate against flood storage displacement yields no measurable benefit on the Byram River. A waiver of this requirement from the Town Board has been requested.

#### Hydrologic Analysis Summary

The existing Base (100-year) Discharge rate on the Byram River in the reach of the proposed warehouse project is 996.0 cubic feet per second. Downstream of the confluence with the Wampus River it is 2576 cfs.

The displacement of flood storage resulting from the project, if not mitigated, will increase this Base (100-year) Discharge rate to 998.0 cfs; an increase of 2.0 cfs. The increase has been added downstream of the confluence to be 2580 cfs.

This increase in discharge rate can theoretically raise Base (100-year) flood elevations on the Byram River downstream of the project site in the range of 0.00 ft to 0.01ft (0 inches to 1/8 inch) during a Base (100-year) flood event. This theoretical increase in flood elevation range is academic. Our analysis procedures and theory are not accurate to 0.01 foot, nor can a flood elevation actually be measured to 0.01 foot. As a matter of perspective, a branch falling from a tree yields a ripple that exceeds 0.01foot rise in 100-year flood elevation. The number 0.01 feet is therefore meaningless other than to suggest that any increase in discharge rate must result in some theoretical increase in Base (100-year) flood elevation even though it cannot be measured and has no actual affect on flooding.

This theoretical increase in 100-year flood elevation could be mitigated by the provision of compensating flood storage. The cost of this mitigation however is great whereas it yields no measurable benefit. The Byram River system which conveys runoff from a twenty nine square mile drainage area at its mouth in Connecticut is simply too large to be hydrologically affected by the proposed warehouse project.

#### Hydraulic Analysis Summary

The hydraulic affects of the proposed warehouse project on Base (100-year) Flood Elevations on the Byram River were analyzed. Post project flood elevations were compared to existing condition Base (100-year) Flood Elevations.

Analyses were prepared for the project with and without the provision of compensating flood storage.

The results of these analyses show that whether or not compensating flood storage is provided, the proposed project will yield hydraulic increases in Base (100-year) flood elevations on the project site ranging from 0.2 to 0.4 feet and will affect no offsite structures.

These rises on the project site are permitted and conform to the FEMA regulations adopted within the Town of North Castle Floodplain Ordinance which permit 100-year Flood Elevation rises of up to one foot. The Floodplain Ordinance specifies that any proposed work within the "Fringe" segment of a "Floodplain" is permitted to yield a rise of up to one foot. Any proposed work within the "Floodway" segment of a "Floodplain" is permitted zero rise.

As no project work is proposed within the "Floodway"; by inspection the project yields zero rise for work within a Floodway, in compliance with the law.

The entire proposed project construction is located within the "Fringe" segment of the Floodplain where the resultant flood elevation increases on the site of 0.2 to 0.4 feet are less than half of the one foot rise permitted, hence the project complies with Floodplain regulations while not adversely affecting any offsite structures.

Leonard Jackson PE PLLC dba Leonard Jackson Associates

The following supporting summary tables are attached:

- Table 1. Byram River Reach 2, 100 year Flood Elevations Existing versus Post Project Conditions with compensating Flood Storage provided.
- Table 2. Byram River Reach 2, 100 year Flood Elevations Existing versus Post Project

   Conditions with No

   Compensating Flood Storage provided.
- Table 3. Byran River Reach 2, 100 year Flood Elevations Post Project conditions with and without Compensating Flood Storage provided.

Also attached is our Byram River Reach 2 Hydrologic and Hydraulic analyses report detailing and documenting the methodology and the results of these analyses.

Very/truly you'rs, LEONARD JACKSON ASSOCIATES

Leonard Jackson, P.E. LJ/ks P:\PROJECTS\/9\19063\MGMTDOCS\CorrespondenceOut\2020-9-17 Analyses letter of the Byram River.doc

### Table 1

### Byram River Reach 2 100 Year Flood Elevations Existing Versus Post-Project Conditions With Compensating Flood Storage

	Section	Existing	Post-Project	Δ
	Number		with Comp Storage	
		Q = 996&2576 CFS	Q = 996&2576 CFS	
	6350	372.77	372.77	0
	6283	372.36	372.36	0
	5968	371.95	371.95	0
	5656	371.28	371.27	-0.01
4	5380	371.18	371.18	0
	5156	371.07	371.07	0
	5064	370.4	370.33	-0.07
	4996	370.24	370.13	-0.11
Project	4959	370.13	370.54	0.41
Site	4788	370.23	370.53	0.3
	4731	370.24	370.49	0.25
	4699	370.15	370.51	0.36
	4500	370.18	370.51	0.33
	4441	370.18	370.51	0.33
	4393	370.18	370.48	0.3
+	4268	370.1	370.29	0.19
	4080	369.05	369.05	0
3	3575	369.23	369.23	0
	3290	369.2	369.2	0
	2949	369.24	369.24	0
	2600	369.23	369.23	0
	2383	369.22	369.22	0
	2079	369.21	369.21	0
	1851	369.19	369.19	0
	1418	369.11	369.11	0
	1121	369.09	369.09	0
	661	369.05	369.05	0
	425	369.03	369.03	0
	50	367.45	367.45	0

## Table 2

### Byram River Reach 2 100 Year Flood Elevations Existing Versus Post-Project Conditions No Compensating Flood Storage

1	Section	Existing	Post-Project	Δ
	Number		no Comp Storage	
		Q = 996&2576 CFS	Q = 998 & 2580 CFS	
	6350	372.77	372.77	0
	6283	372.36	372.36	0
	5968	371.95	371.96	0.01
	5656	371.28	371.3	0.02
1	5380	371.18	371.21	0.03
	5156	371.07	371.11	0.04
	5064	370.4	370.54	0.14
	4996	370.24	370.44	0.2
Project	4959	370.13	370.5	0.37
Site	4788	370.23	370.55	0.32
	4731	370.24	370.52	0.28
	4699	370.15	370.48	0.33
	4500	370.18	370.49	0.31
	4441	370.18	370.5	0.32
	4393	370.18	370.48	0.3
+	4268	370.1	370.3	0.2
	4080	369.05	369.05	0
	3575	369.23	369.24	0.01
	3290	369.2	369.21	0.01
	2949	369.24	369.24	0
	2600	369.23	369.23	0
	2383	369.22	369.23	0.01
	2079	369.21	369.22	0.01
	1851	369.19	369.19	0
	1418	369.11	369.12	0.01
	1121	369.09	369.09	0
	661	369.05	369.05	0
	425	369.03	369.03	0
	50	367.45	367.45	0

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### Byram River Reach 2 100 Year Flood Elevations Post-Project Conditions With and Without Compensating Flood Storage

	Section	Post-Project	Post-Project	Δ
	Number	without Comp Storage	with Comp Storage	
		Q = 998&2580 CFS	Q = 996&2576 CFS	
	6350	372.77	372.77	0
	6283	372.36	372.36	0
	5968	371.96	371.95	-0.01
	5656	371.3	371.27	-0.03
4	5380	371.21	371.18	-0.03
	5156	371.11	371.07	-0.04
	5064	370.54	370.33	-0.21
	4996	370.44	370.13	-0.31
Project	4959	370.5	370.54	0.04
Site	4788	370.55	370.53	-0.02
	4731	370.52	370.49	-0.03
	4699	370.48	370.51	0.03
	4500	370.49	370.51	0.02
	4441	370.5	370.51	0.01
	4393	370.48	370.48	0
+	4268	370.3	370.29	-0.01
	4080	369.05	369.05	0
	3575	369.24	369.23	-0.01
	3290	369.21	369.2	-0.01
	2949	369.24	369.24	0
	2600	369.23	369.23	0
	2383	369.23	369.22	-0.01
	2079	369.22	369.21	-0.01
	1851	369.19	369.19	0
	1418	369.12	369.11	-0.01
	1121	369.09	369.09	0
	661	369.05	369.05	0
	425	369.03	369.03	0
	50	367.45	367.45	0



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# Methodology

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#### Methodology

The Hydrologic and Hydraulic affects of the warehouse project on Reach 2 of the Byram River were analyzed utilizing the US Army Corps of Engineers programs HEC-HMS (Hydrologic) and HEC-RAS (Hydraulics).

To determine the Hydrologic affects of the project on Byram River discharge rates a hydrograph encompassing the Byram River from its confluence with the Wampus River up to and through the project site was prepared utilizing Soil Conservation Service (SCS) methodology. This hydrograph was calibrated so that, when routed through the existing condition flood storage volume versus discharge relationship on the studied reach of the Byram River, its 100-year peak discharge rate would match the FEMA 100-year discharge rate of 996 cubic feet per second (cfs) through that reach, as published in the Effective Flood Insurance Study (FIS) of Westchester County.

This hydrograph routing model is the basis for defining the effects of reducing the flood storage volume of the routing which yields a minor increase in discharge rate.

The next step in the analysis was to route this calibrated inflow hydrograph through the post-project condition reduced flood storage volume versus discharge relationship resulting from the projects construction where no compensating flood storage is provided. If compensating flood storage is provided there is no change in discharge and no hydrologic analysis is necessary.

The analysis subsequently showed that the reduced flood storage volume versus discharge relationship resulting from constructing the warehouse project increased the peak 100-year discharge rate on the Byram River from an existing 996.0 cfs to a post-project 998.0 cfs an increase of 2.0 cfs.

In order to prepare this Hydrologic Analysis it was necessary to derive the existing condition and post-project condition flood storage volume vs, discharge relationships on the studied Byram River Reach 2 from its confluence with the Wampus River to just above the project site. To do this a HEC-RAS Hydraulic model was prepared by stripping off cross sections from available topographic mapping.

On the site the existing topographic survey and post-project grading plan prepared for the project's design by JMC was utilized for stripping off HEC-RAS model cross sections. Off the site, Westchester County topographic mapping was utilized. In this manner HEC-RAS models were prepared for existing conditions and post-project with no compensating storage conditions.

The existing condition HEC-RAS model was then utilized to derive the existing condition flood storage volume versus discharge relationship on the river by running a multiple series of discharge rates.

HEC-RAS models calculate a backwater flood elevation profile and records the cumulative flood storage volume within the waterway starting from the first (downstream) cross section in the model up to each cross section within the model. The cumulative flood storage volume within the studied reach of the waterway for each of the multiple discharge rates is found in the detailed output data at the uppermost (last) cross section 6350 within the existing condition HEC-RAS model.

Deducting the cumulative flood storage at the confluence (section 2600) yields the cumulative flood storage between these cross sections in the study reach for each discharge rate.

This data yielded the flood storage volume versus discharge rate relationship tabulation for the existing condition routing of the study reach.

For post-project conditions it is was then necessary to reduce the flood storage volume at each discharge rate by deducting the flood storage volume displaced by the project at each discharge rate resulting from the project's construction.

The series of multiple discharge HEC-RAS runs also provides us with the elevation versus discharge rate at any cross section in the model. At a selected representative cross section on mid site, the elevation versus discharge rate on the site was defined.

Utilizing their site design drawings, JMC calculated the flood storage volume versus elevation on the site for existing conditions, and the reduced flood storage volume versus elevation on the site for post-project conditions when no compensating flood storage is provided. If compensating flood storage is provided, it is underground and discharge rates and flood storage volumes are unchanged by the project.

The JMC tabulation of flood storage volume versus elevation for existing and postproject conditions yields a tabulation and curve of the reduction in flood storage volume versus elevation <u>at the site</u>, resulting from the project.

Since the elevation versus discharge rate and elevation versus storage reduction at the site from the multiple discharge HEC-RAS runs, we therefore read off the reduction in flood storage volume versus any discharge rate for post-project conditions.

The reduction in flood storage volume versus discharge rate curve was then applied to the existing condition flood storage volume versus discharge rate on the studied Byram River Reach to derive the post-project flood storage volume versus discharge tabulation necessary to perform the post-project routing of the calibrated Byram River Reach 2 Hydrograph. This post-project routing shows the increased 100-year peak discharge rate at the river of 998.0 cfs and increases of 2.0 cfs.

Having defined the existing and post-project condition discharge rates (996 cfs & 998 cfs respectively) and having prepared existing condition and post-project HEC-RAS hydraulic models, we performed the following analyses for which elevation comparison tables were prepared.

- A. HEC-RAS Hydraulic Analyses
  - 1. Existing Conditions Multiple Discharges
  - 2. Post Project Conditions Multiple Discharges
  - Existing Condition Existing 100-year Discharges Q=996 & 2576
  - Post-Project Conditions Existing 100 year Discharges (with Compensating Flood Storage Provided)
    - Q= 996 & 2576 cfs (Unchanged)
  - Post-Project Conditions Post Project 100 year Discharges No Compensating Flood Storage provided.
- B. HEC-RAS Hydrologic Analyses
  - a) Existing Conditions Routing (Q=996 cfs)
  - b) Post-Project No Compensating Storage provided (Q= 998cfs)

These analyses show that the small rises on the project site for post-project conditions results from site grading, not from the deminimus increase in discharge rate on the Byram River resulting from flood storage displacement. They also show that the deminimus increases in discharge rate (if compensating flood storage is not provided) does not measurably affect flood elevations on the river.

We note that any rise in flood elevation yields an increased flood storage volume that will in turn reduce discharge rates that will reduce flood elevations. This effect has been ignored because the analyzed increase in discharge rate and flood elevations on the Byram River resulting from volume displacement are too small to have any meaning.

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# Appendix

## SCS HYDROGRAPH DATA

.

101 BUSINESS PARK WAREHOUSE PROJECT BYRAM RIVER REACH2 HYDRAULIC ANALYSIS 09/30/2020

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#### **DRAINAGE BASIN MAP**

Subbasin - B Area: 3.49 MI2 CN: 62.1 TC: 95 min Q= 1459 cfs

Subbasin - C Area: 0.156 MI2 CN: 60.38 TC: 39.4 min Q= 2576 cfs Subbasin - A Area: 4.59 MI2 CN: 66 TC: 132 min Q= 996 cfs



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0 1,500 3,000 Feet

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## **SOILS MAP**



Custom Soil Resource Report

M	AP LEGEND		MAP INFORMATION	
Area of Interest (AOI)	8	c	The soil surveys that comprise your AOI were mapped at	
Area of Interest (	AOI)	C/D	1:12,000.	
Soils		D	Please rely on the bar scale on each map sheet for map	
Soll Rating Polygons	-	Not rated or not available	measurements.	
A	Water Feetu		Source of Man: Natural Pacources Concentation Service	
A/D	water reatu	Streams and Canals	Web Soil Survey URL:	
В	Transmontal		Coordinate System: Web Mercator (EPSG:3857)	
B/D	mansportan	Rails	Mana from the Mich Coll Concerns are bound on the Web Manadar	
С		Internation Highwave	projection, which preserves direction and shape but distorts	
C/D	-	interstate Highways	distance and area. A projection that preserves area, such as the	
	100	US Routes	Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required	
	14.	Major Roads		
Not rated or not a	vailable	Local Roads	This product is generated from the USDA-NRCS certified data a	
Soil Rating Lines	Background	1	of the version date(s) listed below.	
A *	1961	Aerial Photography	Soil Survey Area: State of Connecticut	
A/D			Survey Area Data: Version 20, Jun 9, 2020	
ни В				
B/D			Soil Survey Area: Westchester County, New York	
C C				
C/D			Your area of interest (AOI) includes more than one soil survey	
			area. These survey areas may have been mapped at different scales with a different land use in mind at different times or at	
	127672		different levels of detail. This may result in map unit symbols, so	
<ul> <li>Not rated or not a</li> </ul>	vailable		properties, and interpretations that do not completely agree	
Soil Rating Points			across soil survey area boundaries.	
			Soil map units are labeled (as space allows) for map scales	
A/D			1:50,000 or larger.	
B			Data(a) parial images were photographed: Dec 31 2000 Oct	
B/D			16, 2017	
			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background	

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
3	Ridgebury, Leicester, and Whitman soils, 0 to 8 percent slopes, extremely stony	D	0.0	0.0%
45B	Woodbridge fine sandy loam, 3 to 8 percent slopes	C/D	0.0	0.0%
73C	Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky	В	0.1	0.0%
73E	Charlton-Chatfield complex, 15 to 45 percent slopes, very rocky	В	0.0	0.0%
84B	Paxton and Montauk fine sandy loams, 3 to 8 percent slopes	С	0.0	0.0%
84C	Paxton and Montauk fine sandy loams, 8 to 15 percent slopes	С	0.0	0.0%
308	Udorthents, smoothed	С	0.0	0.0%
w	Water		0.0	0.0%
Subtotals for Soil Survey Area			0.3	0.0%
Totals for Area of Interest			2.955.3	100.0%

### Table—Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
Ce	Catden muck, 0 to 2 percent slopes	B/D	54.9	1.9%
ChB	Charlton fine sandy loam, 3 to 8 percent slopes	В	31.9	1.1%
ChC	Charlton fine sandy loam, 8 to 15 percent slopes	В	37.4	1.3%
ChD	Charlton fine sandy loam, 15 to 25 percent slopes	В	26.4	0.9%
ChE	Charlton loam, 25 to 35 percent slopes	В	17.6	0.6%
CIE	Charlton loam, 25 to 35 percent slopes, very stony	В	4.6	0.2%
CrC	Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky	В	429.2	14.5%

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#### Custom Soil Resource Report

map unit symbol	Map unit name	Raung	ACTES IN AUI	Percent of AUI
CsD	Chatfield-Charlton complex, 15 to 35 percent slopes, very rocky	В	216.6	7.3%
CłC	Chatfield-Hollis-Rock outcrop complex, 0 to 15 percent slopes	В	74.8	2.5%
CuD	Chatfield-Hollis-Rock outcrop complex, 15 to 35 percent slopes	D	160.3	5.4%
Ff	Fluvaquents-Udifluvents complex, frequently flooded	A/D	225.9	7.6%
HnB	Hinckley loamy sand, 3 to 8 percent slopes	А	2.1	0.1%
HrF	Hollis-Rock outcrop complex, 35 to 60 percent slopes	D	72.8	2.5%
KnB	Knickerbocker fine sandy loam, 2 to 8 percent slopes	A	9.2	0.3%
KnC	Knickerbocker fine sandy loam, 8 to 15 percent slopes	A	3.4	0.1%
LcB	Leicester loam, 3 to 8 percent slopes, stony	A/D	109.0	3.7%
LeB	Leicester loam, 2 to 8 percent slopes, very stony	A/D	7.0	0.2%
NcA	Natchaug muck, 0 to 2 percent slopes	B/D	19.3	0.7%
PnB	Paxton fine sandy loam, 3 to 8 percent slopes	с	306.3	10.4%
PnC	Paxton fine sandy loam, 8 to 15 percent slopes	с	279.7	9.5%
PnD	Paxton fine sandy loam, 15 to 25 percent slopes	С	40.3	1.4%
Pw	Pompton silt loam, loamy substratum	B/D	12.3	0.4%
Ra	Raynham silt loam	C/D	9.2	0.3%
RdB	Ridgebury complex, 3 to 8 percent slopes	D	57.9	2.0%
RhB	Riverhead loam, 3 to 8 percent slopes	A	28.5	1.0%
RhC	Riverhead loam, 8 to 15 percent slopes	A	16.5	0.6%
RhD	Riverhead loam, 15 to 25 percent slopes	A	15.5	0.5%
Sh	Sun loam	C/D	38.9	1.3%

#### Custom Soil Resource Report

			T	
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
Sm	Sun loam, extremely stony	C/D	1.0	0.0%
SuB	Sutton loam, 3 to 8 percent slopes	B/D	29.5	1.0%
Ub	Udorthents, smoothed	В	224.5	7.6%
Uc	Udorthents, wet substratum	A/D	12.0	0.4%
Uf	Urban land		7.3	0.2%
UvB	Urban land-Riverhead complex, 2 to 8 percent slopes		11.7	0.4%
UwB	Urban land-Woodbridge complex, 3 to 8 percent slopes	D	2.8	0.1%
w	Water		199.4	6.7%
WdB	Woodbridge loam, 3 to 8 percent slopes	C/D	124.3	4.2%
WdC	Woodbridge loam, 8 to 15 percent slopes	C/D	34.9	1.2%
Subtotals for Soil Survey Area			2,955.0	100.0%
Totals for Area of Inter	est		2,955.3	100.0%

#### Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

# **CURVE NUMBER CALCULATION**

OBJECTID <sup>*</sup>	HSG	Shape_Area (square meter)	Shape_Area (square mi2)	CN	AreaxCN
1	А	8198.555176	0.003165	98	0.067599
2	A/D	1675.58426	0.000646947	98	0.013816
3	В	18366.01272	0.007091157	98	0.151432
4	B/D	61.262541	2.36536E-05	98	0.000505
5	С	656968.3067	0.253656881	98	5.416871
6	C/D	2708.146581	0.001045621	98	0.022329
7	D	968.795287	0.000374054	98	0.007988
8	А	79311.04926	0.030622167	39	0.260241
9	A/D	424807.6538	0.164019152	39	1.393911
10	В	988518.7047	0.381669206	61	5.073325
11	B/D	108928.5782	0.042057559	61	0.559049
12	С	740450.4733	0.285889526	74	4.610051
13	C/D	243381.1614	0.093969992	74	1.515293
14	D	89014.96998	0.034368872	80	0.599144
15	А	21145.78624	0.008164434	51	0.090734
16	A/D	145662.5711	0.056240633	51	0.625023
17	В	349354.0641	0.134886358	68	1.998723
18	B/D	9585.993789	0.003701173	68	0.054843
19	С	134852.6123	0.052066885	79	0.896323
20	C/D	60264.47564	0.023268244	79	0.400559
21	D	35531.18069	0.013718666	84	0.251112
22	А	15812.87226	0.006105384	57	0.075834
23	A/D	60984.49187	0.023546244	57	0.292464
24	В	185964.9706	0.071801477	72	1.126527
25	B/D	12882.39203	0.004973919	72	0.078038
26	С	40354.12284	0.015580814	81	0.275012
27	C/D	20942.98724	0.008086133	81	0.142726
28	D	44042.09091	0.017004746	86	0.318672
29	A/D	5096.71259	0.001967852	77	0.033019
30	В	47173.75237	0.018213888	85	0.337363
31	B/D	3507.857267	0.001354391	85	0.025086
32	С	4676.276441	0.00180552	90	0.03541
33	C/D	2655.377192	0.001025247	90	0.020107
34	D	7366.147091	0.002844085	92	0.057017
35	В	1524.710079	0.000588694	96	0.012315
36	С	4214.763873	0.001627329	96	0.034043
37	D	130.665349	5.04502E-05	96	0.001055
38	A	140065.602	0.054079631 .	36	0.42424
39	A/D	469297.5688	0.181196804	36	1.421441
40	В	2150207.408	0.830199722	60	10.85449
41	B/D	76291.10911	0.029456162	60	0.385126
42	С	1113355.996	0.429869153	73	6.838091
43	C/D	344066.8784	0.132844964	73	2.113215
44	D	965197.7076	0.372664918	79	6.415365
45	A	3148.995298	0.001215834	30	0.007948
46	A/D	12061.13343	0.00465683	30	0.030443

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		Total Area=	4.58906557	CN	66.00709
80	C	629.214	0.000242941	81	0.004288
79	В	847.1325	0.00032708	71	0.00506
78	D	5622.992831	0.00217105	83	0.039267
77	C/D	46386.01256	0.01790974	77	0.300508
76	С	41067.83194	0.015856379	77	0.266054
75	B/D	211432.5245	0.081634554	66	1.174069
74	В	23854.03107	0.009210093	66	0.13246
73	A/D	261051.1127	0.100792398	45	0.988362
72	A	8541.889075	0.003298042	45	0.03234
71	D	25024.11748	0.009661866	80	0.168433
70	C/D	4712.183337	0.001819384	74	0.029338
69	С	94009.45941	0.036297255	74	0.585304
68	B/D	16180.40772	0.00624729	61	0.083042
67	В	95001.34557	0.036680225	61	0.487571
66	A/D	1967.997518	0.000759848	39	0.006458
65	C	12417.83731	0.004794554	74	0.077314
64	В	3062.49959	0.001182438	62	0.015975
63	A/D	564.974847	0.000218138	62	0.002947
62	C/D	267.450283	0.000103263	83	0.001868
61	C	1783.753308	0.000688711	77	0.011556
60	B/D	336.616992	0.000129969	67	0.001898
59	A/D	4488.523864	0.001733029	48	0.018127
58	D	83603.90491	0.032279648	77	0.541621
57	C/D	104400.8164	0.040309381	70	0.614865
56	C	426729.4547	0.164761164	70	2.513209
55	B/D	27105.63426	0.010465544	55	0.12543
54	В	335507.965	0.12954035	55	1.552542
53	A/D	41123.20237	0.015877757	30	0.103797
52	A	28406.53047	0.010967823	30	0.0717
51	D	5180.205247	0.002000088	77	0.03356
50	C/D	9377.762843	0.003620774	70	0.05523
49	C	56026.76784	0.021632056	70	0.329968
48	B/D	1800	0.000694984	55	0.008329
4/	D	30334.30333	0.021750905	55	0.200005

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## TIME OF CONCENTRATION CALCULATION

### LEONARD JACKSON ASSOCIATES

26 Firemens Memorial Drive Pomona, NY 10970 845-354-4382 Worksheet 3: Time of Concentration (Tc) Calculations

PROJECT:	Warehouse		JOB	#: 19063	BY: SB
LOCATION:	Westchester,	NY	DAT	E: Jan, 17	
Mark One:	Existing	Developed			
	То	Design Point:	#1		
Time of	Concentration	thru Sub-Area:		Byram River	

Notes: Space for as many as two segments per flow type can be used for each worksheet. Include a map, schematic or description of flow segments.

Sheet Flow: Se	egment ID:					
1. Surface description (table 3-1)	f	orest				
2. Manning's Roughness Coeff., n (table 3-1)		0.45				
3. Flow Length, L (total $<$ or $=$ to 100 ft.)	(ft.)	250				
4. Two year, 24 hr rainfall, P2	(in.)	3.58				
5. Land Slope, S	(ft./ft.)	0.008				
6. $T_t = \underline{0.007 (nL)}^{0.8}$ Compute $T_t =$ P 2 <sup>0.5</sup> S <sup>0.4</sup>	(hr.)	1.12		= [	1.12	(hrs.)
Shallow Concentrated Flow: So	egment ID:			1		
7. Surface description (paved or unpaved)		unpaved	paved			
8. Flow Length, L	(ft.)	14000				
9. Watercourse Slope, s	(ft./ft.)	0.024				
10. Average Velocity, V (figure 3-1)	(ft./s)	2.5				
11. $T_t = \_\_L$ Compute $T_t =$	(hr.)	1.56		= [	1.56	(hrs.)
3600 V	60 . 10 <b>-</b>					
Channel Flow (SCS Method):						
S	egment ID:					
12. Cross Sectional Flow Area, a	145.750		*Assumed			
13. Wetted Perimeter, Pw	(ft.)	44.785		10' wide channel		
14. Hydraulic Radius, r = a/P <sub>w</sub>	3.254		with 1:3 sideslopes			
15. Channel Slope, s	(ft./ft.)	0.002				
16. Mannings Roughness Coefficient, n		0.035				
17. V=1.49 $r^{2/3} s^{1/2}/n$ Compute V	(ft/s)	4.4				
18. Flow Length, L	(ft.)	15860				
19. $T_t = \_\_L$ Compute $T_t =$	(hr.)	0.99		= [	0.99	(hrs.)
3600 V	-	DO T-4-1 W-4-		r	0.00	(has )
		20. 10tal water	sned $I_t$ or $I_c$	-	219.8	(nrs.) (min.)

Lag Time

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# Work Map Section Location Plan

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# Site Plan Section Location Plan





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# Site Plan Non-Effective Flow Areas





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# FEMA Data From Effective Flood Insurance Study

### SUMMARY OF DISCHARGES - EFFECTIVE FIS DATA

	DRAINAGE	DRAINAGE							
FLOODING SOURCE	AREA	PEAK DISCHARGES (cfs)							
AND LOCATION	(sq. miles)	10-PERCENT	2-PERCENT	<b>1-PERCENT</b>	0.2-PERCENT				
BROWN BROOK									
Upstream of Muscoot									
Reservoir	3.58	464	736	888	1,469				
Upstream of Somertown		2222	122420	1202	2 222				
Road	3.07	410	657	796	1,314				
Upstream of Mill									
Street/Old Somers Road	2.16	330	525	632	1,016				
Upstream of Unnamed									
Tributary near Hachaliah									
Brown Drive and Warren	-	1. 1971 - D. L.V.		1111.111.00					
Street	1.69	263	419	504	849				
Upstream of Green Brier									
Drive	1.18	182	298	362	652				
Upstream of Warren Street	12 12 13		0.23						
(first crossing)	1.01	162	259	312	581				
BYRAM RIVER REACH 1									
At mouth	30.4	2,719	4,442	5,439	7,259				
Upstream Metro North									
Railroad bridge	29.0	2,567	4,222	5,180	6,796				
	An and a second s	THE R - REAL PROPERTY OF							
BYRAM RIVER REACH 2			121120-2121	111 - 142 - 14 H					
At county boundary	8.39	1,171	2,164	2,576	*				
Upstream of confluence of									
Wampus River	4.59	436	796	996	•				
Upstream of State Route 22	4.10	400	745	927	*				
Approximately 900 feet									
downstream of Tributary 1									
to Byram River	3.73	376	708	886	*				
Upstream of confluence of									
Tributary 1 to Byram River	2.87	281	521	656	*				
Upstream of Byram Lake	2.22	1000	222	10.00	525				
Road	0.28	114	224	289	*				
	Sublimite Sublime over	and a second			and the second second				
CANEY BROOK									
Upstream of confluence		220	100		-				
with Pocantico River	1.4	328	482	552	706				
Upstream of Leroy Road	1.0	234	342	390	494				
CLOVE DDOCK									
CLOVE BROOK									
At downstream limit of		1.00		0.15					
Study	1.3	460	760	945	1,450				

#### TABLE 4 - SUMMARY OF DISCHARGES - continued

\*Data not available

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FLOOD PROFILE - EFFECTIVE FIS DATA



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# Storage vs. Discharge vs. Elevation Calculations
#### SUMMARY

#### Discharge Versus Flood Storage Volume on Byram River Reach 2 for Existing and Post Project Conditions

Discharge (CFS)	Existing Storage Volume (Acre-Feet)	Post-Project no Comp Storage Storage Volume (Acre-Feet)
1200	83.3	77.9
996	71.15	66.15
796	60.69	56.29
436	37.6	35
100	13.16	12.98
0	0	0

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#### Summary of Existing and Post Project Conditions Cummulative Flood Storage Volumes within Study Reach (Acre-Feet)

1	2	3	4		
Discharge (CFS)	Existing Storage Volume	Volume Reduction from Post Project Construction	Post-Project no Comp Storage Storage Volume		
1200	83.3	(-)5.40	77.9		
996	71.15	(-)5.00	66.15		
796	60.69	(-)4.40	56.29		
436	37.6	(-)2.60	35		
100	13.16	(-)0.18	12.98		
0	0	(-)0.00	0		

Notes:

Columns 1&2: Obtained from multiple discharge HEC-RAS series for Existing Conditions Column 3: Derived from JMC OnSite calculations for Existing and Post Project Conditions Existing Discharge versus Cummulative Volume(Acre-Feet) Between Sections 6350 to 2600 within Study Reach From Multiple HEC-RAS Runs for Existing Conditions

Discharge (CFS)	Cummulative Volume Section 6350 (Acre-Feet)	Cummulative Volume Section 2600 (Acre-Feet)	Net Volume		
1200	233.73	150.43	83.3		
996	203.84	132.69	71.15		
796	179	118.31	60.69		
436	116.91	79.31	37.6		
100	46.08	32.92	13.16		

#### Elevation versus Storage JMC-Data Measurement

Elevation	Existing Floodplain Storage Volume (CY)	Proposed Floodplain Storage Volume with Compensating Storage Basins	Proposed Floodplain Storage Volume without Compensating Storage Basins		
370	11582.77	10512.37	4977.53		
369.5	8116.74	8489.28	3256.84		
369	5111.48	6754.4	1861.11		
368.5	2555.97	5301.44	812.3		
368	634.13	4146.15	216.06		
367.5	95.08	3379.45	54.29		
367	8.07	2764.74	6.16		
366.5	0.53	2218.86	0		
366	0	1708.97	0		
365.5	0	1232.89	0		
365	0	789.8	0		
364.5	0	379.06	0		
364	0	0	0		

Existing and Post Project OnSite Flood Storage
Volume Vs Elevation (JMC Data)

Elevation	Exi	isting Volume	Post-Project no Comp Storage				
	(CY)	(ACRE-FT)	(CY) ACRE-FT)				
370	11582.77	7.18	4977.53	3.09			
369.5	8116.74	5.03	3256.84	2.02			
369	5111.48	3.17	1861.11	1.15			
368.5	2555.97	1.58	812.3	0.50			
368	634.13	0.39	216.06	0.13			
367.5	95.08	0.06	54.29	0.03			
367	8.07	0.01	6.16	0.00			
366.5	0.53	0.00	0	0.00			
366	0	0.00	0	0.00			

#### Elevation versus Volume Reduction At Site (Section 4500) (Acre-Feet)

Elevation	Existing Volume	Post-Project no Comp Storage Volume	∆ Volume Reduction		
370	7.18	3.09	4.09		
369.5	5.03	2.02	3.01		
369	3.17	1.15	2.01		
368.5	1.58	0.50	1.08		
368	0.39	0.13	0.26		
367.5	0.06	0.03	0.03		
367	0.01	0.00	0.00		
366.5	0.00	0.00	0.00		
366	0.00	0.00	0.00		
364	0	0	0.00		



CURVE 1 ELEVATION VERSUS VOLUME REDUCTION AT SITE

#### Post-Project Elevation versus Discharge versus Volume Reduction At Site (Section 4500)

1	2	3
Discharge	Elevation	Volume Reduction
1200	370.69	5.40
996	370.49	5.00
796	370.18	4.40
436	369.3	2.60
100	367.84	0.18
50	367.42	Neligible
40	367.3	Neligible
10	366.72	Neligible

Notes:

a) Columns 1&2 from post-project HEC-RAS multiple runs at section 4500 (On Site)

b) Column 3 from Curve 1: "Elevation versus Volume Reduction at site"

#### Discharge versus Flood Storage Volume On Study Reach (Acre-Feet)

Discharge	Existing Volume	Volume Reduction from Post-Project Construction	Post-Project no Comp Storage Volume
1200	83.30	5.40	77.90
996	71.15	5.00	66.15
796	60.69	4.40	56.29
436	37.60	2.60	35.00
100	13.16	0.18	12.98
0	0.00	0.00	0.00



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# HEC-RAS Existing Conditions Multiple Run Series

Leonard Jackson PE PLLC dba Leonard Jackson Associates

## Multiple Run Series Existing 100, 270 CFS

HEC-RAS Plan: EXISTING River: River 1 Reach: Reach 1

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Reach	River Sta	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	100.00	368.00	369.83		369.88	0.001240	1.91	52.32	39.52	46.08
Reach 1	6283	100.00	368.00	369.41		369.50	0.002799	2.37	42.27	38.87	45.85
Reach 1	5968	100.00	366.49	368.81		368.88	0.001449	2.05	48.74	33.66	45.52
Reach 1	5656	100.00	366.00	368.32		368.39	0.001684	2.18	45.87	32.36	45.18
Reach 1	5380	100.00	366.00	368.27		368.28	0.000135	0.75	132.70	69.78	44.61
Reach 1	5156	100.00	366.00	368.24		368.25	0.000160	0.78	128.85	75.91	43.92
Reach 1	5064	100.00	366.00	368.18		368.22	0.000413	1.29	67.14	41.25	43.73
Reach 1	4996	100.00	366.00	368.14		368.18	0.000771	1.65	60.56	37.96	43.63
Reach 1	4959	100.00	366.00	368.09		368.15	0.001344	1.82	54.90	42.87	43.58
Reach 1	4788	100.00	365.18	368.02		368.03	0.000284	1.05	95.54	55.72	43.25
Reach 1	4731	100.00	365.29	367.98		368.02	0.000280	1.08	81.08	45.99	43.14
Reach 1	4699	100.00	366.00	367.96		368.00	0.000657	1.52	64.39	42.06	43.08
Reach 1	4500	100.00	366.00	367.90		367.93	0.000516	1.34	74.09	54.34	42.89
Reach 1	4441	100.00	365.53	367.84		367.87	0.000374	1.26	74.50	52.43	42.66
Reach 1	4393	100.00	366.00	367.80		367.84	0.000815	1.59	62.80	69.68	42.58
Reach 1	4268	100.00	366.00	367.67		367.72	0.001101	1.79	56.05	43.93	42.41
Reach 1	4080	100.00	366.00	366.80	366.80	367.14	0.020803	4.66	21.46	32.21	42.24
Reach 1	3575	100.00	364.00	365.90		365.92	0.000682	1.15	87.14	86.09	41.61
Reach 1	3290	100.00	364.00	365.41		365.51	0.004081	2.62	38.21	40.15	41.20
Reach 1	2949	100.00	364.00	365.48	364.12	365.48	0.000001	0.05	624.02	444.20	38.61
Reach 1	2600	270.00	362.00	365.48		365.48	0.000006	0.21	819.54	519.24	32.92
Reach 1	2383	270.00	364.00	365.47	364.21	365.48	0.000041	0.34	802.34	586.57	28.78
Reach 1	2079	270.00	364.00	365.46	364.21	365.46	0.000043	0.34	783.48	572.20	23.24
Reach 1	1851	270.00	363.65	365.45	364.23	365.45	0.000074	0.45	597.92	437.49	19.63
Reach 1	1418	270.00	362.13	365.40		365.41	0.000141	0.68	399.57	257.93	14.68
Reach 1	1121	270.00	363.13	365.33		365.35	0.000288	0.77	302.75	280.19	12.28
Reach 1	661	270.00	362.00	365.31		365.31	0.000026	0.40	680.61	286.81	7.09
Reach 1	425	270.00	362.00	365.31		365.31	0.000020	0.38	715.99	262.57	3.31
Reach 1	50	270.00	364.00	364.85	364.85	365.24	0.019694	5.03	53.72	69.23	

### Multiple Run Series Existing 436, 1171 CFS (10YR)

HEC-RAS Plan: EXISTING River: River 1 Reach: Reach 1 Profile: 10 YR

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
	-		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	10 YR	436.00	368.00	371.55		371.72	0.001141	3.09	133.93	55.32	116.91
Reach 1	6283	10 YR	436.00	368.00	371.21		371.39	0.002263	3.36	129.80	59.95	116.26
Reach 1	5968	10 YR	436.00	366.49	370.63		370.79	0.001585	3.32	134.76	77.57	115.30
Reach 1	5656	10 YR	436.00	366.00	369.89		370.14	0.002901	3.95	110.49	49.79	114.43
Reach 1	5380	10 YR	436.00	366.00	369.80		369.84	0.000390	1.76	247.26	79.96	113.28
Reach 1	5156	10 YR	436.00	366.00	369.72		369.76	0.000316	1.62	263.38	100.11	111.95
Reach 1	5064	10 YR	436.00	366.00	369.42		369.69	0.001174	2.83	123.69	50.25	111.57
Reach 1	4996	10 YR	436.00	366.00	369.33		369.58	0.002280	3.91	109.68	44.71	111.39
Reach 1	4959	10 YR	436.00	366.00	369.22		369.48	0.003116	4.06	107.74	50.39	111.29
Reach 1	4788	10 YR	436.00	365.18	369.14		369.21	0.000566	2.03	209.27	126.21	110.60
Reach 1	4731	10 YR	436.00	365.29	369.13		369.18	0.000313	1.50	270.14	225.12	110.29
Reach 1	4699	10 YR	436.00	366.00	369.01		369.15	0.001400	2.97	220.51	208.16	110.10
Reach 1	4500	10 YR	436.00	366.00	369.04		369.06	0.000226	1.26	346.88	341.68	109.30
Reach 1	4441	10 YR	436.00	365.53	369.02		369.04	0.000101	0.90	446.64	389.50	108.07
Reach 1	4393	10 YR	436.00	366.00	369.02		369.03	0.000060	0.64	511.42	447.67	107.54
Reach 1	4268	10 YR	436.00	366.00	368.94	367.86	369.01	0.000634	2.07	201.26	179.82	106.52
Reach 1	4080	10 YR	436.00	366.00	367.91	367.91	368.61	0.016242	6.71	65.02	46.83	105.94
Reach 1	3575	10 YR	436.00	364.00	367.44		367.47	0.000180	1.07	328.69	173.24	103.66
Reach 1	3290	10 YR	436.00	364.00	367.38		367.42	0.000190	1.11	289.35	170.56	101.64
Reach 1	2949	10 YR	436.00	364.00	367.40	364.33	367.41	0.000001	0.09	1598.49	660.59	93.89
Reach 1	2600	10 YR	1171.00	362.00	367.40		367.40	0.000007	0.32	2011.14	692.91	79.31
Reach 1	2383	10 YR	1171.00	364.00	367.40	364.55	367.40	0.000040	0.58	2005.36	809.48	68.53
Reach 1	2079	10 YR	1171.00	364.00	367.38	364.55	367.39	0.000042	0.60	1929.74	751.04	53.46
Reach 1	1851	10 YR	1171.00	363.65	367.36	364.65	367.38	0.000072	0.80	1457.19	508.35	44.06
Reach 1	1418	10 YR	1171.00	362.13	367.30		367.33	0.000172	1.22	957.16	305.80	31.90
Reach 1	1121	10 YR	1171.00	363.13	367.26		367.28	0.000150	0.99	1046.44	471.71	25.07
Reach 1	661	10 YR	1171.00	362.00	367.22		367.23	0.000066	0.92	1251.53	310.35	12.93
Reach 1	425	10 YR	1171.00	362.00	367.20		367.22	0.000061	0.92	1268.80	322.73	6.11
Reach 1	50	10 YR	1171.00	364.00	366.13	366.13	367.05	0.014900	7.68	152.46	84.16	

### Multiple Run Series Existing 796, 2164 CFS (50 YR)

Reach	River Sta	er Sta Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
		1. Dec	(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	50 YR	796.00	368.00	372.44		372.73	0.001339	3.95	187.17	66.12	179.00
Reach 1	6283	50 YR	796.00	368.00	372.05		372.34	0.002697	4.31	186.24	96.30	178.09
Reach 1	5968	50 YR	796.00	366.49	371.59		371.77	0.001174	3.35	233.82	130.29	176.56
Reach 1	5656	50 YR	796.00	366.00	370.90		371.23	0.002570	4.71	172.40	77.75	175.12
Reach 1	5380	50 YR	796.00	366.00	370.79		370.88	0.000535	2.39	366.04	255.14	173.39
Reach 1	5156	50 YR	796.00	366.00	370.70		370.77	0.000355	2.06	365.54	109.14	171.48
Reach 1	5064	50 YR	796.00	366.00	370.09		370.67	0.001851	3.85	162.82	94.08	170.96
Reach 1	4996	50 YR	796.00	366.00	369.96		370.48	0.003615	5.57	139.20	48.86	170.73
Reach 1	4959	50 YR	796.00	366.00	369.82		370.33	0.004604	5.76	138.70	54.29	170.61
Reach 1	4788	50 YR	796.00	365.18	369.86		369.97	0.000572	2.37	312.80	166.28	169.62
Reach 1	4731	50 YR	796.00	365.29	369.88		369.93	0.000256	1.58	469.87	308.14	169.10
Reach 1	4699	50 YR	796.00	366.00	369.79		369.91	0.000996	2.98	416.41	297.50	168.77
Reach 1	4500	50 YR	796.00	366.00	369.82		369.85	0.000130	1.13	661.69	462.63	167.25
Reach 1	4441	50 YR	796.00	365.53	369.82		369.83	0.000063	0.83	781.00	519.71	165.02
Reach 1	4393	50 YR	796.00	366.00	369.82		369.83	0.000034	0.57	894.61	496.31	164.08
Reach 1	4268	50 YR	796.00	366.00	369.73		369.81	0.000344	1.81	364.16	230.76	162.28
Reach 1	4080	50 YR	796.00	366.00	368.70	368.70	369.55	0.011905	7.41	108.49	64.84	161.26
Reach 1	3575	50 YR	796.00	364.00	368.77		368.81	0.000090	1.00	569.59	187.70	157.33
Reach 1	3290	50 YR	796.00	364.00	368.74		368.79	0.000079	0.94	531.80	185.00	153.72
Reach 1	2949	50 YR	796.00	364.00	368.77	364.49	368.77	0.000001	0.12	2386.47	799.94	141.06
Reach 1	2600	50 YR	2164.00	362.00	368.76		368.77	0.000008	0.40	3067.00	844.90	118.32
Reach 1	2383	50 YR	2164.00	364.00	368.76	364.82	368.77	0.000040	0.74	2918.89	880.06	101.88
Reach 1	2079	50 YR	2164.00	364.00	368.74	364.82	368.75	0.000043	0.77	2782.48	807.72	79.04
Reach 1	1851	50 YR	2164.00	363.65	368.72	364.98	368.74	0.000075	1.02	2094.00	627.42	64.84
Reach 1	1418	50 YR	2164.00	362.13	368.65		368.69	0.000177	1.56	1381.49	329.84	46.76
Reach 1	1121	50 YR	2164.00	363.13	368.62		368.65	0.000097	1.05	1771.17	585.04	36.02
Reach 1	661	50 YR	2164.00	362.00	368.58		368.61	0.000085	1.25	1683.82	324.94	17.76
Reach 1	425	50 YR	2164.00	362.00	368.56		368.59	0.000075	1.22	1738.10	363.62	8.51
Reach 1	50	50 YR	2164.00	364.00	367.10	367.10	368.37	0.013423	9.02	240.04	96.24	

### Multiple Run Series Existing 996, 2576 CFS (100 YR)

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
6			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	100 YR	996.00	368.00	372.77		373.12	0.001506	4.41	212.20	99.02	203.80
Reach 1	6283	100 YR	996.00	368.00	372.36		372.69	0.002743	4.67	221.10	128.22	202.73
Reach 1	5968	100 YR	996.00	366.49	371.95		372.14	0.001043	3.37	285.31	154.20	200.90
Reach 1	5656	100 YR	996.00	366.00	371.28		371.65	0.002449	4.95	205.02	91.15	199.15
Reach 1	5380	100 YR	996.00	366.00	371.18		371.28	0.000567	2.59	482.35	328.55	196.95
Reach 1	5156	100 YR	996.00	366.00	371.07		371.17	0.000387	2.28	406.97	111.57	194.62
Reach 1	5064	100 YR	996.00	366.00	370.40		371.06	0.001946	4.13	194.19	104.89	194.03
Reach 1	4996	100 YR	996.00	366.00	370.24		370.87	0.004000	6.18	162.37	100.05	193.76
Reach 1	4959	100 YR	996.00	366.00	370.13	369.39	370.69	0.004505	6.11	174.23	170.11	193.61
Reach 1	4788	100 YR	996.00	365.18	370.23		370.32	0.000540	2.46	404.99	314.67	192.34
Reach 1	4731	100 YR	996.00	365.29	370.24		370.29	0.000240	1.63	645.09	578.36	191.65
Reach 1	4699	100 YR	996.00	366.00	370.15		370.27	0.000885	3.01	568.13	568.69	191.20
Reach 1	4500	100 YR	996.00	366.00	370.18		370.21	0.000124	1.17	848.72	560.14	189.19
Reach 1	4441	100 YR	996.00	365.53	370.18		370.19	0.000065	0.89	978.76	571.05	186.37
Reach 1	4393	100 YR	996.00	366.00	370.18		370.19	0.000029	0.56	1074.43	503.60	185.23
Reach 1	4268	100 YR	996.00	366.00	370.10		370.18	0.000305	1.82	469.58	420.69	183.02
Reach 1	4080	100 YR	996.00	366.00	369.05	369.05	369.93	0.010155	7.59	132.65	75.49	181.71
Reach 1	3575	100 YR	996.00	364.00	369.23		369.28	0.000087	1.06	656.92	191.89	177.14
Reach 1	3290	100 YR	996.00	364.00	369.20		369.26	0.000074	0.99	618.41	190.27	172.96
Reach 1	2949	100 YR	996.00	364.00	369.24	364.58	369.24	0.000002	0.14	2666.76	823.63	158.48
Reach 1	2600	100 YR	2576.00	362.00	369.23		369.24	0.000007	0.41	3466.62	870.63	132.67
Reach 1	2383	100 YR	2576.00	364.00	369.22	364.92	369.23	0.000040	0.79	3237.21	889.84	114.16
Reach 1	2079	100 YR	2576.00	364.00	369.21	364.93	369.22	0.000043	0.82	3079.30	814.90	88.56
Reach 1	1851	100 YR	2576.00	363.65	369.19	365.10	369.21	0.000075	1.09	2323.09	663.16	72.59
Reach 1	1418	100 YR	2576.00	362.13	369.11		369.16	0.000179	1.67	1536.60	344.77	52.25
Reach 1	1121	100 YR	2576.00	363.13	369.09		369.12	0.000083	1.05	2047.17	601.37	40.04
Reach 1	661	100 YR	2576.00	362.00	369.05		369.08	0.000090	1.36	1835.97	330.46	19.53
Reach 1	425	100 YR	2576.00	362.00	369.03		369.06	0.000079	1.31	1909.70	375.64	9.39
Reach 1	50	100 YR	2576.00	364.00	367.46	367.46	368.82	0.012893	9.36	275.18	101.14	

HEC-RAS Plan: EXISTING River: River 1 Reach: Reach 1 Profile: 100 YR

### Multiple Run Series Existing 1200, 3120 CFS

HEC-RAS Plan: EXISTING River: River 1 Reach: Reach 1

Reach	River Sta	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	1200.00	368.00	373.05		373.43	0.001628	4.78	245.52	137.70	233.74
Reach 1	6283	1200.00	368.00	372.64		372.98	0.002608	4.82	260.68	154.80	232.49
Reach 1	5968	1200.00	366.49	372.30		372.46	0.000950	3.41	370.35	264.46	230.21
Reach 1	5656	1200.00	366.00	371.58		371.99	0.002416	5.18	233.30	98.88	228.06
Reach 1	5380	1200.00	366.00	371.50		371.60	0.000586	2.74	590.76	350.76	225.41
Reach 1	5156	1200.00	366.00	371.36	· · · · · · · · · · ·	371.49	0.000434	2.51	439.09	113.43	222.72
Reach 1	5064	1200.00	366.00	370.68		371.36	0.001932	4.34	223.93	110.08	222.07
Reach 1	4996	1200.00	366.00	370.34	369.73	371.16	0.005036	7.06	172.84	109.08	221.76
Reach 1	4959	1200.00	366.00	370.44	369.72	370.89	0.003419	5.66	231.43	198.59	221.59
Reach 1	4788	1200.00	365.18	370.51		370.60	0.000432	2.30	496.22	327.64	219.99
Reach 1	4731	1200.00	365.29	370.53		370.57	0.000193	1.53	814.07	580.73	219.14
Reach 1	4699	1200.00	366.00	370.49		370.56	0.000578	2.58	762.58	575.33	218.54
Reach 1	4500	1200.00	366.00	370.50		370.52	0.000096	1.09	1026.29	568.72	216.01
Reach 1	4441	1200.00	365.53	370.49		370.51	0.000054	0.86	1159.86	574.83	212.64
Reach 1	4393	1200.00	366.00	370.49		370.51	0.000027	0.57	1235.24	515.31	211.30
Reach 1	4268	1200.00	366.00	370.44		370.50	0.000267	1.80	618.95	459.27	208.65
Reach 1	4080	1200.00	366.00	369.33	369.33	370.26	0.009173	7.78	155.20	84.02	206.97
Reach 1	3575	1200.00	364.00	369.78		369.83	0.000077	1.08	764.69	198.68	201.64
Reach 1	3290	1200.00	364.00	369.76		369.81	0.000063	0.98	725.78	196.84	196.76
Reach 1	2949	1200.00	364.00	369.79	364.65	369.80	0.000002	0.15	3004.38	851.94	180.03
Reach 1	2600	3120.00	362.00	369.78		369.79	0.000007	0.43	3959.20	900.12	150.44
Reach 1	2383	3120.00	364.00	369.78	365.04	369.79	0.000040	0.85	3619.53	902.36	129.40
Reach 1	2079	3120.00	364.00	369.76	365.04	369.78	0.000044	0.89	3434.95	823.46	100.48
Reach 1	1851	3120.00	363.65	369.74	365.24	369.76	0.000076	1.18	2623.17	760.61	82.28
Reach 1	1418	3120.00	362.13	369.66		369.71	0.000179	1.79	1731.71	365.24	59.01
Reach 1	1121	3120.00	363.13	369.64		369.68	0.000073	1.06	2386.38	621.68	44.98
Reach 1	661	3120.00	362.00	369.60		369.64	0.000097	1.49	2020.35	338.05	21.70
Reach 1	425	3120.00	362.00	369.58		369.61	0.000082	1.41	2120.48	389.35	10.49
Reach 1	50	3120.00	364.00	367.88	367.88	369.36	0.012670	9.76	319.69	110.51	

SI



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# HEC-RAS Post Project Conditions Multiple Run Series No Compensating Flood Storage

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### Multiple Run. Jeries

### Post-Project No Compensating Storage 100, 270 CFS

HEC-RAS Plan: Post - No Comp Storage River: River 1 Reach: Reach 1

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Reach	River Sta	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	100.00	368.00	369.83		369.88	0.001236	1.91	52.38	39.53	46.11
Reach 1	6283	100.00	368.00	369.41		369.50	0.002816	2.37	42.18	38.84	45.87
Reach 1	5968	100.00	366.49	368.80		368.87	0.001473	2.06	48.45	33.58	45.55
Reach 1	5656	100.00	366.00	368.28		368.36	0.001800	2.23	44.78	32.03	45.21
Reach 1	5380	100.00	366.00	368.23		368.24	- 0.000143	0.77	130.19	69.53	44.66
Reach 1	5156	100.00	366.00	368.20		368.21	0.000171	0.79	126.01	74.93	44.00
Reach 1	5064	100.00	366.00	368.13		368.18	0.000450	1.33	65.38	40.95	43.80
Reach 1	4996	100.00	366.00	368.09		368.14	0.000845	1.70	58.77	37.61	43.70
Reach 1	4959	100.00	366.00	368.04		368.10	0.001526	1.90	52.65	42.48	43.65
Reach 1	4788	100.00	365.18	367.97		367.99	0.000309	1.08	92.93	52.98	43.37
Reach 1	4731	100.00	365.29	367.93		367.97	0.000308	1.11	78.72	45.72	43.26
Reach 1	4699	100.00	366.00	367.91		367.95	0.000733	1.58	62.16	41.63	43.20
Reach 1	4500	100.00	366.00	367.84		367.87	0.000599	1.41	70.69	53.22	43.02
Reach 1	4441	100.00	365.53	367.82		367.83	0.000121	0.71	123.53	182.34	42.72
Reach 1	4393	100.00	366.00	367.79		367.82	0.000523	1.27	77.38	108.72	42.60
Reach 1	4268	100.00	366.00	367.67		367.72	0.001101	1.79	56.05	43.93	42.41
Reach 1	4080	100.00	366.00	366.80	366.80	367.14	0.020803	4.66	21.46	32.21	42.25
Reach 1	3575	100.00	364.00	365.90		365.92	0.000682	1.15	87.14	86.09	41.62
Reach 1	3290	100.00	364.00	365.41		365.51	0.004080	2.62	38.21	40.16	41.21
Reach 1	2949	100.00	364.00	365.48	364.12	365.48	0.000001	0.05	624.03	444.20	38.61
Reach 1	2600	270.00	362.00	365.48		365.48	0.000006	0.21	819.55	519.25	32.94
Reach 1	2383	270.00	364.00	365.47	364.21	365.48	0.000041	0.34	802.34	586.57	28.78
Reach 1	2079	270.00	364.00	365.46	364.21	365.46	0.000043	0.34	783.48	572.20	23.24
Reach 1	1851	270.00	363.65	365.45	364.23	365.45	0.000074	0.45	597.92	437.49	19.63
Reach 1	1418	270.00	362.13	365.40		365.41	0.000141	0.68	399.57	257.93	14.68
Reach 1	1121	270.00	363.13	365.33		365.35	0.000288	0.77	302.75	280.19	12.28
Reach 1	661	270.00	362.00	365.31		365.31	0.000026	0.40	680.61	286.81	7.09
Reach 1	425	270.00	362.00	365.31		365.31	0.000020	0.38	715.99	262.57	3.31
Reach 1	50	270.00	364.00	364.85	364.85	365.24	0.019694	5.03	53.72	69.23	

#### Multiple Run Project Post-Project No Compensating Storage 436, 1171 CFS (10 YR)

HEC-RAS I	Plan: Post - No	Comp Storag	e River: River	1 Reach: Re	each 1 Profile	e: 10 YR						
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	10 YR	436.00	368.00	371.56		371.73	0.001134	3.08	134.20	55.37	116.34
Reach 1	6283	10 YR	436.00	368.00	371.22		371.40	0.002238	3.34	130.35	60.08	115.69
Reach 1	5968	10 YR	436.00	366.49	370.66		370.82	0.001510	3.26	137.28	79.24	114.72
Reach 1	5656	10 YR	436.00	366.00	369.99		370.21	0.002569	3.80	115.11	51.08	113.82
Reach 1	5380	10 YR	436.00	366.00	369.90		369.95	0.000352	1.70	255.89	80.61	112.64
Reach 1	5156	10 YR	436.00	366.00	369.84		369.88	0.000273	1.55	275.28	101.22	111.28
Reach 1	5064	10 YR	436.00	366.00	369.57		369.81	0.000982	2.65	131.30	51.46	110.85
Reach 1	4996	10 YR	436.00	366.00	369.50		369.72	0.001850	3.65	117.28	45.67	110.65
Reach 1	4959	10 YR	436.00	366.00	369.43		369.64	0.002316	3.71	118.10	51.61	110.55
Reach 1	4788	10 YR	436.00	365.18	369.42		369.47	0.000344	1.68	245.64	137.51	109.84
Reach 1	4731	10 YR	436.00	365.29	369.38		369.45	0.000318	1.60	227.44	135.53	109.53
Reach 1	4699	10 YR	436.00	366.00	369.25		369.42	0.001286	3.02	141.24	93.90	109.39
Reach 1	4500	10 YR	436.00	366.00	369.30		369.33	0.000186	1.21	303.06	222.34	108.77
Reach 1	4441	10 YR	436.00	365.53	369.30		369.31	0.000042	0.61	476.52	252.03	107.56
Reach 1	4393	10 YR	436.00	366.00	369.28		369.31	0.000100	0.88	361.49	239.94	107.10
Reach 1	4268	10 YR	436.00	366.00	369.10		369.26	0.001311	3.09	135.58	69.84	106.38
Reach 1	4080	10 YR	436.00	366.00	367.91	367.91	368.61	0.016242	6.71	65.02	46.83	105.95
Reach 1	3575	10 YR	436.00	364.00	367.44		367.47	0.000180	1.07	328.69	173.24	103.67
Reach 1	3290	10 YR	436.00	364.00	367.38		367.42	0.000190	1.11	289.35	170.56	101.65
Reach 1	2949	10 YR	436.00	364.00	367.40	364.33	367.41	0.000001	0.09	1598.51	660.60	93.90
Reach 1	2600	10 YR	1171.00	362.00	367.40		367.40	0.000007	0.32	2011.16	692.91	79.37
Reach 1	2383	10 YR	1171.00	364.00	367.40	364.55	367.40	0.000040	0.58	2005.36	809.48	68.53
Reach 1	2079	10 YR	1171.00	364.00	367.38	364.55	367.39	0.000042	0.60	1929.74	751.04	53.46
Reach 1	1851	10 YR	1171.00	363.65	367.36	364.65	367.38	0.000072	0.80	1457.19	508.35	44.06
Reach 1	1418	10 YR	1171.00	362.13	367.30		367.33	0.000172	1.22	957.16	305.80	31.90
Reach 1	1121	10 YR	1171.00	363.13	367.26		367.28	0.000150	0.99	1046.44	471.71	25.07
Reach 1	661	10 YR	1171.00	362.00	367.22		367.23	0.000066	0.92	1251.53	310.35	12.93
Reach 1	425	10 YR	1171.00	362.00	367.20		367.22	0.000061	0.92	1268.80	322.73	6.11
Reach 1	50	10 YR	1171.00	364.00	366.13	366.13	367.05	0.014900	7.68	152.46	84.16	

## **Multiple Run Series** Post-Project No Compensating Storage 796, 2164 CFS (50 YR)

HEC-RAS	Plan: Post - No	Comp Storage	e River: Rive	r 1 Reach: Re	each 1 Profile	e: 50 YR						
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	50 YR	796.00	368.00	372.44		372.74	0.001334	3.94	187.42	66.19	177.65
Reach 1	6283	50 YR	796.00	368.00	372.06		372.34	0.002671	4.30	186.95	97.09	176.73
Reach 1	5968	50 YR	796.00	366.49	371.61		371.79	0.001122	3.30	237.44	131.92	175.20
Reach 1	5656	50 YR	796.00	366.00	370.97		371.29	0.002332	4.56	178.44	80.67	173.71
Reach 1	5380	50 YR	796.00	366.00	370.88		370.96	0.000490	2.31	391.44	281.74	171.91
Reach 1	5156	50 YR	796.00	366.00	370.80		370.87	0.000321	1.99	376.71	109.80	169.93
Reach 1	5064	50 YR	796.00	366.00	370.32		370.78	0.001390	3.44	185.72	103.01	169.34
Reach 1	4996	50 YR	796.00	366.00	370.23		370.64	0.002567	4.95	162.04	99.88	169.07
Reach 1	4959	50 YR	796.00	366.00	370.21		370.52	0.002490	4.61	187.06	178.77	168.92
Reach 1	4788	50 YR	796.00	365.18	370.26		370.32	0.000318	1.90	416.91	316.17	167.73
Reach 1	4731	50 YR	796.00	365.29	370.23		370.30	0.000294	1.80	392.02	315.32	167.20
Reach 1	4699	50 YR	796.00	366.00	370.15		370.28	0.000790	2.84	292.60	291.96	166.95
Reach 1	4500	50 YR	796.00	366.00	370.18		370.22	0.000119	1.15	538.47	349.21	165.78
Reach 1	4441	50 YR	796.00	365.53	370.19		370.21	0.000039	0.69	712.31	305.36	163.85
Reach 1	4393	50 YR	796.00	366.00	370.17		370.20	0.000066	0.85	586.11	274.50	163.12
Reach 1	4268	50 YR	796.00	366.00	369.94		370.16	0.001304	3.66	215.96	141.50	161.97
Reach 1	4080	50 YR	796.00	366.00	368.70	368.70	369.55	0.011905	7.41	108.49	64.84	161.27
Reach 1	3575	50 YR	796.00	364.00	368.77		368.81	0.000090	1.00	569.59	187.70	157.34
Reach 1	3290	50 YR	796.00	364.00	368.74		368.79	0.000079	0.94	531.81	185.00	153.74
Reach 1	2949	50 YR	796.00	364.00	368.77	364.49	368.77	0.000001	0.12	2386.49	799.95	141.07
Reach 1	2600	50 YR	2164.00	362.00	368.76		368.77	0.000008	0.40	3067.03	844.90	118.41
Reach 1	2383	50 YR	2164.00	364.00	368.76	364.82	368.77	0.000040	0.74	2918.91	880.06	101.88
Reach 1	2079	50 YR	2164.00	364.00	368.74	364.82	368.75	0.000043	0.77	2782.50	807.72	79.04
Reach 1	1851	50 YR	2164.00	363.65	368.72	364.98	368.74	0.000075	1.02	2094.02	627.42	64.84
Reach 1	1418	50 YR	2164.00	362.13	368.65		368.69	0.000177	1.56	1381.50	329.84	46.76
Reach 1	1121	50 YR	2164.00	363.13	368.62		368.65	0.000097	1.05	1771.19	585.04	36.02
Reach 1	661	50 YR	2164.00	362.00	368.58		368.61	0.000085	1.25	1683.84	324.94	17.76
Reach 1	425	50 YR	2164.00	362.00	368.56		368.59	0.000075	1.22	1738.11	363.62	8.51
Reach 1	50	50 YR	2164.00	364.00	367.10	367.10	368.37	0.013424	9.02	240.04	96.24	

#### Multiple . ....n Series Post-Project No Compensating Storage 996, 2576 CFS (100 YR)

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	100 YR	996.00	368.00	372.77		373.12	0.001505	4.40	212.25	99.09	201.57
Reach 1	6283	100 YR	996.00	368.00	372.36		372.69	0.002734	4.66	221.40	128.43	200.51
Reach 1	5968	100 YR	996.00	366.49	371.96		372.15	0.001030	3.36	286.68	154.98	198.67
Reach 1	5656	100 YR	996.00	366.00	371.30		371.66	0.002390	4.91	206.73	91.62	196.90
Reach 1	5380	100 YR	996.00	366.00	371.21		371.31	0.000551	2.56	491.54	331.32	194.69
Reach 1	5156	100 YR	996.00	366.00	371.10		371.20	0.000376	2.25	410.49	111.77	192.37
Reach 1	5064	100 YR	996.00	366.00	370.54		371.10	0.001604	3.86	209.09	107.59	191.72
Reach 1	4996	100 YR	996.00	366.00	370.43		370.94	0.002991	5.54	183.79	113.78	191.41
Reach 1	4959	100 YR	996.00	366.00	370.50		370.78	0.002074	4.46	243.05	203.23	191.23
Reach 1	4788	100 YR	996.00	365.18	370.55		370.61	0.000275	1.85	508.85	329.65	189.75
Reach 1	4731	100 YR	996.00	365.29	370.52		370.59	0.000254	1.76	486.14	322.52	189.10
Reach 1	4699	100 YR	996.00	366.00	370.47		370.58	0.000557	2.52	390.34	304.22	188.78
Reach 1	4500	100 YR	996.00	366.00	370.49		370.53	0.000109	1.16	647.73	360.00	187.31
Reach 1	4441	100 YR	996.00	365.53	370.49		370.52	0.000042	0.76	807.49	314.93	185.07
Reach 1	4393	100 YR	996.00	366.00	370.48		370.52	0.000068	0.90	674.58	298.62	184.24
Reach 1	4268	100 YR	996.00	366.00	370.29	368.97	370.48	0.001136	3.63	350.79	455.59	182.77
Reach 1	4080	100 YR	996.00	366.00	369.05	369.05	369.93	0.010155	7.59	132.65	75.49	181.73
Reach 1	3575	100 YR	996.00	364.00	369.23		369.28	0.000087	1.06	656.93	191.89	177.15
Reach 1	3290	100 YR	996.00	364.00	369.20		369.26	0.000074	0.99	618.42	190.27	172.98
Reach 1	2949	100 YR	996.00	364.00	369.24	364.58	369.24	0.000002	0.14	2666.80	823.63	158.49
Reach 1	2600	100 YR	2576.00	362.00	369.23		369.24	0.000007	0.41	3466.68	870.63	132.77
Reach 1	2383	100 YR	2576.00	364.00	369.22	364.92	369.23	0.000040	0.79	3237.23	889.84	114.16
Reach 1	2079	100 YR	2576.00	364.00	369.21	364.93	369.22	0.000043	0.82	3079.32	814.90	88.57
Reach 1	1851	100 YR	2576.00	363.65	369.19	365.10	369.21	0.000075	1.09	2323.11	663.16	72.59
Reach 1	1418	100 YR	2576.00	362.13	369.11		369.16	0.000179	1.67	1536.61	344.77	52.25
Reach 1	1121	100 YR	2576.00	. 363.13	369.09		369.12	0.000083	1.05	2047.19	601.37	40.05
Reach 1	661	100 YR	2576.00	362.00	369.05		369.08	0.000090	1.36	1835.98	330.46	19.53
Reach 1	425	100 YR	2576.00	362.00	369.03		369.06	0.000079	1.31	1909.71	375.64	9.39
Reach 1	50	100 YR	2576.00	364.00	367.46	367.46	368.82	0.012893	9.36	275.18	101.14	

#### Multiple Run Series Post-Project No Compensating Storage 1200, 3120 CFS

Reach	River Sta	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	1200.00	368.00	373.05		373.43	0.001630	4.78	245.38	137.54	230.06
Reach 1	6283	1200.00	368.00	372.64		372.98	0.002611	4.82	260.55	154.71	228.82
Reach 1	5968	1200.00	366.49	372.30		372.46	0.000946	3.40	370.88	264.52	226.54
Reach 1	5656	1200.00	366.00	371.58		371.99	0.002420	5.19	233.18	98.85	224.38
Reach 1	5380	1200.00	366.00	371.50		371.61	0.000583	2.73	592.30	351.03	221.76
Reach 1	5156	1200.00	366.00	371.36		371.49	0.000432	2.51	439.87	113.48	219.11
Reach 1	5064	1200.00	366.00	370.69		371.37	0.001900	4.32	225.30	110.31	218.40
Reach 1	4996	1200.00	366.00	370.55	369.73	371.18	0.003603	6.21	197.70	118.32	218.08
Reach 1	4959	1200.00	366.00	370.69		370.98	0.001980	4.51	283.91	219.41	217.87
Reach 1	4788	1200.00	365.18	370.74		370.81	0.000274	1.90	573.74	335.47	216.18
Reach 1	4731	1200.00	365.29	370.72		370.80	0.000256	1.82	549.92	326.43	215.45
Reach 1	4699	1200.00	366.00	370.67		370.78	0.000520	2.52	451.40	312.19	215.08
Reach 1	4500	1200.00	366.00	370.69		370.74	0.000116	1.23	719.51	367.03	213.43
Reach 1	4441	1200.00	365.53	370.69		370.72	0.000049	0.84	870.12	321.62	210.97
Reach 1	4393	1200.00	366.00	370.67		370.72	0.000076	0.99	733.31	306.72	210.08
Reach 1	4268	1200.00	366.00	370.56	369.28	370.69	0.000764	3.11	474.67	461.79	208.35
Reach 1	4080	1200.00	366.00	369.33	369.33	370.26	0.009173	7.78	155.20	84.02	206.99
Reach 1	3575	1200.00	364.00	369.78		369.83	0.000077	1.08	764.70	198.68	201.65
Reach 1	3290	1200.00	364.00	369.76		369.81	0.000063	0.98	725.80	196.84	196.78
Reach 1	2949	1200.00	364.00	369.79	364.65	369.80	0.000002	0.15	3004.43	851.95	180.05
Reach 1	2600	3120.00	362.00	369.78		369.79	0.000007	0.43	3959.28	900.12	150.56
Reach 1	2383	3120.00	364.00	369.78	365.04	369.79	0.000040	0.85	3619.59	902.37	129.40
Reach 1	2079	3120.00	364.00	369.76	365.04	369.78	0.000044	0.89	3435.03	823.46	100.48
Reach 1	1851	3120.00	363.65	369.74	365.24	369.76	0.000076	1.18	2623.24	760.63	82.28
Reach 1	1418	3120.00	362.13	369.66		369.71	0.000179	1.79	1731.76	365.25	59.01
Reach 1	1121	3120.00	363.13	369.64		369.68	0.000073	1.06	2386.47	621.68	44.98
Reach 1	661	3120.00	362.00	369.60		369.64	0.000097	1.49	2020.40	338.06	21.70
Reach 1	425	3120.00	362.00	369.58		369.61	0.000082	1.41	2120.54	389.35	10.49
Reach 1	50	3120.00	364.00	367.88	367.88	369.36	0.012688	9.77	319.53	110.47	

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XI



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# HEC-RAS 100 Year Existing Conditions Q=996, 2576 cfs

Leonard Jackson PE PLLC dba Leonard Jackson Associates

### 100 Year -Existing Conditions Existing Discharges (996, 2576 CFS)

HEC-RAS Plan: EXISTING River: River 1 Reach: Reach 1 Profile: 100 YR

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
	10.2		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	100 YR	996.00	368.00	372.77		373.12	0.001506	4.41	212.20	99.02	203.80
Reach 1	6283	100 YR	996.00	368.00	372.36		372.69	0.002743	4.67	221.10	128.22	202.73
Reach 1	5968	100 YR	996.00	366.49	371.95		372.14	0.001043	3.37	285.31	154.20	200.90
Reach 1	5656	100 YR	996.00	366.00	371.28		371.65	0.002449	4.95	205.02	91.15	199.15
Reach 1	5380	100 YR	996.00	366.00	371.18		371.28	0.000567	2.59	482.35	328.55	196.95
Reach 1	5156	100 YR	996.00	366.00	371.07		371.17	0.000387	2.28	406.97	111.57	194.62
Reach 1	5064	100 YR	996.00	366.00	370.40		371.06	0.001946	4.13	194.19	104.89	194.03
Reach 1	4996	100 YR	996.00	366.00	370.24		370.87	0.004000	6.18	162.37	100.05	193.76
Reach 1	4959	100 YR	996.00	366.00	370.13	369.39	370.69	0.004505	6.11	174.23	170.11	193.61
Reach 1	4788	100 YR	996.00	365.18	370.23		370.32	0.000540	2.46	404.99	314.67	192.34
Reach 1	4731	100 YR	996.00	365.29	370.24		370.29	0.000240	1.63	645.09	578.36	191.65
Reach 1	4699	100 YR	996.00	366.00	370.15		370.27	0.000885	3.01	568.13	568.69	191.20
Reach 1	4500	100 YR	996.00	366.00	370.18		370.21	0.000124	1.17	848.72	560.14	189.19
Reach 1	4441	100 YR	996.00	365.53	370.18		370.19	0.000065	0.89	978.76	571.05	186.37
Reach 1	4393	100 YR	996.00	366.00	370.18		370.19	0.000029	0.56	1074.43	503.60	185.23
Reach 1	4268	100 YR	996.00	366.00	370.10		370.18	0.000305	1.82	469.58	420.69	183.02
Reach 1	4080	100 YR	996.00	366.00	369.05	369.05	369.93	0.010155	7.59	132.65	75.49	181.71
Reach 1	3575	100 YR	996.00	364.00	369.23		369.28	0.000087	1.06	656.92	191.89	177.14
Reach 1	3290	100 YR	996.00	364.00	369.20		369.26	0.000074	0.99	618.41	190.27	172.96
Reach 1	2949	100 YR	996.00	364.00	369.24	364.58	369.24	0.000002	0.14	2666.76	823.63	158.48
Reach 1	2600	100 YR	2576.00	362.00	369.23		369.24	0.000007	0.41	3466.62	870.63	132.67
Reach 1	2383	100 YR	2576.00	364.00	369.22	364.92	369.23	0.000040	0.79	3237.21	889.84	114.16
Reach 1	2079	100 YR	2576.00	364.00	369.21	364.93	369.22	0.000043	0.82	3079.30	814.90	88.56
Reach 1	1851	100 YR	2576.00	363.65	369.19	365.10	369.21	0.000075	1.09	2323.09	663.16	72.59
Reach 1	1418	100 YR	2576.00	362.13	369.11		369.16	0.000179	1.67	1536.60	344.77	52.25
Reach 1	1121	100 YR	2576.00	363.13	369.09		369.12	0.000083	1.05	2047.17	601.37	40.04
Reach 1	661	100 YR	2576.00	362.00	369.05		369.08	0.000090	1.36	1835.97	330.46	19.53
Reach 1	425	100 YR	2576.00	362.00	369.03		369.06	0.000079	1.31	1909.70	375.64	9.39
Reach 1	50	100 YR	2576.00	364.00	367.46	367.46	368.82	0.012893	9.36	275.18	101.14	

500



26 Firemens Memorial Drive . Pomona, New York 10970 . (845) 354-4382 . FAX (845) 354-4401

# HEC-RAS Post Project Conditions Existing Discharges Q=996, 2576 cfs

Leonard Jackson PE PLLC dba Leonard Jackson Associates

#### 100 Year -

#### **Post-Project Conditions (With Compensating Storage) Existing Discharges (996, 2576 CFS)**

HEC-RAS Plan: Post with CompStorage River: River 1 Reach: Reach 1 Profile: 100 YR

Reach	River Sta	Profile	Q Totał	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Reach 1	6350	100 YR	996.00	368.00	372.77		373.12	0.001507	4.41	212.09	98.84	0.37
Reach 1	6283	100 YR	996.00	368.00	372.36		372.69	0.002748	4.67	220.97	128.12	0.47
Reach 1	5968	100 YR	996.00	366.49	371.95		372.14	0.001042	3.37	285.39	154.24	0.30
Reach 1	5656	100 YR	996.00	366.00	371.27		371.65	0.002472	4.97	204.37	90.97	0.46
Reach 1	5380	100 YR	996.00	366.00	371.18		371.28	0.000569	2.59	481.33	328.24	0.22
Reach 1	5156	100 YR	996.00	366.00	371.07		371.17	0.000387	2.28	406.83	111.56	0.19
Reach 1	5064	100 YR	996.00	366.00	370.33	369.46	371.04	0.002150	4.28	186.65	103.22	0.42
Reach 1	4996	100 YR	996.00	366.00	370.13	369.34	370.84	0.004658	6.53	152.09	90.76	0.62
Reach 1	4959	100 YR	996.00	364.64	370.54		370.61	0.000385	2.11	463.48	213.35	0.18
Reach 1	4788	100 YR	996.00	364.29	370.53		370.56	0.000112	1.23	727.78	320.41	0.10
Reach 1	4731	100 YR	996.00	364.79	370.49		370.55	0.000224	1.62	514.60	321.45	0.14
Reach 1	4699	100 YR	996.00	366.00	370.51		370.54	0.000085	1.03	755.55	324.43	0.09
Reach 1	4500	100 YR	996.00	366.00	370.51		370.53	0.000045	0.76	965.72	359.98	0.06
Reach 1	4441	100 YR	996.00	365.76	370.51		370.52	0.000028	0.61	1058.83	314.80	0.05
Reach 1	4393	100 YR	996.00	366.00	370.48		370.52	0.000068	0.90	674.58	298.62	0.08
Reach 1	4268	100 YR	996.00	366.00	370.29	368.97	370.48	0.001136	3.63	350.79	455.59	0.32
Reach 1	4080	100 YR	996.00	366.00	369.05	369.05	369.93	0.010155	7.59	132.65	75.49	0.87
Reach 1	3575	100 YR	996.00	364.00	369.23		369.28	0.000087	1.06	657.11	191.90	0.09
Reach 1	3290	100 YR	996.00	364.00	369.20		369.26	0.000074	0.99	618.60	190.28	0.08
Reach 1	2949	100 YR	996.00	364.00	369.24	364.58	369.24	0.000002	0.14	2667.37	823.68	0.01
Reach 1	2600	100 YR	2576.00	362.00	369.23		369.24	0.000007	0.41	3467.50	870.68	0.03
Reach 1	2383	100 YR	2576.00	364.00	369.22	364.92	369.23	0.000040	0.79	3237.88	889.86	0.06
Reach 1	2079	100 YR	2576.00	364.00	369.21	364.93	369.22	0.000043	0.82	3079.92	814.91	0.06
Reach 1	1851	100 YR	2576.00	363.65	369.19	365.10	369.21	0.000075	1.09	2323.60	663.24	0.09
Reach 1	1418	100 YR	2576.00	362.13	369.11		369.16	0.000178	1.67	1536.97	344.81	0.13
Reach 1	1121	100 YR	2576.00	363.13	369.09		369.12	0.000083	1.05	2047.85	601.41	0.09
Reach 1	661	100 YR	2576.00	362.00	369.05		369.08	0.000090	1.36	1836.34	330.47	0.10
Reach 1	425	100 YR	2576.00	362.00	369.03		369.06	0.000079	1.31	1910.14	375.66	0.09
Reach 1	50	100 YR	2576.00	364.00	367.45	367.45	368.82	0.013073	9.41	273.84	100.96	1.01



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# HEC-RAS Post Project Conditions Post No Compensating Storage Discharges Q=998, 2580 cfs

Leonard Jackson PE PLLC dba Leonard Jackson Associates

#### 100 Year -

## **Post-Project Conditions (No Compensating Storage)** Post-Project Discharges (998, 2580 CFS) HEC-RAS Plan: Post - No Comp Storage River: River 1 Reach: Reach 1 Profile: 100 YR

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Volume
			(cfs)	(稅)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	(acre-ft)
Reach 1	6350	100 YR	998.00	368.00	372.77		373.12	0.001506	4.41	212.56	99.57	201.79
Reach 1	6283	100 YR	998.00	368.00	372.36		372.69	0.002733	4.66	221.77	128.71	200.72
Reach 1	5968	100 YR	998.00	366.49	371.96		372.15	0.001030	3.36	287.17	155.26	198.88
Reach 1	5656	100 YR	998.00	366.00	371.30		371.67	0.002390	4.91	206.99	91.69	197.11
Reach 1	5380	100 YR	998.00	366.00	371.21		371.31	0.000551	2.56	492.51	331.61	194.90
Reach 1	5156	100 YR	998.00	366.00	371.11		371.21	0.000376	2.26	410.78	111.79	192.58
Reach 1	5064	100 YR	998.00	366.00	370.54		371.10	0.001607	3.86	209.24	107.61	191.92
Reach 1	4996	100 YR	998.00	366.00	370.44		370.94	0.002998	5.55	183.90	113.82	191.62
Reach 1	4959	100 YR	998.00	366.00	370.50		370.78	0.002074	4.46	243.41	203.37	191.43
Reach 1	4788	100 YR	998.00	365.18	370.55		370.61	0.000275	1.85	509.43	329.74	189.95
Reach 1	4731	100 YR	998.00	365.29	370.52		370.60	0.000255	1.76	486.71	322.56	189.30
Reach 1	4699	100 YR	998.00	366.00	370.48		370.58	0.000557	2.52	390.88	304.29	188.98
Reach 1	4500	100 YR	998.00	366.00	370.49		370.53	0.000109	1.16	648.36	360.06	187.51
Reach 1	4441	100 YR	998.00	365.53	370.50		370.52	0.000043	0.76	808.04	314.98	185.26
Reach 1	4393	100 YR	998.00	366.00	370.48		370.52	0.000068	0.90	675.09	298.69	184.44
Reach 1	4268	100 YR	998.00	366.00	370.30	368.98	370.48	0.001131	3.63	352.04	455.66	182.96
Reach 1	4080	100 YR	998.00	366.00	369.05	369.05	369.93	0.010137	7.59	132.91	75.59	181.92
Reach 1	3575	100 YR	998.00	364.00	369.23		369.28	0.000087	1.06	657.73	191.94	177.33
Reach 1	3290	100 YR	998.00	364.00	369.21		369.26	0.000074	0.99	619.22	190.32	173.15
Reach 1	2949	100 YR	998.00	364.00	369.24	364.58	369.24	0.000002	0.14	2669.36	823.84	158.65
Reach 1	2600	100 YR	2580.00	362.00	369.23		369.24	0.000007	0.41	3470.37	870.86	132.91
Reach 1	2383	100 YR	2580.00	364.00	369.23	364.92	369.24	0.000040	0.79	3240.16	889.93	114.27
Reach 1	2079	100 YR	2580.00	364.00	369.21	364.92	369.23	0.000043	0.82	3082.03	814.96	88.65
Reach 1	1851	100 YR	2580.00	363.65	369.19	365.10	369.21	0.000075	1.10	2325.25	663.83	72.66
Reach 1	1418	100 YR	2580.00	362.13	369.12		369.16	0.000179	1.67	1538.09	344.94	52.31
Reach 1	1121	100 YR	2580.00	363.13	369.09		369.12	0.000083	1.05	2049.79	601.52	40.08
Reach 1	661	100 YR	2580.00	362.00	369.05		369.08	0.000090	1.36	1837.40	330.51	19.55
Reach 1	425	100 YR	2580.00	362.00	369.03		369.06	0.000079	1.31	1911.34	375.74	9.40
Reach 1	50	100 YR	2580.00	364.00	367.46	367.46	368.83	0.012891	9.36	275.50	101.19	



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# HEC-HMS (100 Year Summary Table)

Leonard Jackson PE PLLC dba Leonard Jackson Associates



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# HEC-HMS 100 Year Existing Conditions

Leonard Jackson PE PLLC dba Leonard Jackson Associates

EXISTING CONDITION HE-HUS File Edit View Components GIS Parameters Compute Results Tools 🏤 🖶 🗐 🍄 🌩 🙄 🚵 🕂 -None Select . 쮬 BYRAM **Basin Models** 🖻 🖉 Post Project 🗄 🛃 Byram river Routed E Pre Project 🗄 😹 Byram River - Routed Meteorologic Models 回 第 100 year - Ad Hypothetical Storm Control Specifications E Paired Data 8 Storage-Discharge Functions Storage Discharge(Existing) Storage-Discharge(PostProje) Components Compute Results Pared Data Table Graph Storage (ACRE-FT) Discharge (CFS) 0.00 0 100 13.16 . 37.60 . 436

796

996

1200

60.69 .

71.15,

83.304

65

Standard Report

EXISTING

Project: Byram Simulation Run: Run I Simulation Start: 8 September 2020, 24:00 Simulation End: 10 September 2020, 00:04

HMS Version: 4.6.1 Executed: 09 October 2020, 21:17

#### Global Parameter Summary - Subbasin

	Area		
Element Name		Area	
Byram River		4-59	
	Downstream		
Element Name		Downstream	
Byram River		Routed	
	Loss Rate: Scs		
Element Name	Percent Impervious Area	Curve Number	Initial Abstraction
Byram River	2.95	66	I
	Transform: Scs		
Element Name	Lag	Unitgra	oh Type
Byram River	132	Prfi	150

#### Global Results Summary

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
Byram River	4.59	1010.21	09Sep2020, 15:20	3.35
Routed	4.59	996.03	09Sep2020, 16:12	3.16
		LEXISTIC		

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66

.

820.65

0

#### Subbasin: Byram River

Direct Runoff Volume (AC - FT)

Baseflow Volume (AC - FT)

Area : 4.59 Downstream : Routed

	Loss Rate: Scs	
Percent Impervious Area	2.95	
Curve Number	66	
Initial Abstraction	I	
	Transform: Scs	
Lag	132	
Unitgraph Type	Prf150	
Re	sults: Byram River	
Peak Discharge (CFS)	1010.21	
Time of Peak Discharge	09Sep2020, 15:20	
Volume (IN)	3-35	
Precipitation Volume (AC - FT)	2053.47	
Loss Volume (AC - FT)	958.56	
Excess Volume (AC - FT)	1094.91	









#### **Cumulative Precipitation**









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Soil Infiltration



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#### Reservoir: Routed

	Results: Routed	RACTING
Peak Discharge (CFS)	996.03	iction in o
Time of Peak Discharge	09Sep2020, 16:12	
Volume (IN)	3.16	
Peak Inflow (CFS)	1010.21	
Time of Peak Inflow	09Sep2020, 15:20	
Inflow Volume (AC - FT)	820.65	
Maximum Storage (AC - FT)	71.15	
Peak Elevation (FT)	Not specified	
Discharge Volume (AC - FT)	773.96	
Observed Pool Elevation Gage	Not specified	
Observed Peak Pool Elevation (FT)	Not specified	
Observed Pool Elevation RMSE Stdev	Not specified	
Observed Pool Elevation Percent Bias	Not specified	
Time of Maximum Observed Pool Elevation	Not specified	
Observed Pool Elevation Nash Sutcliffe	Not specified	








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Leonard Jackson Associates Consulting Engineers

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# HEC-HMS 100 Year Post Project Conditions No Compensating Flood Storage

Leonard Jackson PE PLLC dba Leonard Jackson Associates

			Pas	+ Proj	ect
File Edit	View Component	ts GIS Parame	ters Compute	e Results Tools	
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BYRAM Bas	in Models Port Project				
	Byram river Routed Pre Project Byram River				
E Met	teorologic Models 100 vear				
Der Cor	O Hypothetical Storn Itrol Specifications	n			
e g Par	ed Data Storage-Discharge Fui Storage-Discharge Storage Discharge	nctions :(Exisbng) :(PastProje)			

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## Components Compute Results

1.

Paired Data Table Graph

Storage (ACRE-FT)		Discharge (CFS)	
	0.00		0
	12.98		100
	35.00	•	436
	56.29		796
	66.15		995
	77.90	14) 18 MB	1200

10/9/2020-

Standard Report

Project: Byram Simulation Run: Run 1 Simulation Start: 8 September 2020, 24:00 Simulation End: 10 September 2020, 00:04

HMS Version: 4.6.1 Executed: 09 October 2020, 22:13

## Global Parameter Summary - Subbasin

Area		
	Area	
	4-59	
Downstream		
	Downstream	
	Routed	
Loss Rate: Scs		
Percent Impervious Area	Curve Number	Initial Abstraction
2.95	66	I
Transform: Scs		
Lag	Unitgra	oh Type
170	Dreft	50
	Area Downstream Loss Rate: Scs Percent Impervious Area 2.95 Transform: Scs Lag	Area Area 4.59 Downstream Downstream Routed Loss Rate: Scs Percent Impervious Area 2.95 66 Transform: Scs Lag Unitgrag

## Global Results Summary



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# Subbasin: Byram River

Direct Runoff Volume (AC - FT)

Baseflow Volume (AC - FT)

#### Area : 4.59 Downstream : Routed

	Loss Rate: Scs	
Percent Impervious Area	2.95	
Curve Number	66	
Initial Abstraction	I	
	Fransform: Scs	
Lag	132	
Unitgraph Type	Prf150	
Res	ults: Byram River	
Peak Discharge (CFS)	1010.21	
Time of Peak Discharge	09Sep2020, 15:20	
Volume (IN)	3-35	
Precipitation Volume (AC - FT)	2053.47	
Loss Volume (AC - FT)	958.56	
Excess Volume (AC - FT)	1094.91	

820.65 0









#### **Cumulative Precipitation**



Cumulative Precipitation Loss



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**Results: Routed** 

## Reservoir: Routed

Peak Discharge (CFS) Time of Peak Discharge Volume (IN) Peak Inflow (CFS) Time of Peak Inflow Inflow Volume (AC - FT) Maximum Storage (AC - FT) Peak Elevation (FT) Discharge Volume (AC - FT) **Observed Pool Elevation Gage Observed Peak Pool Elevation (FT) Observed Pool Elevation RMSE** Stdev **Observed Pool Elevation Percent** Bias Time of Maximum Observed Pool Elevation **Observed Pool Elevation Nash** Sutcliffe

997.96 09Sep2020, 16:08 3.18 1010.21 09Sep2020, 15:20 820.65 66.26 Not specified 777.47 Not specified Not specified Not specified Not specified

Not specified

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From:	<u>Alison Simon</u>
To:	Paul R. Sysak, RLA, ASLA; Paul J. Dumont, EIT
Cc:	Kevin Hay; "Roland Baroni (RBaroni@SBRLLaw.com)"; Barbara Pesquera; Maria Scharf; Robert Melillo; "John
	Kellard"; "Joe Cermele"; Adam Kaufman
Subject:	12/9/2020 TB Action Re 100 Business Park Drive - Application for Variance
Date:	Thursday, December 10, 2020 2:54:23 PM
Attachments:	IV.E 100 Business Park Drive Flood Study Review Kellard Memo Signed 2020-12-03 NCTB .pdf

At the 12/9/2020 meeting the Town Board moved receipt of Memo from Town Engineers Kellard Sessions, dated December 3, 2020, regarding the application from A&R Real Estate Holdings LLC for Variance from Town Code Section 177-14(3) Floodplain Compensatory Storage for a new warehouse at 100 Business Park Drive, Armonk, and further moved approval of requested variance.

# Alison Simon

North Castle Town Clerk

From: Alison Simon
Sent: Thursday, November 19, 2020 12:33 PM
To: 'Paul R. Sysak, RLA, ASLA' <PSysak@jmcpllc.com>; 'Paul J. Dumont, EIT'
<PDumont@jmcpllc.com>; 'Joe Cermele' <jcermele@kelses.com>; Adam Kaufman
<akaufman@northcastleny.com>
Cc: Kevin Hay <khay@northcastleny.com>; Roland Baroni (RBaroni@SBRLLaw.com)
<RBaroni@SBRLLaw.com>; Barbara Pesquera <bpesquera@northcastleny.com>; Maria Scharf
<mscharf@northcastleny.com>
Subject: 11/18/2020 TB Action Re 100 Business Park Drive - Application for Variance

At the 11/18/2020 meeting the Town Board moved receipt of the application from A&R Real Estate Holdings LLC for Variance from Town Code Section 177-14(3) Floodplain Compensatory Storage for a new warehouse at 100 Business Park Drive, Armonk; and receipt of Memo from Town Engineers Kellard Sessions, dated November 12, 2020. The Town Board requested that Kellard Sessions review specifics of the application with the applicant and provide the Board with a revised memo at the December 9, 2020 Town Board Meeting.

Please provide revised memo by 12/2/20 for inclusion on the 12/9/20 TBM Agenda.

Thank you,

# Alison Simon

North Castle Town Clerk 15 Bedford Road Armonk, NY 10504 914-273-3000 ext. 42 asimon@northcastlenv.com

#### STORMWATER CONTROL FACILITY MAINTENANCE AGREEMENT WITH THE TOWN OF NORTH CASTLE

**THIS AGREEMENT**, entered into this \_\_\_\_\_ day of \_\_\_\_\_\_, 2021, by and between the Town of North Castle, New York ("Town"), a municipal corporation organized and existing under the laws of the State of New York with offices at 15 Bedford Road, North Castle, New York 10504 and A&R Real Estate Holdings LLC ("Company"), a domestic limited liability company organized and existing under the laws of the State of New York with offices at 100 Business Park Drive, Armonk, NY 10504;

**WHEREAS,** that the Town and the Company (collectively "Parties"), for the consideration hereinafter named, agree as follows:

WHEREAS, the Town and the Company wish to enter into an agreement to provide for the long term maintenance and continuation of stormwater control measures approved by the Town for the project located on 100 Business Park Drive, Armonk, NY 10567 and Section Block and Lot Number 108.03-1-51 ("Project");

WHEREAS, the Town and the Company desire that the storm water control measures be built in accordance with the approved project plans and thereafter be maintained, cleaned, repaired, replaced and continued in perpetuity in order to ensure optimum performance of the components.

**THEREFORE**, the Town and the Company agree as follows:

1. This Agreement binds the Town and the Company, its successors and assigns, to the maintenance provisions depicted in the approved project plans which are attached as Schedule A of this Agreement.

2. The Company shall maintain, clean, repair, replace and continue the stormwater control measures depicted in the Maintenace Schedule provided within the approved Stormwater Pollution Prevention Plan (SWPPP) as necessary to ensure optimum performance of the measures to design specifications. The stormwater control measures shall include, but shall not be limited to, the following: infiltration basin, hydrodynamic separators, porous pavement sections, and conveyance systems.

3. The Company shall be responsible for all expenses related to the maintenance of the stormwater control measures and shall establish a means for the collection and distribution of expenses among parties for any commonly owned facilities.

4. The Company shall provide for the periodic inspection of the stormwater control measures, not less than once in every five year period, to determine the condition and integrity of the measures.

5. The Company shall not authorize, undertake or permit alteration, abandonment,

modification or discontinuation of the stormwater control measures except in accordance with written approval of the Town.

6. The Company shall undertake necessary repairs and replacement of the stormwater control measures at the direction of the Town or in accordance with the recommendations of the Town Engineer.

7. This Agreement shall be recorded in the Office of the County Clerk, County of Westchester together with the deed for the common property and shall be included in the offering plan and/or prospectus in connection with the Project. The Company shall be responsible for payment of any fees in connection with the recording with the Office of the County Clerk.

8. If ever the Town determines that the Company has failed to construct or maintain the stormwater control measures in accordance with the project plan or has failed to undertake corrective action specified by the Town or by the Town Engineer, the Town shall serve on the Company the notice to cure on thirty (30) days' notice. If the Company fails to comply with the notice to cure to the discretion of the Town Engineer, the Company hereby consents to the Town undertaking such measures and steps as reasonably necessary for the preservation, continuation or maintenance of the stormwater control measures and to affix the expenses thereof as a lien against the property. In the event that the Town is required to undertake such measures as a result of the Company failing to comply with the notice to cure, the Company shall be required to deposit with the Town an escrow amount determined by the Town Engineer. Nothing in this Agreement prevents the Town from immediately undertaking such measures and steps as reasonably necessary for the preservation or maintenance of the stormwater control measures in the event of an emergency in the discretion of the Town Engineer and to affix the expenses thereof as a lien against the property.

9. Any and all notices required hereunder shall be addressed as follows, or to such other address as may hereafter be designated in writing by either party hereto:

To Town of Yorktown:

Town Clerk Town Hall 15 Bedford Road Armonk, New York 10504

With a copy to:

Town Consulting Engineer Town Hall 15 Bedford Road Armonk, New York 10504 Town Attorney Town Hall 15 Bedford Road Armonk, New York 10504

To Company:

At the address first above written

10. The Company hereby agrees to indemnify and save harmless the Town, its officers, employees, elected officials, and agents from and against all liability, loss or damage the Town may suffer, arising directly or indirectly out of the contract between the Company and the Town. The Company further agrees to provide defense for and defend any claims or causes of action of any kind or character directly or indirectly arising out of this Agreement at its sole expense and agrees to bear all other costs and expenses relating thereto.

11. This Agreement constitutes the entire Agreement between the Parties in connection with the long term maintenance and continuation of stormwater control measures approved by the Town for the Project and supersedes any and all prior agreements, whether oral or written. If one or more of the provisions in this Agreement are deemed by a Court of competent jurisdiction to be void by law, then the remaining provisions will continue in full force and effect. This Agreement may not be amended or modified except by an instrument in writing signed by all Parties. There will be no presumption against any Party (or its counsel) on the ground that such Party (or its counsel) was responsible for preparing this Agreement or any part of it.

12. Each and every provision of law and clause required by law to be inserted in this Agreement shall be deemed to have been inserted herein. If any required contractual provision is not inserted, through mistake or otherwise, then upon the application of either party, this Contract shall be physically amended forthwith to make such insertion.

13. This Agreement shall be governed by and construed in accordance with the laws of the State of New York without giving effect to that State's choice of law rules. The Parties hereby submit to the exclusive jurisdiction of the Supreme Court of the State of New York, County of Westchester, in any action or proceeding arising out of or relating to this Agreement.

#### IN WITNESS WHEREOF, the Parties hereto have executed this Agreement:

TOWN OF NORTH CASTLE			A&R REAL ESTATE HOLDINGS LLC	
By:			By:	
Michael Schiliro, Town Supe	ervisor		Robert Troccoli, Member	
STATE OF NEW YORK	)			
	)	ss.:		
COUNTY OF WESTCHESTER	)			

On the \_\_\_\_\_ day of \_\_\_\_\_\_ in the year 2021, before me, the undersigned, personally appeared Matthew Slater personally known to me or proved to me on the same basis of satisfactory evidence to be the individual(s) whose names(s) is (are) subscribed to the within instrument and acknowledge to me that he/she/they executed the same in his/her/their capacity(ies), and that by his/her/their signature(s) on the instrument, the individual(s), or the person upon behalf of which the individual(s) acted, executed the instrument.

Notary Public

Commission Expires:\_\_\_\_\_

STATE OF NEW YORK )

ss.:

)

COUNTY OF WESTCHESTER )

On the \_\_\_\_\_ day of \_\_\_\_\_\_ in the year 2021 before me, the undersigned, personally appeared Robert Troccoli personally known to me or proved to me on the same basis of satisfactory evidence to be the individual(s) whose names(s) is (are) subscribed to the within instrument and acknowledge to me that he/she/they executed the same in his/her/their capacity(ies), and that by his/her/their signature(s) on the instrument, the individual(s), or the person upon behalf of which the individual(s) acted, executed the instrument.

Notary Public

Commission Expires:\_\_\_\_\_

APPROVED AS TO FORM

Town Attorney

# NOI for coverage under Stormwater General Permit for Construction Activity

?

Alternate ID Proposed Warehouse Submission HP5-P8KT-1VHTK Revision 1 Form Version 1.29

#### Review

This step allows you to review the form to confirm the form is populated completely and accurately, prior to certification and submission.

Please note: Any work you perform filling out a form will not be accessible by NYSDEC staff or the public until you actually submit the form in the 'Certify & Submit' step.

# **OWNER/OPERATOR INFORMATION** Owner/Operator Name (Company/Private Owner/Municipality/Agency/Institution, etc.) A&R Real Estate Holdings LLC **Owner/Operator Contact Person Last Name (NOT CONSULTANT)** Troccoli **Owner/Operator Contact Person First Name** Robert **Owner/Operator Mailing Address** 100 Business Park Drive City Armonk State New York Zip 10504 Phone 7186555450 Email Rob@Jantile.com

#### Federal Tax ID

None Specified

PROJECT LOCATION
Project/Site Name Proposed Warehouse
<b>Street Address (Not P.O. Box)</b> 100 Business Park Drive
Side of Street East
City/Town/Village (THAT ISSUES BUILDING PERMIT) North Castle
State NY
<b>Zip</b> 10504
County WESTCHESTER
DEC Region 3
Name of Nearest Cross Street Bedford Road (Route 22)
Distance to Nearest Cross Street (Feet) 1000
Project In Relation to Cross Street South
Tax Map Numbers Section-Block-Parcel 108.03-1-51
Tax Map Numbers 108.03

#### 1. Coordinates

Provide the Geographic Coordinates for the project site. The two methods are:

- Navigate to the project location on the map (below) and click to place a marker and obtain the XY coordinates.

- The "Find Me" button will provide the lat/long for the person filling out this form. Then pan the map to the correct location and click the map to place a marker and obtain the XY coordinates.

#### Navigate to your location and click on the map to get the X,Y coordinates

Latitude	Longitude
41.11839442588413	-73.7080564787985

#### **PROJECT DETAILS**

#### 2. What is the nature of this project?

New Construction

3. Select the predominant land use for both pre and post development conditions.

#### **Pre-Development Existing Landuse**

Forest

#### Post-Development Future Land Use

Commercial

**3a. If Single Family Subdivision was selected in question 3, enter the number of subdivision lots.** *None Specified* 

4. In accordance with the larger common plan of development or sale, enter the total project site acreage, the acreage to be disturbed and the future impervious area (acreage)within the disturbed area.

\*\*\* ROUND TO THE NEAREST TENTH OF AN ACRE. \*\*\*

#### Total Site Area (acres)

11.26

**Total Area to be Disturbed (acres)** 4.60

Existing Impervious Area to be Disturbed (acres) 1.11

**Future Impervious Area Within Disturbed Area (acres)** 2.34

#### 5. Do you plan to disturb more than 5 acres of soil at any one time?

No

6. Indicate the percentage (%) of each Hydrologic Soil Group(HSG) at the site.
<b>A (%)</b> O
<b>B (%)</b> 37.1
<b>C (%)</b> 0
D (%) 62.9
7. Is this a phased project? No
8. Enter the planned start and end dates of the disturbance activities.
<b>Start Date</b> 1/1/2022
End Date 6/1/2021
9. Identify the nearest surface waterbody(ies) to which construction site runoff will discharge. Byram River
9a. Type of waterbody identified in question 9? River Off Site
<b>Other Waterbody Type Off Site Description</b> None Specified
<b>9b. If "wetland" was selected in 9A, how was the wetland identified?</b> None Specified
<b>10. Has the surface waterbody(ies in question 9 been identified as a 303(d) segment in Appendix E of GP-0-20-001?</b> No
11. Is this project located in one of the Watersheds identified in Appendix C of GP-0-20-001? No

12. Is the project located in one of the watershed areas associated with AA and AA-S classified water	rs?
No	

If No, skip question 13.

13. Does this construction activity disturb land with no existing impervious cover and where the Soil Slope Phase is identified as an E or F on the USDA Soil Survey?

None Specified

If Yes, what is the acreage to be disturbed? None Specified

14. Will the project disturb soils within a State regulated wetland or the protected 100 foot adjacent area? No

**15. Does the site runoff enter a separate storm sewer system (including roadside drains, swales, ditches, culverts, etc)?** Yes

16. What is the name of the municipality/entity that owns the separate storm sewer system? North Castle

17. Does any runoff from the site enter a sewer classified as a Combined Sewer? No

18. Will future use of this site be an agricultural property as defined by the NYS Agriculture and Markets Law? No

19. Is this property owned by a state authority, state agency, federal government or local government? No

20. Is this a remediation project being done under a Department approved work plan? (i.e. CERCLA, RCRA, Voluntary Cleanup Agreement, etc.)

No

#### **REQUIRED SWPPP COMPONENTS**

21. Has the required Erosion and Sediment Control component of the SWPPP been developed in conformance with the current NYS Standards and Specifications for Erosion and Sediment Control (aka Blue Book)? Yes

22. Does this construction activity require the development of a SWPPP that includes the post-construction stormwater management practice component (i.e. Runoff Reduction, Water Quality and Quantity Control practices/techniques)? Yes

If you answered No in question 22, skip question 23 and the Post-construction Criteria and Post-construction SMP Identification sections.

## 23. Has the post-construction stormwater management practice component of the SWPPP been developed in conformance with the current NYS Stormwater Management Design Manual? Yes

#### 24. The Stormwater Pollution Prevention Plan (SWPPP) was prepared by:

Professional Engineer (P.E.)

#### SWPPP Preparer

JMC Planning Engineering Landscape Architecture & Land Surveying, PLLC

#### Contact Name (Last, Space, First)

Lombardi, , David

#### Mailing Address

120 Bedford Road

#### City

Armonk

#### State

NY

#### Zip

10504

#### Phone

9142735225

#### Email

DLombardi@JMCPLLC.com

#### **Download SWPPP Preparer Certification Form**

Please take the following steps to prepare and upload your preparer certification form:

1) Click on the link below to download a blank certification form

- 2) The certified SWPPP preparer should sign this form
- 3) Scan the signed form

4) Upload the scanned document

Download SWPPP Preparer Certification Form

#### Please upload the SWPPP Preparer Certification

No files uploaded

#### Comment

None Specified

At least one file is required.

#### 25. Has a construction sequence schedule for the planned management practices been prepared? Yes

26. Select all of the erosion and sediment control practices that will be employed on the project site:

#### **Temporary Structural** Dust Control Silt Fence Stabilized Construction Entrance Storm Drain Inlet Protection

#### Biotechnical

None

#### Vegetative Measures

Mulching Protecting Vegetation Seeding

#### Permanent Structural

None

#### Other

None Specified

#### POST-CONSTRUCTION CRITERIA

#### \* IMPORTANT: Completion of Questions 27-39 is not required if response to Question 22 is No.

27. Identify all site planning practices that were used to prepare the final site plan/layout for the project.

Preservation of Undisturbed Area

Preservation of Buffers

27a. Indicate which of the following soil restoration criteria was used to address the requirements in Section 5.1.6("Soil Restoration") of the Design Manual (2010 version).

All disturbed areas will be restored in accordance with the Soil Restoration requirements in Table 5.3 of the Design Manual (see page 5-22).

**28.** Provide the total Water Quality Volume (WQv) required for this project (based on final site plan/layout). (Acre-feet) 0.33

#### 29. Post-construction SMP Identification

Use the Post-construction SMP Identification section to identify the RR techniques (Area Reduction), RR techniques(Volume Reduction) and Standard SMPs with RRv Capacity that were used to reduce the Total WQv Required (#28).

Identify the SMPs to be used by providing the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

Note: Redevelopment projects shall use the Post-Construction SMP Identification section to identify the SMPs used to treat and/or reduce the WQv required. If runoff reduction techniques will not be used to reduce the required WQv, skip to question 33a after identifying the SMPs.

30. Indicate the Total RRv provided by the RR techniques (Area/Volume Reduction) and Standard SMPs with RRv capacity identified in question 29. (acre-feet)

0.24

**31. Is the Total RRv provided (#30) greater than or equal to the total WQv required (#28)?** No

If Yes, go to question 36. If No, go to question 32.

**32.** Provide the Minimum RRv required based on HSG. [Minimum RRv Required = (P) (0.95) (Ai) / 12, Ai=(s) (Aic)] (acre-feet) 0.11

**32a. Is the Total RRv provided (#30) greater than or equal to the Minimum RRv Required (#32)?** Yes

#### If Yes, go to question 33.

Note: Use the space provided in question #39 to summarize the specific site limitations and justification for not reducing 100% of WQv required (#28). A detailed evaluation of the specific site limitations and justification for not reducing 100% of the WQv required (#28) must also be included in the SWPPP.

If No, sizing criteria has not been met; therefore, NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

#### 33. SMPs

Use the Post-construction SMP Identification section to identify the Standard SMPs and, if applicable, the Alternative SMPs to be used to treat the remaining total WQv (=Total WQv Required in #28 - Total RRv Provided in #30).

Also, provide the total impervious area that contributes runoff to each practice selected.

NOTE: Use the Post-construction SMP Identification section to identify the SMPs used on Redevelopment projects.

# 33a. Indicate the Total WQv provided (i.e. WQv treated) by the SMPs identified in question #33 and Standard SMPs with RRv Capacity identified in question #29. (acre-feet)

0.33

Note: For the standard SMPs with RRv capacity, the WQv provided by each practice = the WQv calculated using the contributing drainage area to the practice - provided by the practice. (See Table 3.5 in Design Manual)

**34. Provide the sum of the Total RRv provided (#30) and the WQv provided (#33a).** 0.57

**35.** Is the sum of the RRv provided (#30) and the WQv provided (#33a) greater than or equal to the total WQv required (#28)? Yes

If Yes, go to question 36.

If No, sizing criteria has not been met; therefore, NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

#### 36. Provide the total Channel Protection Storage Volume (CPv required and provided or select waiver (#36a), if applicable.

#### CPv Required (acre-feet)

None Specified

#### **CPv Provided (acre-feet)**

None Specified

#### 36a. The need to provide channel protection has been waived because:

Reduction of the total CPv is achieved on site through runoff reduction techniques or infiltration systems.

37. Provide the Overbank Flood (Qp) and Extreme Flood (Qf) control criteria or select waiver (#37a), if applicable.

Overbank Flood Control Criteria (Qp)

Pre-Development (CFS) 17.98

Post-Development (CFS) 17.45

Total Extreme Flood Control Criteria (Qf)

Pre-Development (CFS) 38.16 37.93

37a. The need to meet the Qp and Qf criteria has been waived because:

None Specified

38. Has a long term Operation and Maintenance Plan for the post-construction stormwater management practice(s) been developed?

Yes

If Yes, Identify the entity responsible for the long term Operation and Maintenance A&R Real Estate Holdings LLC

39. Use this space to summarize the specific site limitations and justification for not reducing 100% of WQv required (#28). (See question #32a) This space can also be used for other pertinent project information. None Specified

POST-CONSTRUCTION SMP IDENTIFICATION

#### Runoff Reduction (RR) Techniques, Standard Stormwater Management Practices (SMPs) and Alternative SMPs

Identify the Post-construction SMPs to be used by providing the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

#### RR Techniques (Area Reduction)

Round to the nearest tenth

**Total Contributing Acres for Conservation of Natural Area (RR-1)** None Specified

**Total Contributing Impervious Acres for Conservation of Natural Area (RR-1)** *None Specified* 

**Total Contributing Acres for Sheetflow to Riparian Buffers/Filter Strips (RR-2)** *None Specified* 

**Total Contributing Impervious Acres for Sheetflow to Riparian Buffers/Filter Strips (RR-2)** None Specified

**Total Contributing Acres for Tree Planting/Tree Pit (RR-3)** None Specified

Total Contributing Impervious Acres for Tree Planting/Tree Pit (RR-3)

None Specified

#### Total Contributing Acres for Disconnection of Rooftop Runoff (RR-4)

None Specified

#### **RR Techniques (Volume Reduction)**

**Total Contributing Impervious Acres for Disconnection of Rooftop Runoff (RR-4)** None Specified

**Total Contributing Impervious Acres for Vegetated Swale (RR-5)** None Specified

**Total Contributing Impervious Acres for Rain Garden (RR-6)** None Specified

**Total Contributing Impervious Acres for Stormwater Planter (RR-7)** *None Specified* 

**Total Contributing Impervious Acres for Rain Barrel/Cistern (RR-8)** None Specified

**Total Contributing Impervious Acres for Porous Pavement (RR-9)** 0.47

Total Contributing Impervious Acres for Green Roof (RR-10) None Specified

Standard SMPs with RRv Capacity

Total Contributing Impervious Acres for Infiltration Trench (I-1) None Specified

**Total Contributing Impervious Acres for Infiltration Basin (I-2)** 1.72

**Total Contributing Impervious Acres for Dry Well (I-3)** None Specified

**Total Contributing Impervious Acres for Underground Infiltration System (I-4)** *None Specified* 

**Total Contributing Impervious Acres for Bioretention (F-5)** *None Specified* 

**Total Contributing Impervious Acres for Dry Swale (0-1)** None Specified Total Contributing Impervious Acres for Micropool Extended Detention (P-1) None Specified

**Total Contributing Impervious Acres for Wet Pond (P-2)** None Specified

**Total Contributing Impervious Acres for Wet Extended Detention (P-3)** *None Specified* 

**Total Contributing Impervious Acres for Multiple Pond System (P-4)** None Specified

**Total Contributing Impervious Acres for Pocket Pond (P-5)** None Specified

**Total Contributing Impervious Acres for Surface Sand Filter (F-1)** *None Specified* 

**Total Contributing Impervious Acres for Underground Sand Filter (F-2)** None Specified

**Total Contributing Impervious Acres for Perimeter Sand Filter (F-3)** *None Specified* 

**Total Contributing Impervious Acres for Organic Filter (F-4)** None Specified

**Total Contributing Impervious Acres for Shallow Wetland (W-1)** None Specified

**Total Contributing Impervious Acres for Extended Detention Wetland (W-2)** None Specified

**Total Contributing Impervious Acres for Pond/Wetland System (W-3)** None Specified

**Total Contributing Impervious Acres for Pocket Wetland (W-4)** None Specified

**Total Contributing Impervious Acres for Wet Swale (0-2)** None Specified

Alternative SMPs (DO NOT INCLUDE PRACTICES BEING USED FOR PRETREATMENT ONLY)

#### Total Contributing Impervious Area for Hydrodynamic

0.51

#### Total Contributing Impervious Area for Wet Vault

None Specified

#### Total Contributing Impervious Area for Media Filter

None Specified

#### "Other" Alternative SMP?

None Specified

#### Total Contributing Impervious Area for "Other" None Specified

Provide the name and manufaturer of the alternative SMPs (i.e. proprietary practice(s)) being used for WQv treatment.

Note: Redevelopment projects which do not use RR techniques, shall use questions 28, 29, 33 and 33a to provide SMPs used, total WQv required and total WQv provided for the project.

#### Manufacturer of Alternative SMP

Hydro International

#### Name of Alternative SMP

First Defense

#### **OTHER PERMITS**

40. Identify other DEC permits, existing and new, that are required for this project/facility. None

If SPDES Multi-Sector GP, then give permit ID None Specified

# If Other, then identify

None Specified

41. Does this project require a US Army Corps of Engineers Wetland Permit? No

If "Yes," then indicate Size of Impact, in acres, to the nearest tenth None Specified

42. If this NOI is being submitted for the purpose of continuing or transferring coverage under a general permit for stormwater runoff from construction activities, please indicate the former SPDES number assigned. *None Specified* 

#### **43. Is this project subject to the requirements of a regulated, traditional land use control MS4?** Yes - Please attach the MS4 Acceptance form below

#### If No, skip question 44

44. Has the "MS4 SWPPP Acceptance" form been signed by the principal executive officer or ranking elected official and submitted along with this NOI?

Yes

#### MS4 SWPPP Acceptance Form Download

Download form from the link below. Complete, sign, and upload.

MS4 SWPPP Acceptance Form

#### MS4 Acceptance Form Upload

No files uploaded

**Comment** None Specified

#### OWNER/OPERATOR CERTIFICATION

The owner/operator must download, sign, and upload the certification form in order to complete this application.

#### **Owner/Operator Certification Form Download**

Download the certification form by clicking the link below. Complete, sign, scan, and upload the form.

Owner/Operator Certification Form (PDF, 45KB)

#### Upload Owner/Operator Certification Form

No files uploaded

**Comment** None Specified

At least one file is required.

Permit No.	
Stream	

#### COUNTY OF WESTCHESTER DEPARTMENT OF PUBLIC WORKS AND TRANSPORTATION 148 MARTINE AVENUE WHITE PLAINS, NEW YORK 10601

#### APPLICATION FOR PERMIT TO DO WORK WITHIN CHANNEL LINES OR WITHIN 100 FEET THEREFROM (To Be Executed in Triplicate)

TO: Hugh J. Greechan, Jr., P.E. Commissioner, Public Works and Transportation

Application is hereby made for a permit, under Chapter 241, Article III of the Westchester County Administrative Code, to carry out the following described project within channel lines or within 100 feet therefrom.

1.	Applicant A&R Real Estate Holdings LLC
2.	Address 100 Business Park Drive, Armonk, NY
	(Business or Residence) 10504
3.	Zip Code       Telephone         Owner       A&R Real Estate Holdings LLC       Deed recorded in Liber       10972       Page       309
4.	Tax Lot(s)         51         Tax Block(s)         I         Sheet No.(s)         108.03
5.	Name and Location of Stream Byram River North Castle
6.	Location of Stream 4,500± (Stream) (Municipality) feet downstream from Interstate 684
	(Highway)
7.	Proposed project and extent <u>Construction of a 74,850 square foot warehouse with associated improvements consisting of on-street parking, according a street parking, according the street parking, according the street parking according to the street parking of the street parking according to be constructed in the undeveloped southern portion of the site, and the existing building is proposed to remain.</u>
8.	Work To Be Started   IBD   To Be Completed   IBD
9.	Water Elevations (F.E.M.A. Flood Insurance) 10 Yr. N/A 100 Yr. 370.00
10.	Applicant attaches herewith THREE copies of the plans showing location of work, distances to nearest highway crossings, and details of design stamped by a Professional Engineer or Registered Architect. Applicant will furnish additional data when required by the Commissioner.
11.	List all approvals, certifications or permits required by Federal, State or Local Agencies: Westchester County Department of Public Works and Transportation
	Town of North Castle - Site Plan Approval, Area Variance, Building Permit
	NYSDEC - SPDES Permit
	Witness:
	(Applicant's Signature)

Approval and consent to grant the permit requested by this applicant is hereby given by the duly authorized municipal representative.

**NOTE:** Application must be signed by municipality prior to submission.

Signature and Title

Municipality

# THE PERMIT IS ISSUED SUBJECT TO THE FOLLOWING TERMS, AGREEMENTS, COVENANTS AND CONDITIONS

- 1) The term "Commissioner" shall mean the Commissioner of Public Works and Transportation of the County of Westchester or his authorized representative.
- 2) The term "Permittee" shall mean the applicant obtaining the permit, or his or its duly authorized agents or representatives.
- 3) The Commissioner, by issuing the permit, assumes no liability or responsibility on his part or on the part of the County of Westchester of any kind or nature for this sufficiency of the design or the operations covered by the Permit.
- 4) The Permittee assumes all risks in the operations covered by the permit and shall be solely responsible and answerable in damages for all accidents or injuries to persons or property.
- 5) The Permittee shall indemnify and save harmless the County of Westchester and the Commissioner of Public Works and Transportation from any and all claims, suits, losses, damage to property or injury to persons of whatsoever kind and nature, whether direct or indirect, arising out of the Permittee's operations under the permit, and the Permittee agrees to reimburse the County of Westchester and the Commissioner of Public Works and Transportation for all expenses, costs of judgments to which they may be put arising from such operation.
- 6) No changes in the plans or in the nature and extent of the work shall be made without the Commissioner's written consent. The project shall be subject at all times to inspection by the Commissioner.
- 7) The Permittee agrees that, during the performance of the work, the Permittee will not cause or allow in any way or manner any unreasonable interference with the free flow of the stream, and that the Permittee will not place, store or dump any materials, equipment or debris in or about the stream or channel in any way which may cause interference with the free flow of water.
- 8) The Permittee, upon completion of the work, shall cause to be removed from within the channel lines and within 100 feet therefrom all equipment, surplus materials, debris and structures not shown on the approved plans.
- 9) Within 30 days after completion, the Permittee shall certify that the work has been completed in accordance with the permit and the approved plans and that all unauthorized channel obstructions have been removed.
- 10) Final inspection of all work authorized by the permit will be made by the Commissioner to determine that the work has been performed in compliance with the permit.
- 11) Completed work shall be diligently maintained by the owner of the land to prevent any danger of obstruction of the stream, water course, easement or right-of-way bounded by channel lines by reason of erosion or the collapse or other impairment of the completed work.
- 12) The permit shall be subject to all applicable zoning regulations of the municipality within which the land to which the permit applies is located, to Workmen's Compensation Law, Disability Benefits Law and to all other regulations thereof applying to the construction of buildings and other structures. Every building permit or certificate of occupancy issued by any municipality shall be subject to the limitations and requirements imposed by or pursuant to the Westchester County Stream Control Law, with respect to any work covered by such permit or certificate. In event of any conflict, the more restrictive provision shall prevail.
- 13) The Commissioner reserves the right to revoke or cancel the permit at any time should the Permittee fail to comply with any of the terms, agreements, covenants and conditions of the permit.
- 14) The permit does not give any property rights, either in real property or material, or any exclusive privileges. It does not authorize any injury to public or private property, any invasion of property rights, any occupation of riparian or County property, or any infringement of State or local laws or regulations. Local and State permits and consents must be obtained when necessary.
- 15) The work must be completed on or before the stated completion date and shall be under the direction of a licensed professional engineer or licensed architect until it is completed.
- 16) The Permit is not in force and effect until the executed acceptance form is received by the Commissioner.
- 17) SPECIAL CONDITIONS for this Permit are set forth on the attached sheet.
- 18) The application, the Special conditions and the following approved plans are part of the permit: