

March 28, 2022

Adam R. Kaufman, AICP Director of Planning Town of North Castle 15 Bedford Road Armonk, NY 10504

Re: Mistis Properties Inc, 176 Virginia Road Site Plan Response Memo

Dear Mr. Kaufman,

This Memorandum responds to the outstanding Site Plan comments set forth in the Kellard Sessions Consulting, P.C. Memorandum, dated May 7, 2021. Kellard's comments are provided attached, Our responses are noted below in bold, blue and italics. Included here within:

General Comments

1. The plan requires a retaining wall along the rear and sides of the property with a maximum height of approximately 28 feet. For clarity, the proposed retaining wall shown on the grading plan shall include top of wall and bottom of wall elevations at each change in elevation to be coordinated with the elevation changes shown in section provided on Sheet S-7. The plan shall also illustrate the required grading of the slopes above the retaining wall necessary to maintain the proposed 1V:2H slope and include a detail of the armored slope referenced in the typical retaining wall section.

Plans were updated as per comment. (See Retaining Wall Section on Sheet S-7.)

2. Provide a site lighting and photometric plan for review and consideration by the Planning Board include specifications and details of all fixtures, poles, and pole bases.

Plans were updated as per comment. (See Site Lighting & Photometric Plans, Schedules, Specifications and Detail on Sheet E-1 to E-3.)

3. The proposed Frame and Cover Detail on Sheet S-5 shall require that manhole covers for sanitary sewers be stamped with "SEWER" on the casting.

Plans were updated as per comment. (See Frame and Cover Detail on Sheet S-2 & S-5.)

4. The sanitary manhole detail included on Sheet S-2 shall be revised to required two (2) coats of bitumastic sealant to be applied to the exterior.

Plans were updated as per comment. (See note in Sewer Manhole Detail on Sheet S-2.)

5. Please clarify the purpose and intent of the proposed pipe entering the proposed sewer manhole from the east. It is assumed that this is the sanitary connection for the bathroom in the building. Please specify the pipe size and material as necessary.

Plans were updated as per comment. (Pipe size and material added to plan.)

6. The hydrologic model demonstrates that the proposed stormwater management system will effectively mitigate the increased stormwater runoff generated by the project. However, it is suggested that the plan be revised to also connect the roof leaders of the proposed 2-bay warehouse (north building) and proposed Drain Inlet, DI-1. Doing so would allow the for the connection of Drain Inlet, DI-1, to the proposed Stormtrap System, rather than the current proposed 6"x6" PVC TEE connection, (which would not be acceptable) and eliminate the multiple connections at Drain Inlet, DI-2. This would also provide mitigation and treatment for all impervious surfaces. The outlet structure shall be labeled on plan Sheet S-2.

Plans were updated as per comment. (All roof and driveway area to DI-1 now directed to Stormtrap system. Outlet Structure labeled on S-2.)

7. The proposed outlet structure need clarification. The Detail on Sheet S-5 indicates a connection to the hydrodynamic separator: however, there is no indication of this on plan Sheet S-2. The detail also indicates outlet pipes of twelve (12) inch and four (4) inch diameter, while the plan illustrates a six (6) inch outlet pipe to Drain Inlet, DI-2. Please clarify. It appears that an additional structure will be required for the pipe transition or an alternate outlet structure design. The six (6) inch pipe connection should also be included in the hydrologic model. It currently appears to be undersized.

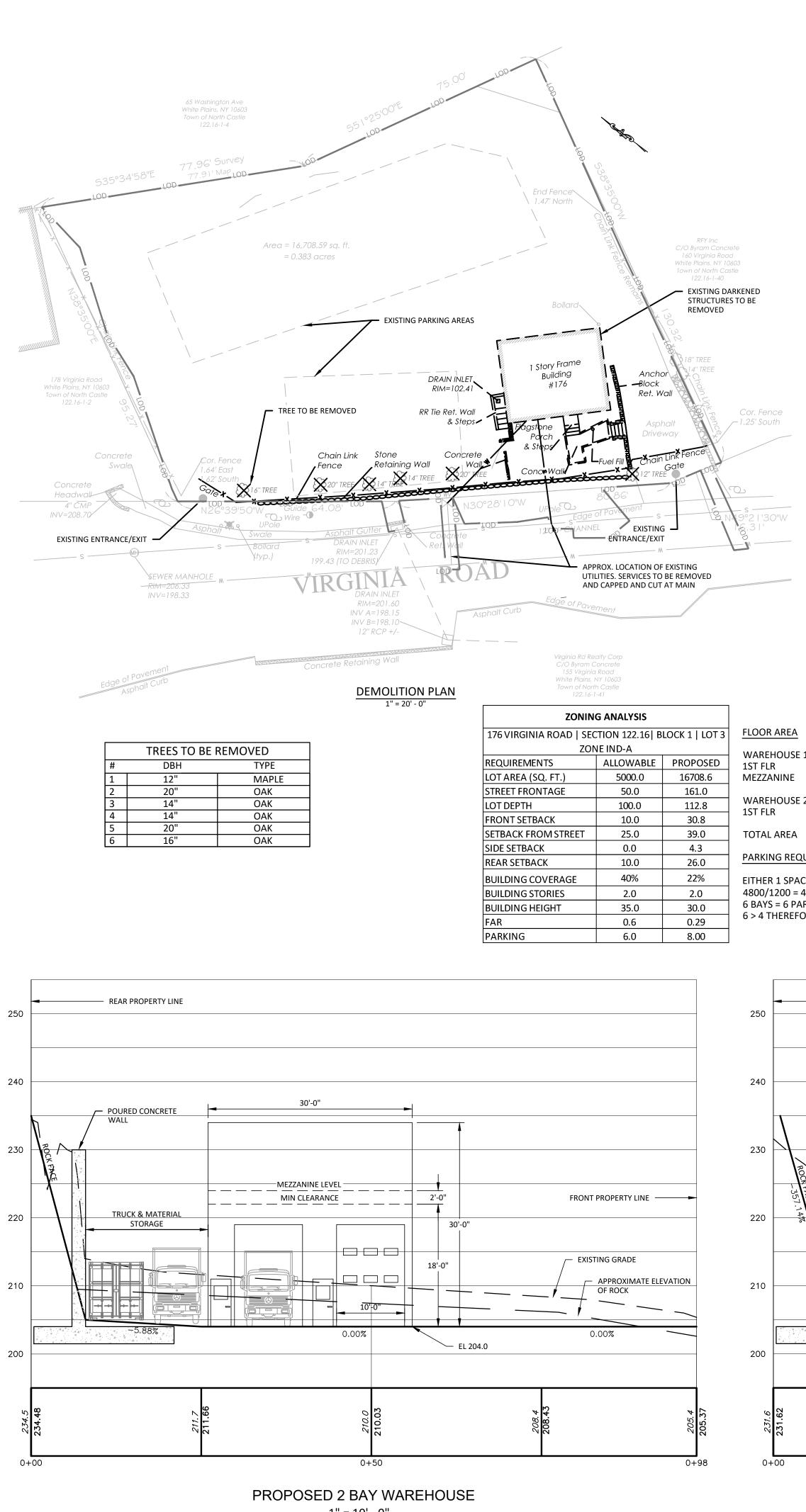
Plans were updated as per comment. (See plan on Sheet S-2 and Detail on Sheet S-5)

8. Drainage area maps shall be included in the Stormwater pollution Prevention Plan (SWPPP). The areas and cover types tributary to each sub-catchment shall be clarified.

Plans were updated as per comment. (Please see the attached SWPPP)

Sincerely, ARQ PC

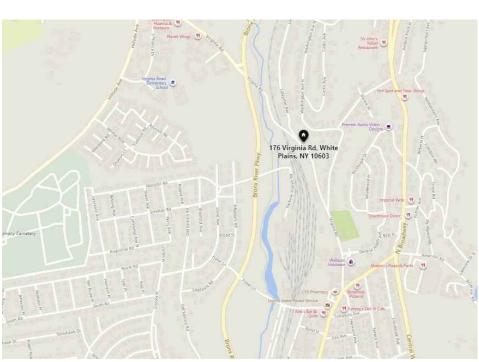
Paul Berté



1" = 10' - 0"

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LOCATION MAP N.T.S.

CELLANEOUS DATA:

TC MERRITTS LAND SURVEYORS 394 BEDFORD ROAD PLEASANTVILLE NY 10570 914-769-8003

FUSION ENGINEERING 600 NORTH BROADWAY SUITE 215 WHITE PLAINS NY 10603 914-358-5009

TOWN OF NORTH CASTLE **BRONX RIVER BASIN**

122.16

MANUEL YANEZ MISTIS PROPERTIES INC. **132 FULTON STREET** WHITE PLAINS, NY 10608 914-774-3625

NORTH WHITE PLAINS FD IND-A

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1. THIS PLAN WAS PREPARED TO COMPLY WITH THE APPLICATION REQUIREMENT FOR BUILDING PERMIT IN THE TOWN OF NORTH CASTLE.

GENERAL NOTES

EXISTING TOPOGRAPHIC AND SURVEY INFORMATION SHOWN HEREON PROVIDED BY A SURVEY PREPARED BY TC MERRITS LAND SURVEYORS ON AUGUST 30, 2018. PROPOSED GRADING SHALL DRAIN AWAY FROM THE PROPOSED STRUCTURE AT A MINIMUM SLOPE REQUIRED TO PROVIDE POSITIVE DRAINAGE AWAY FROM THE FOUNDATION.

- UNDERGROUND UTILITIES, FACILITIES AND STRUCTURES MAY NOT BE ALL SHOWN HEREON. THE LOCATIONS OF PORTIONS OF THE UNDERGROUND UTILITIES INDICATED HEREON WHERE OBTAIN FROM THE MAP REFERRED TO ABOVE AND FIELD MARK-OUTS BY THE UTILITY COMPANY PERSONNEL. THERE MAY BE OTHER UNDERGROUND UTILITIES TO WHICH THE LOCATIONS ARE CURRENTLY UNKNOWN. ANY PARTY UTILIZING THE INFORMATION AND DATA DEPICTED ON THIS PLAN SHALL CONTACT "DIG SAFELY. NEW YORK" AT PHONE NUMBER 1-800-962-7962 OR 811 A MINIMUM OF 48 HOURS PRIOR TO ANY CONSTRUCTION ACTIVITIES TO VERIFY THE LOCATION OF ANY AND ALL UNDERGROUND UTILITIES.
- CONTRACTOR IS SOLELY RESPONSIBLE FOR THE MEANS AND METHODS AND SEQUENCES OF CONSTRUCTION AND FOR THE SAFETY OF WORKERS AND OTHERS ON THE CONSTRUCTION SITE. THE CONTRACTOR SHALL LOCATE AND VERIFY THE SIZE, LOCATION, DEPTH AND INVERTS OF ANY AND ALL EXISTING UTILITIES PRIOR TO COMMENCING OPERATIONS.
- 5. PVC DRAIN PIPES SHALL BE SCHEDULE 40, SLOPES HAVING A MINIMUM SLOPE OF
- 6. NO SOIL STOCKPILES, CONSTRUCTION MATERIALS, AND NO EQUIPMENT SHALL BE STORED IN THE AREA OF THE PROPOSED (AND EXISTING) STORMWATER INFILTRATION PRACTICES.
- 7. THE CONTRACTOR SHALL PROVIDE A TRAINED INDIVIDUAL (CARRYING CARD/CERTIFICATION BY THE NYSDEC) TO BE PRESENT ON SITE AT ALL TIMES DURING SOIL DISTURBING ACTIVITIES.
- 8. DURING CONSTRUCTION, SEDIMENT AND EROSION CONTROLS SHALL BE IN ACCORDANCE WITH THE CURRENT DPW STANDARDS FOR SEDIMENT AND EROSION CONTROL. DPW RESERVES THE RIGHT TO ORDER ADDITIONAL SEDIMENT CONTROL PRACTICES INSTALLED DURING CONSTRUCTION.
- 9. OWNER/OWNER'S REPRESENTATIVE SHALL CONTACT TOWN BUILDING DEPARTMENT TO INSPECT SEDIMENT AND EROSION CONTROL PRACTICES PRIOR TO START OF CONSTRUCTION. ANY DESIGN CHANGES TO THE STORMWATER SYSTEM DURING CONSTRUCTION DUE TO SHALLOW GROUNDWATER, ROCK, ETC. MUST BE RESUBMITTED TO THE TOWN ENGINEER FOR APPROVAL PRIOR TO CONSTRUCTION START.
- 10. STORMWATER SYSTEM MUST BE INSPECTED AND CERTIFIED BY A PROFESSIONAL ENGINEER
- 11. UNDERGROUND UTILITIES (ELECTRIC, GAS, & COMMUNICATION) TO BE FIELD LOCATED BY CONTRACTOR.
- 12. ROOF LEADER CONNECTIONS SHOWN HEREON ARE APPROXIMATE AND SHALL BE COORDINATED WITH GUTTER/DOWNSPOUT INSTALLATION. DRAINAGE SHALL BE INSTALLED TO PROVIDE A CONNECTION TO ALL REQUIRED ROOF LEADER DOWNSPOUTS.
- 13. NO SOIL STOCKPILING ALLOWED ON SITE. ALL EXCAVATED SOIL TO BE IMMEDIATELY TRUCKED OFF SITE.

EROSION CONTROL NOTES:

- 1. TREE PROTECTION MEASURES SHALL BE INSTALLED PRIOR TO THE COMMENCEMENT OF WORK.
- 2. SUGGESTED LOCATIONS OF EROSION AND SEDIMENT CONTROL MEASURES ARE SHOWN HEREON. PLACEMENT OF BEST MANAGEMENT PRACTICES TO MANAGE SOIL EROSION AND POLLUTION PREVENTION ON SITE MAY BE MODIFIED IN THE FIELD AFTER CONSULTATION WITH THE APPROPRIATE REGULATORY AGENCY HAVING JURISDICTION PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. PRACTICES MUST BE PROPERLY INSTALLED PRIOR TO START OF CONSTRUCTION AND SHALL BE INSPECTED AND MAINTAINED AS NEEDED TO INSURE THE CONTROLS ARE FUNCTIONING AS DESIGNED. CONTRACTOR SHALL TAKE CARE TO VISUALLY INSPECT CONTROLS, ESPECIALLY PRIOR TO PRECIPITATION EVENTS AND MAKE ANY CORRECTIONS OR PROVIDE ADDITIONAL MEASURES AS NECESSARY TO TRY TO PREVENT SEDIMENT OR POLLUTANTS FROM LEAVING THE SITE.
- 3. CONSTRUCTION ACCESS TO EXPOSED/GRADED SOILS SHALL BE DEFINED BY THE PLACEMENT OF AN ANTI-TRACKING MANAGEMENT PRACTICE PRIOR TO THE START OF CONSTRUCTION. TRACK OUT ONTO PUBLIC STREETS SHALL BE SWEPT DAILY AND BEFORE PRECIPITATION EVENTS.
- 4. DISTURBED SOILS SHALL BE TEMPORARILY STABILIZED WITHIN 14 DAYS.
- 5. ALL EROSION CONTROL MEASURES SHALL BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH NYSDEC 'NEW YORK STATE STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL.
- 6. THE ENGINEER MAY AT HIS DISCRETION REQUIRE ADDITIONAL EROSION CONTROL MEASURES THROUGHOUT CONSTRUCTION TO MITIGATE UNFORESEEN EROSION AND SILTATION.
- 7. PROVISIONS SHALL BE MADE TO PREVENT SURFACE WATER FROM DAMAGING THE CUT FACE OF EXCAVATIONS OR THE SLOPING SURFACES FILLS.
- 8. TREES, ROOT SYSTEMS AND OTHER VEGETATION REMOVED FOR CONSTRUCTION PURPOSES SHALL BE CHIPPED OR REMOVED FROM SITE. NO ON-SITE BURIAL OR BURNING SHALL BE PERMITTED. 9. DURING GRADING OPERATIONS, APPROPRIATE MEASURES FOR DUST CONTROL SHALL BE EXERCISED.
- 10. ALL FILLS SHALL BE COMPACTED TO PROVIDE STABILITY OF MATERIAL AND TO PREVENT UNDESIRABLE SETTLEMENT.
- 11. AFTER FINAL GRADES ARE ESTABLISHED, DISTURBED AREAS SHALL BE COVERED WITH FOUR INCHES OF TOPSOIL AND SEEDED; LANDSCAPE AREAS SHALL BE MULCHED.
- 12. FOR DEWATERING ACTIVITIES: A DEWATERING PUMP SHALL BE LOCATED IN A PERFORATED TUB SURROUNDED BY FILTER FABRIC AND STONE (OR APPROVED ALTERNATIVE). CLEAN DISCHARGE SHOULD BE DIRECTED TO ONSITE DRAINAGE APPURTENANCES TO MINIMIZE EROSION OF SOILS. DISCHARGE WITH SUSPENDED SEDIMENT SHALL BE CONNECTED TO A SEDIMENT BAG ON UNDISTURBED GROUND IN A LOCATION WHERE THE DISCHARGE WILL NOT CAUSE EROSION OR FLOW OVER EXPOSED SOILS.



100 EXECUTIVE BLVD. SUITE 204 OSSINING, NY 10562 PHONE: (914) 944-3377 FAX: (866) 567-6240

JORGE B. HERNANDEZ R.A. A.I.A. LICENSE NUMBER: 030424-1 CERTIFICATE NUMBER: 0973256

PAUL A. BERTE, P.E 100 EXECUTIVE BLVD. SUITE 204 OSSINING, NY 10562

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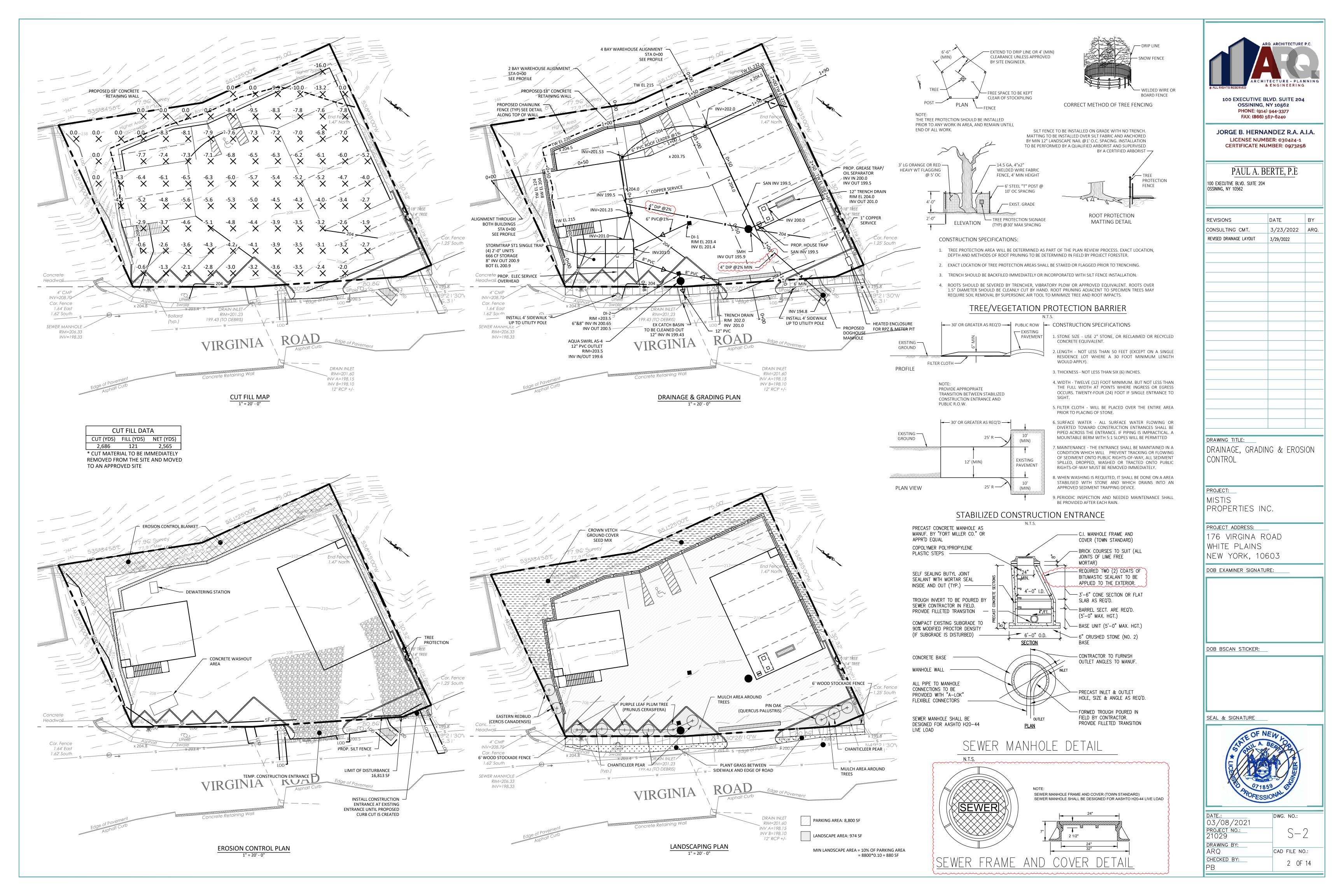
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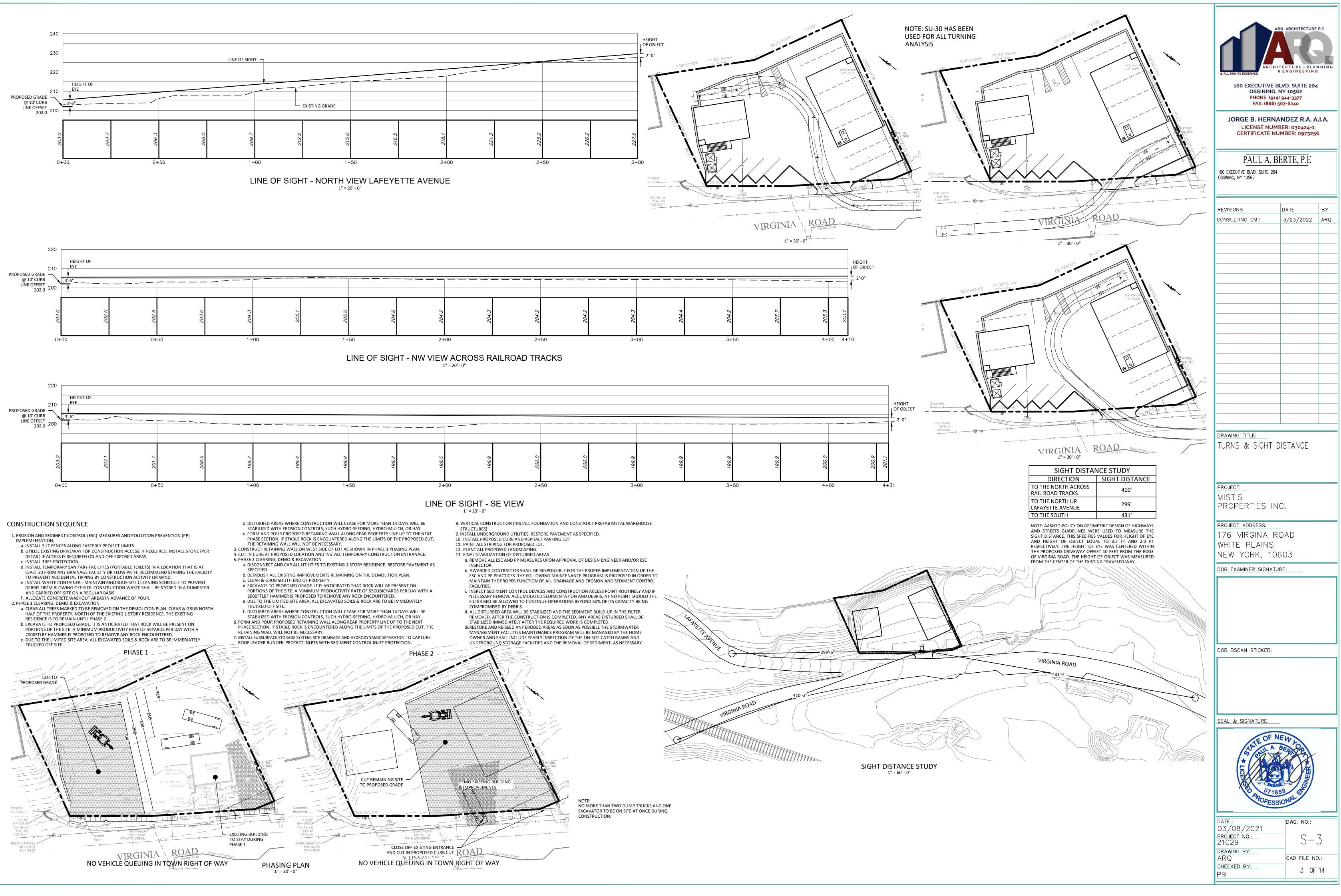
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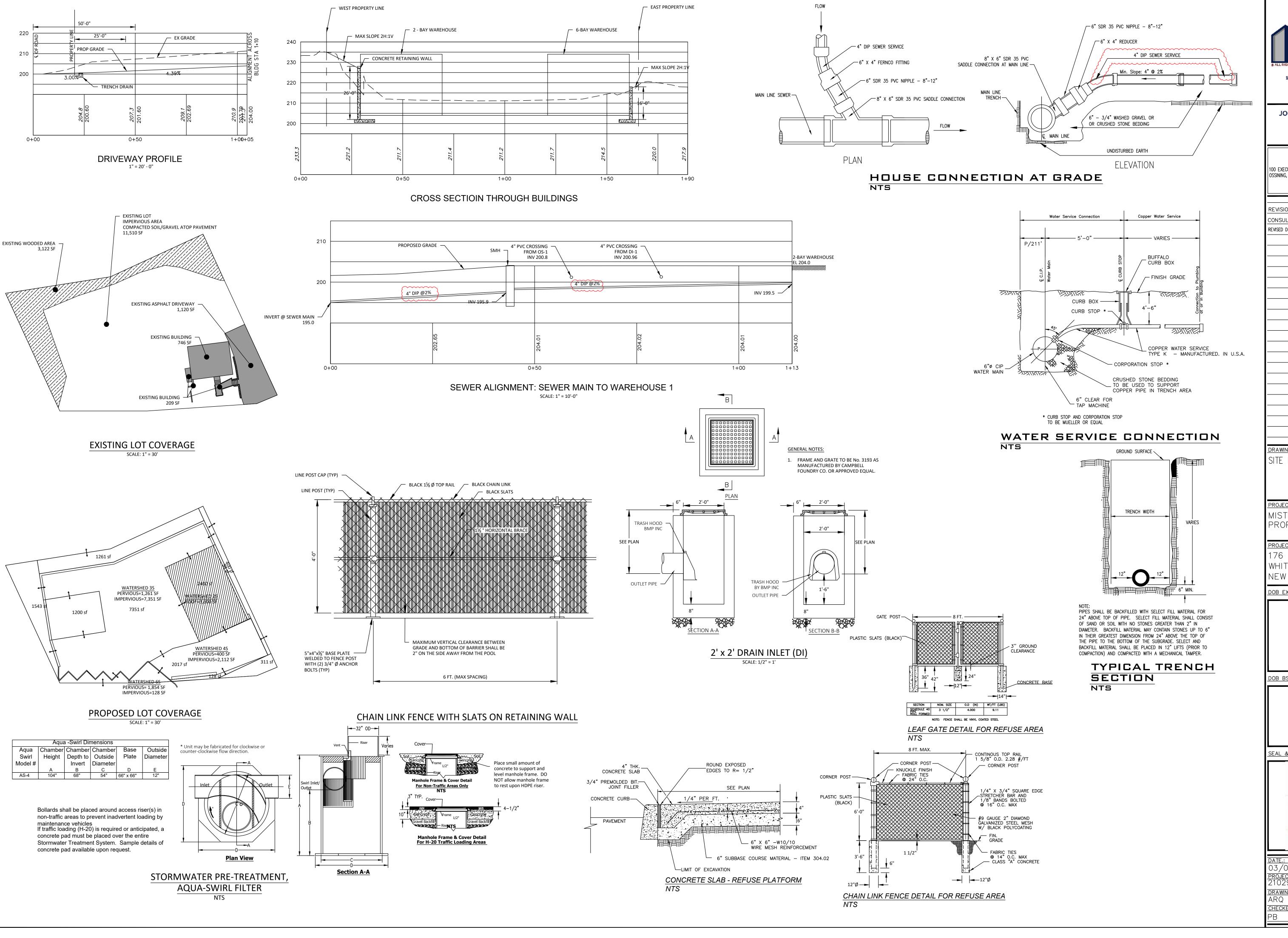
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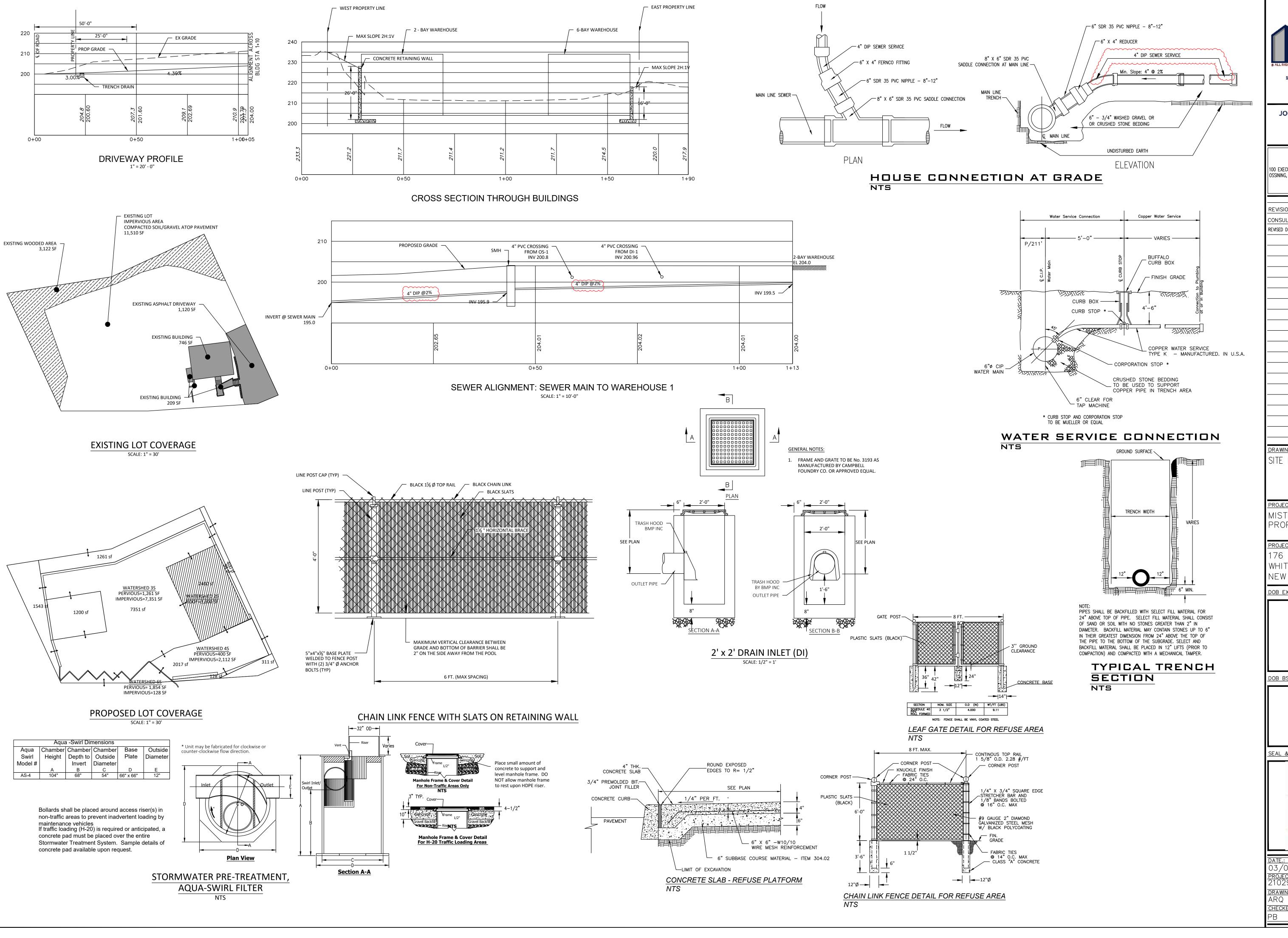
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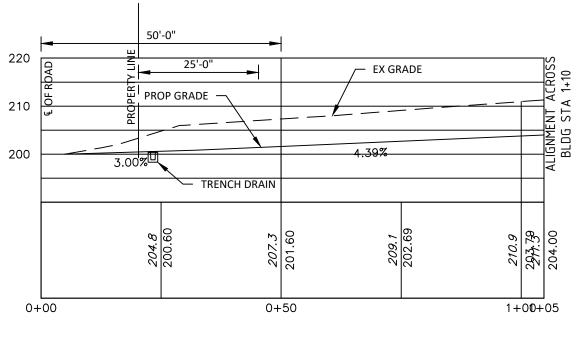
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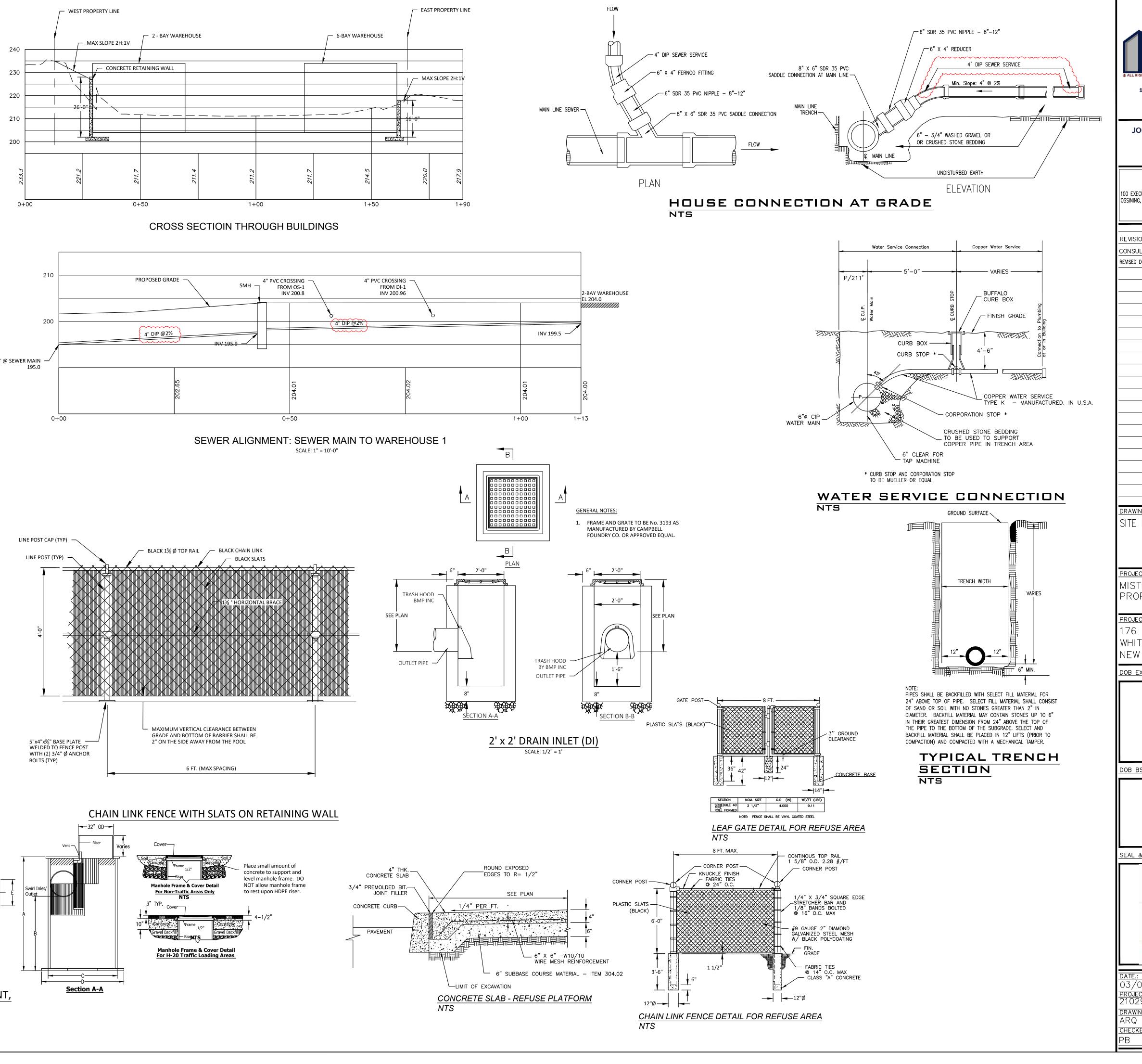




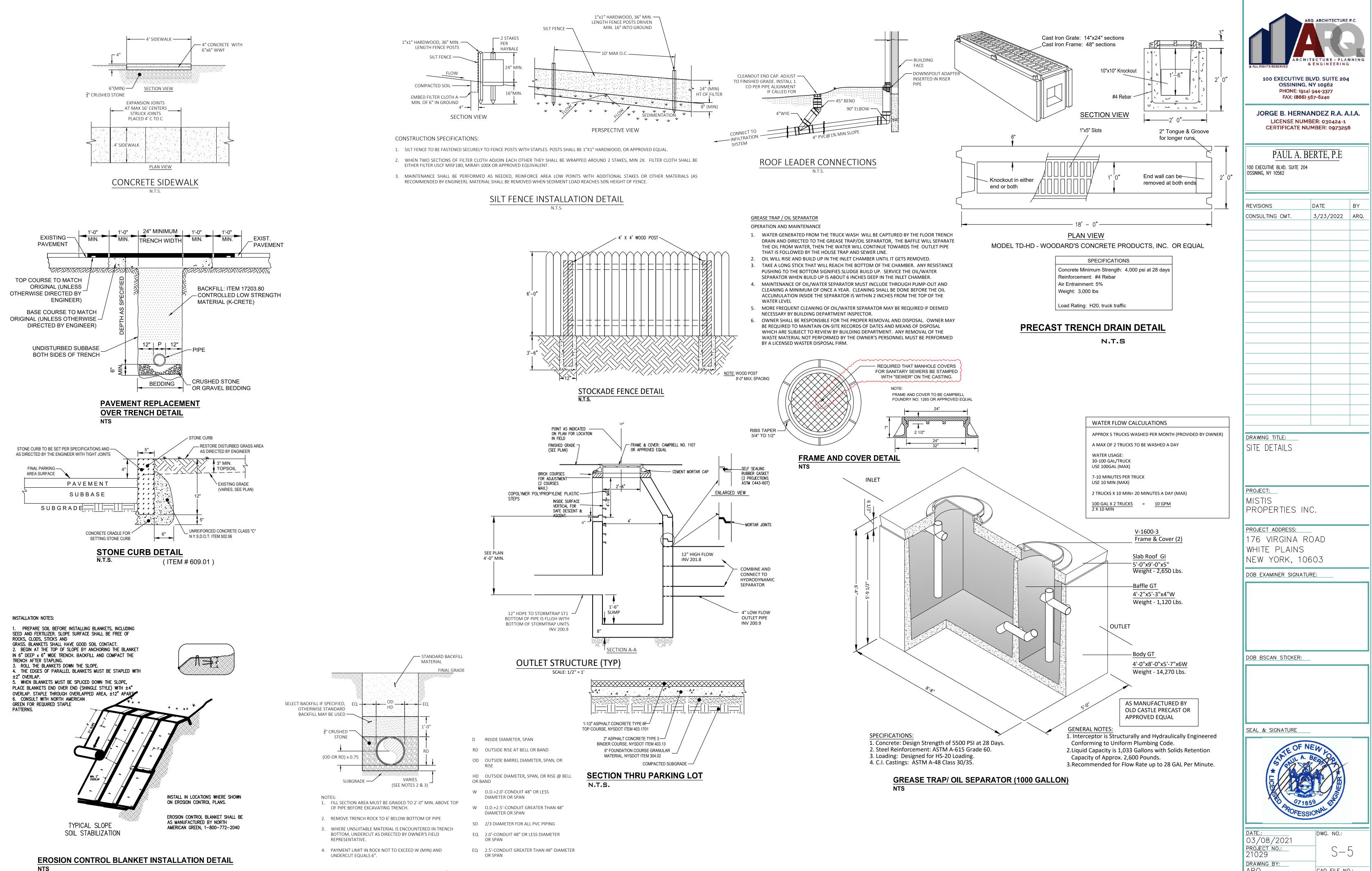








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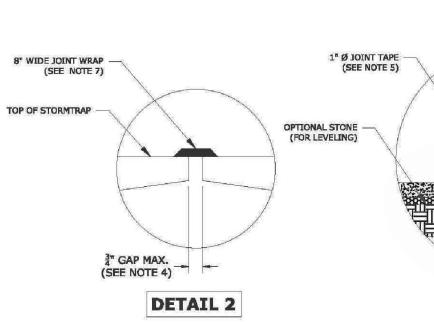


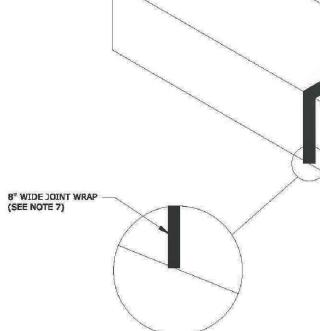
PIPE TRENCH EXCAVATION & BACKFILL

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SINGLE TRAP MODULE INSTALLATION





DETAIL 4

STORMTRAP INSTALLATION SPECIFICATIONS

- STORMTRAP SHALL BE INSTALLED IN ACCORDANCE WITH ASTM C891, STANDARD FOR INSTALLATION OF UNDERGROUND PRECAST
- CONCRETE UTILITY STRUCTURES, THE FOLLOWING ADDITIONS AND/OR EXCEPTIONS SHALL APPLY:

1. STORMTRAP MODULES SHALL BE MANUFACTURED AND INSTALLED ACCORDING TO SHOP DRAWINGS APPROVED BY THE

3. ALL DIMENSIONS AND SOIL CONDITIONS, INCLUDING BUT NOT LIMITED TO GROUNDWATER AND SOIL BEARING

CAPACITY ARE REQUIRED TO BE VERIFIED IN THE FIELD BY OTHERS PRIOR TO STORMTRAP INSTALLATION.

2. COVER RANGE: MIN. 1.08' MAX. 10.00' (CONSULT STORMTRAP FOR ADDITIONAL COVER OPTIONS).

INSTALLING CONTRACTOR AND ENGINEER OF RECORD. THE SHOP DRAWINGS SHALL INDICATE SIZE AND LOCATION OF ROOF OPENINGS AND INLET/ OUTLET PIPE TYPES, SIZES, INVERT ELEVATIONS AND SIZE OF OPENINGS.

NOTES:

4.

5.

NET ALLOWABLE SOIL PRESSURE AS INDICATED ON SHEET 1.0.

ALL DIMENSIONS TO BE VERIFIED IN THE FIELD BY OTHERS.

STORMTRAP STRUCTURAL DESIGN CRITERIA

SOIL CONDITIONS TO BE VERIFIED ON SITE BY OTHERS.

REBAR: ASTM A-615 GRADE 60. BLACK BAR.

9. SEE SHEET 3.0 FOR INSTALLATION SPECIFICATIONS.

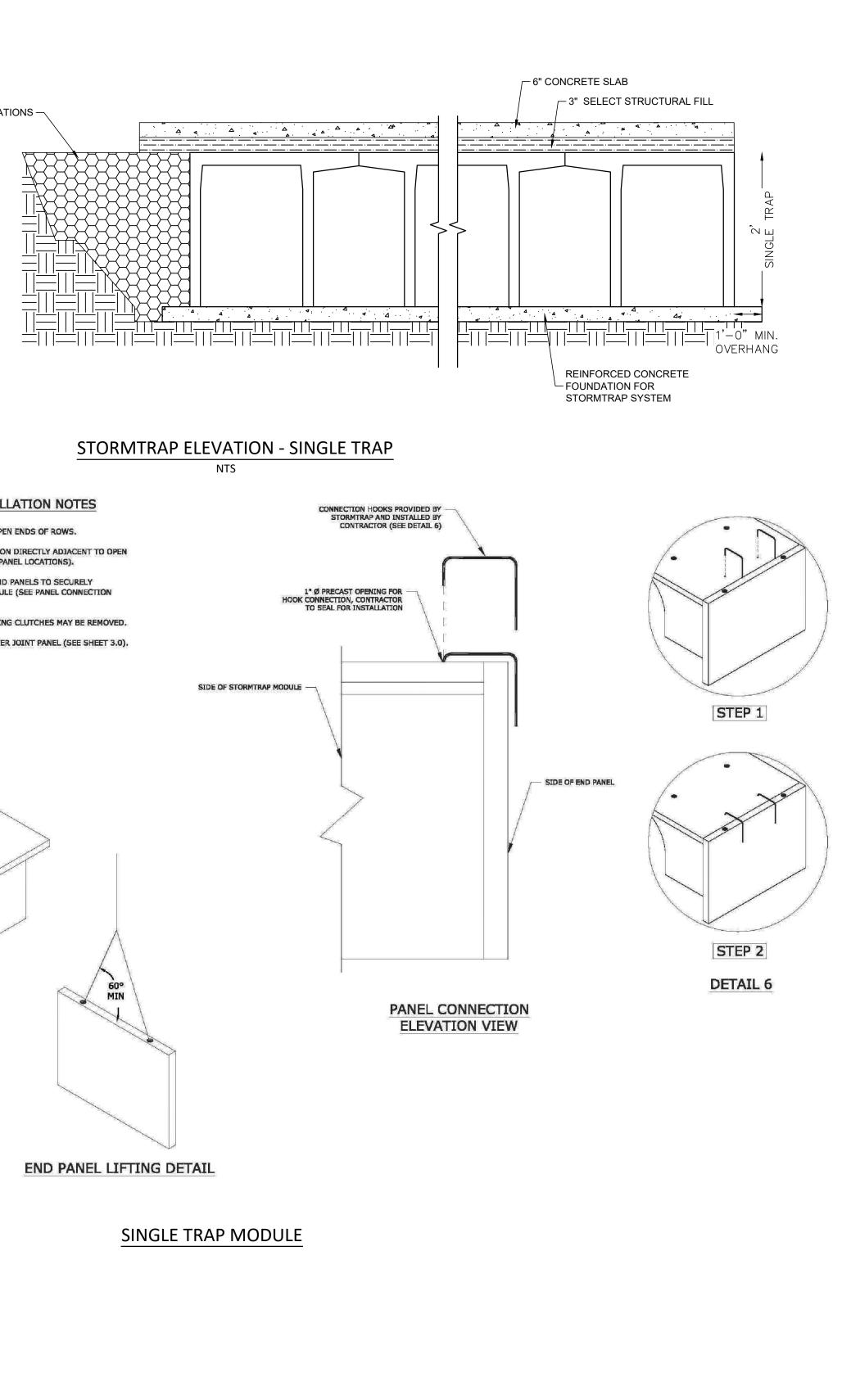
16'-0" TO 24'-0" MAX APART.

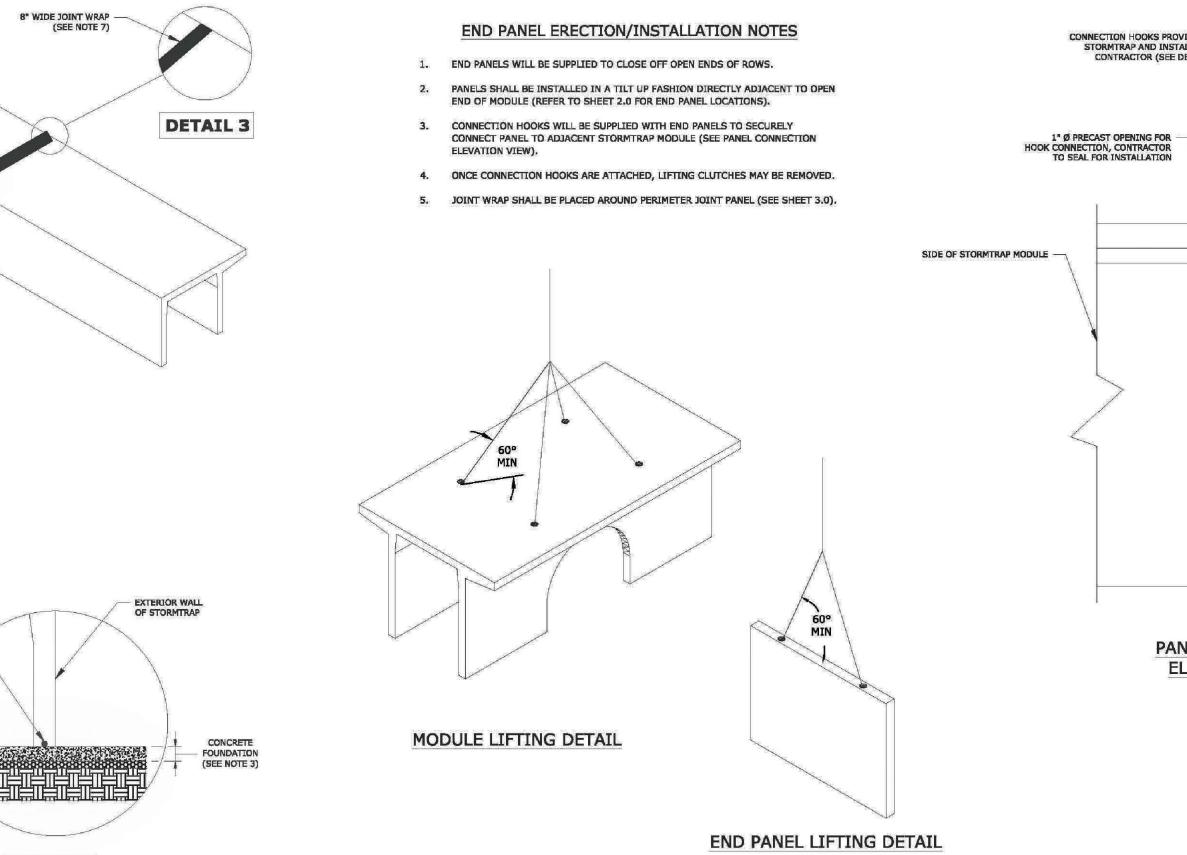
- 2. IT IS THE RESPONSIBILITY OF THE INSTALLING CONTRACTOR TO ENSURE THAT PROPER/ADEQUATE EQUIPMENT IS USED TO SET/INSTALL THE MODULES.
- 3. STORMTRAP MODULES SHALL BE PLACED ON A LEVEL CONCRETE FOUNDATION (SEE SHEET 2.1) WITH A 1'-0" OVERHANG ON ALL SIDES THAT SHALL BE POURED IN PLACE BY INSTALLING CONTRACTOR. A QUALIFIED GEOTECHNICAL ENGINEER WILL BE EMPLOYED, BY OWNER, TO PROVIDE ASSISTANCE IN EVALUATING THE EXISTING SOIL CONDITIONS. TO ENSURE THAT HE SOIL BEARING PRESSURE MEET OR EXCEED THE STRUCTURAL DESIGN LOADING CRITERIA AS SPECIFIED ON SHEET 1.0.
- 4. THE STORMTRAP MODULES SHALL BE PLACED SUCH THAT THE MAXIMUM SPACE BETWEEN ADJACENT MODULES DOES NOT EXCEED ₹" (SEE DETAIL 2). IF THE SPACE EXCEEDS \$", THE MODULES SHALL BE RESET WITH APPROPRIATE ADJUSTMENT MADE TO LINE AND GRADE TO BRING THE SPACE INTO SPECIFICATION.
- 5. THE PERIMETER HORIZONTAL JOINT BETWEEN THE STORMTRAP MODULES AND THE CONCRETE FOUNDATION SHALL BE SEALED TO THE FOUNDATION WITH PRE-FORMED MASTIC JOINT SEALER ACCORDING TO ASTM C891, 8.8 AND 8.12 (SEE DETAIL 1). MASTIC JOINT TAPE DOES NOT PROVIDE A WATERTIGHT SEAL. THE SOLE PURPOSE OF THE JOINT TAPE IS TO PROVIDE A SILT AND SOIL TIGHT SYSTEM.
- 6. STORMTRAP MODULES ARE NOT WATERTIGHT. IF A WATERTIGHT SOLUTION IS REQUIRED, CONTACT STORMTRAP FOR RECOMMENDATIONS. THE WATERTIGHT APPLICATION IS TO BE PROVIDED AND IMPLEMENTED BY THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE TO ENSURE THAT THE SELECTED WATERTIGHT SOLUTION PERFORMS AS SPECIFIED BY THE MANUFACTURER. CONTACT STORMTRAP IF A WATERTIGHT APPLICATION IS REQUIRED.
- ALL EXTERIOR JOINTS BETWEEN ADJACENT STORMTRAP MODULES SHALL BE SEALED WITH 8" WIDE PRE-FORMED, COLD-APPLIED, SELF-ADHERING ELASTOMERIC RESIN, BONDED TO A WOVEN, HIGHLY PUNCTURE RESISTANT POLYMER WRAP, CONFORMING TO ASTM C891 AND SHALL BE INTEGRATED WITH PRIMER SEALANT AS APPROVED BY STORMTRAP (SEE DETAILS 3 & 4). THE JOINT WRAP DOES NOT PROVIDE A WATERTIGHT SEAL. THE SOLE PURPOSE OF THE JOINT WRAP IS TO PROVIDE A SILT AND SOLL TIGHT SYSTEM. THE ADHESIVE EXTERIOR JOINT WRAP SHALL BE INSTALLED ACCORDING TO THE FOLLOWING INSTALLATION INSTRUCTIONS:
- 7.1. USE A BRUSH OR WET CLOTH TO THOROUGHLY CLEAN THE OUTSIDE SURFACE AT THE POINT WHERE JOINT WRAP IS TO BE APPLIED.
- 7.2. A RELEASE PAPER PROTECTS THE ADHESIVE SIDE OF THE JOINT WRAP. PLACE THE ADHESIVE TAPE (ADHESIVE SIDE DOWN) AROUND THE STRUCTURE, REMOVING THE RELEASE PAPER AS YOU GO. PRESS THE JOINT WRAP FIRMLY AGAINST THE
- STORMTRAP MODULE SURFACE WHEN APPLYING. 8. IF THE CONTRACTOR NEEDS TO CANCEL ANY SHIPMENTS, THEY MUST DO SO 48 HOURS PRIOR TO THEIR SCHEDULED ARRIVAL AT THE JOB SITE. IF CANCELED AFTER THAT TIME, PLEASE CONTACT THE PROJECT MANAGER.
- 9. IF THE STORMTRAP MODULE(S) IS DAMAGED IN ANY WAY PRIOR, DURING, OR AFTER INSTALL, STORMTRAP MUST BE CONTACTED IMMEDIATELY TO ASSESS THE DAMAGE AND DETERMINE WHETHER OR NOT THE MODULE(S) WILL NEED TO BE REPLACED. IF ANY MODULE ARRIVES AT THE JOBSITE DAMAGED DO NOT UNLOAD IT; CONTACT STORMTRAP IMMEDIATELY. ANY DAMAGE NOT REPORTED BEFORE THE TRUCK IS UNLOADED WILL BE THE CONTRACTOR'S RESPONSIBILITY.
- 10. STORMTRAP MODULES CANNOT BE ALTERED IN ANY WAY AFTER MANUFACTURING WITHOUT WRITTEN CONSENT FROM STORMTRAP.

- CONCRETE STRENGTH @ 28 DAYS, 5%-8% ENTRAINED AIR, 4" MAX. SLUMP. PAD REINFORCEMENT CONTROL JOINT DETAIL DIMENSION OF FOUNDATION MUST HAVE 1'-0" OVERHANG BEYOND EXTERNAL FACE OF MODULE. TOP OF FOUNDATION -6. DIMENSION OF STORMTRAP SYSTEM ALLOW FOR A 3/4" GAP BETWEEN EACH MODULE. THE CONTROL JOINTS SHALL BE BETWEEN (IF REQUIRED BY ENGINEER OF RECORD) SLAB THICKNESS-
- MAXIMUM SYSTEM SLAE COVER 6" - 12" >1'-0" - 2'-0" >2'-0" - 3'-0" >3'-0" - 4'-0" >4'-0" - 5'-0" >5'-0" - 6'-0" >6'-0" - 7'-0" >7'-0" - 8'-0" REBAR PLACED IN >8'-0" - 9'-0" CENTER OF SLAB >9'-0" - 10'-0" STORMTRAP FOUNDATION DETAIL
- CONTROL JOINT TO BE CUT TOP OF FOUNDATION -SLAB IS POURED SLAB THICKNESS
- - INTO SLAB WITHIN 8 HOURS AFTER

| B THICKNESS | CONCRETE STRENGTH | REINFORCEMENT (BOTH DIRECTIONS) |
|-------------|-------------------|------------------------------------|
| 0'-8" | 4000 PSI | #4 @ 18" O.C. |
| 0'-8" | 4000 PSI | #4 @ 16" O.C. |
| 0'-8" | 4000 PSI | #4 @ 12" O.C. |
| 0'-8" | 4000 PSI | #4 @ 12" O.C. |
| 0'-8" | 4000 PSI | #5 @ 18" O.C. |
| 0'-8" | 4000 PSI | #5 @ 16" O.C. |
| 0'-8" | 4000 PSI | #5 @ 16" O.C. |
| 0'-9" | 4000 PSI | #5 @ 12" O.C. |
| 0'-10" | 4000 PSI | #5 @ 12" O.C. |
| 0'-10" | 4500 PSI | #5 @ 12" O.C. |

SEE BACKFILL SPECIFICATIONS





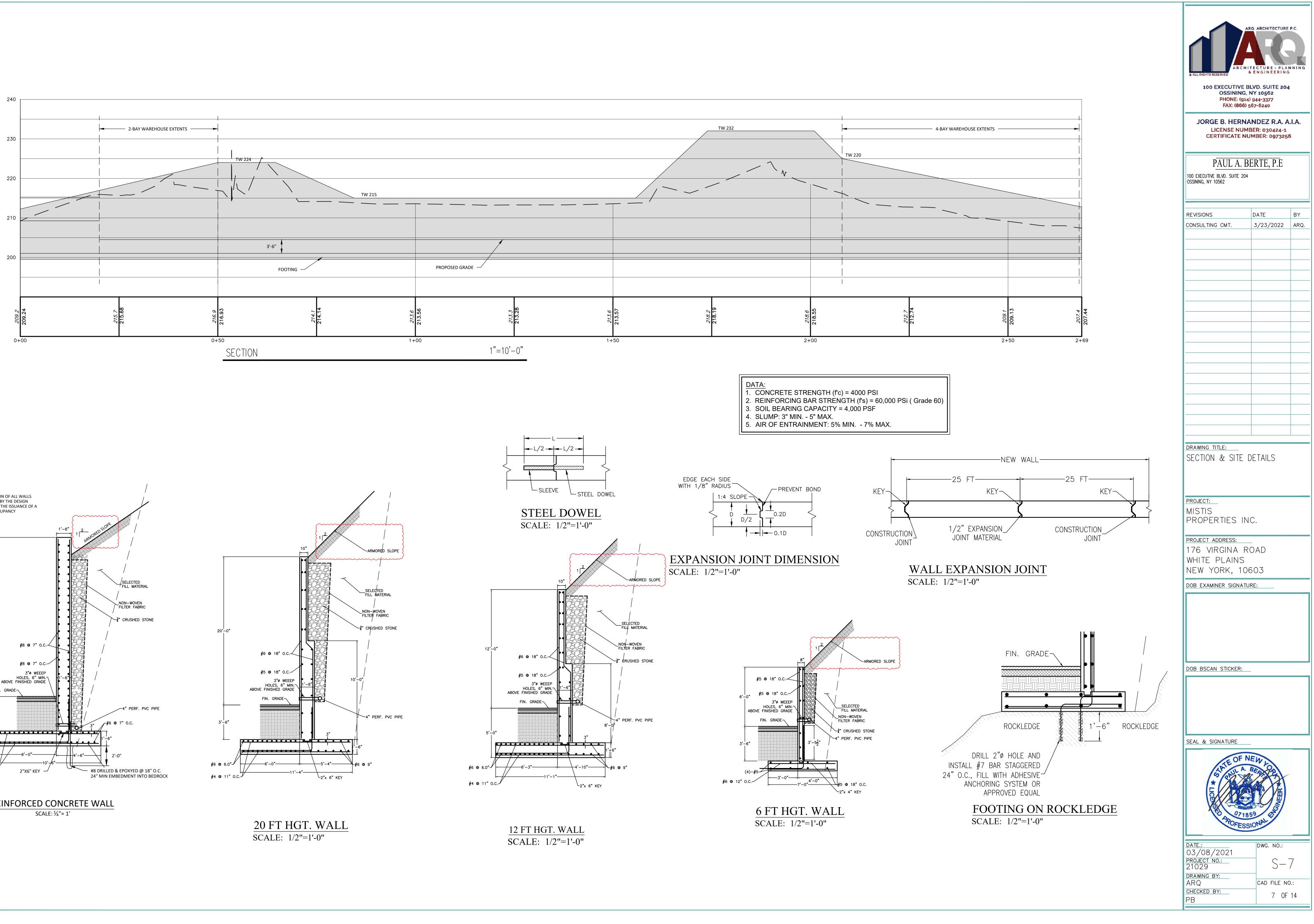
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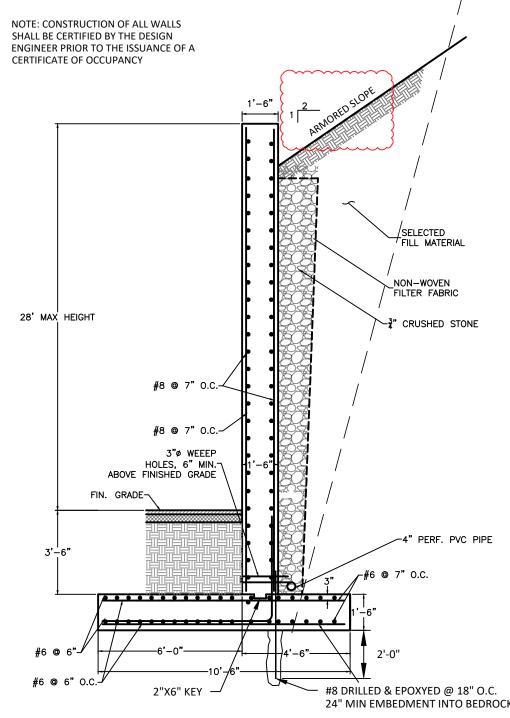
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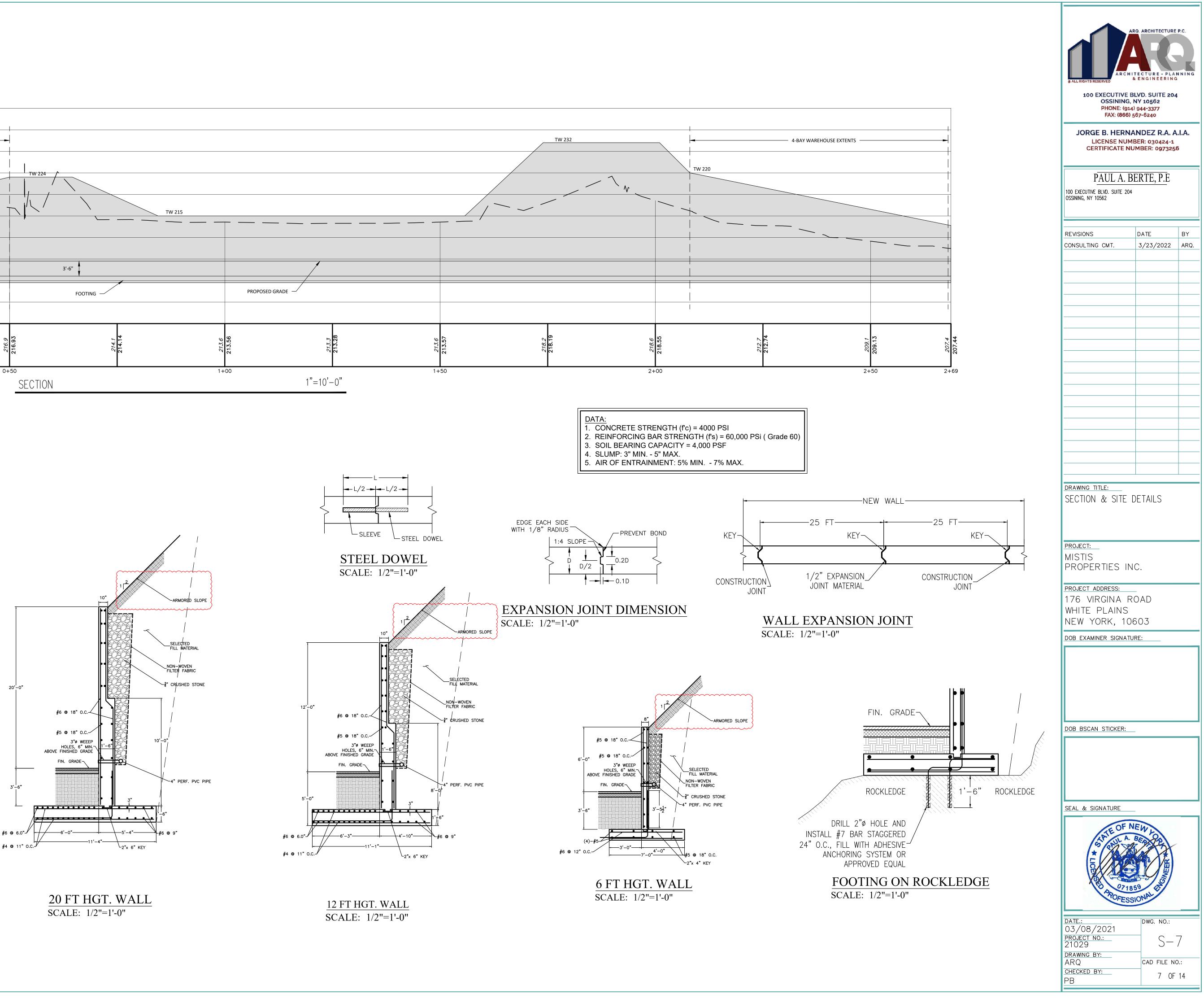
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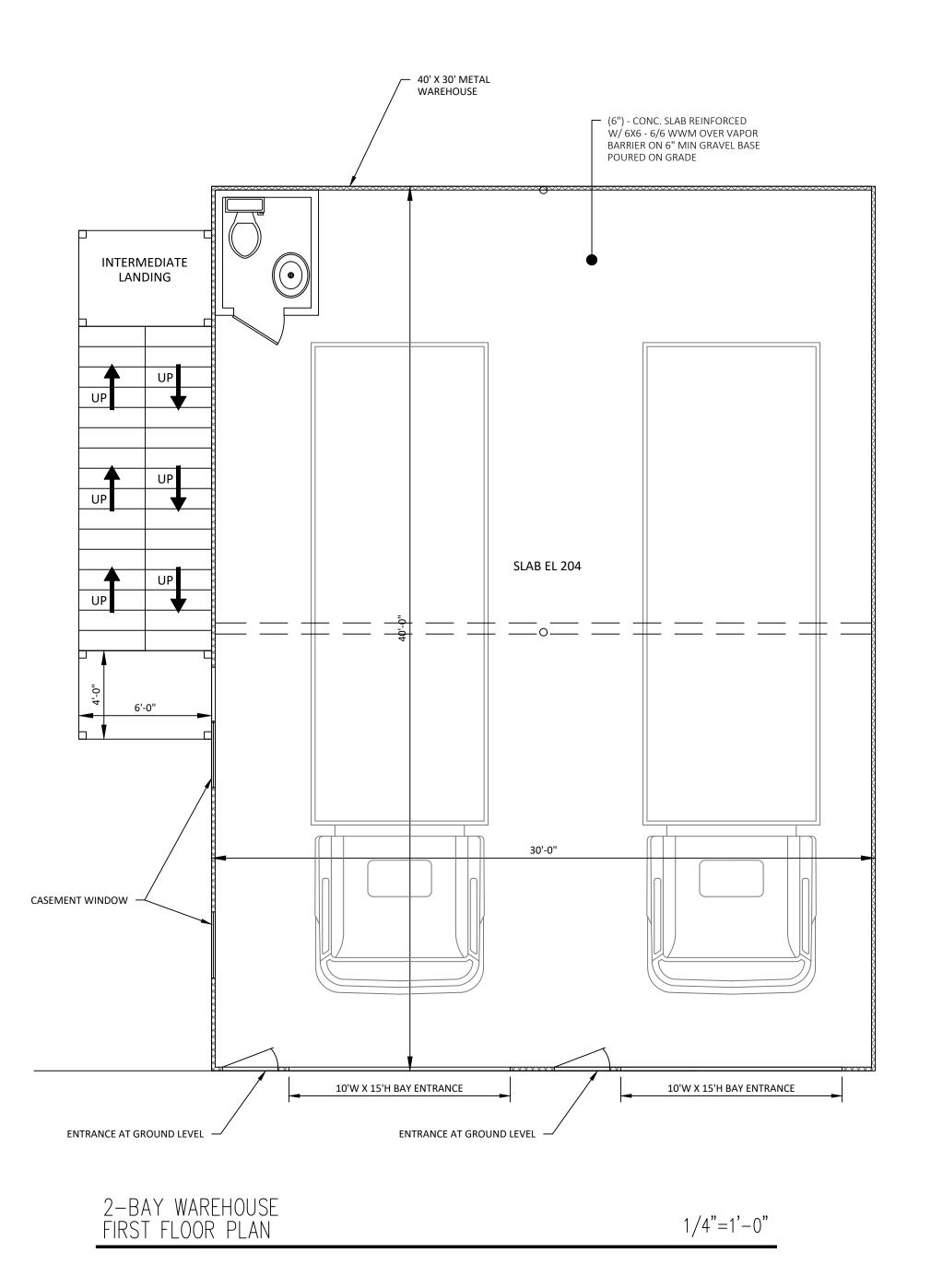
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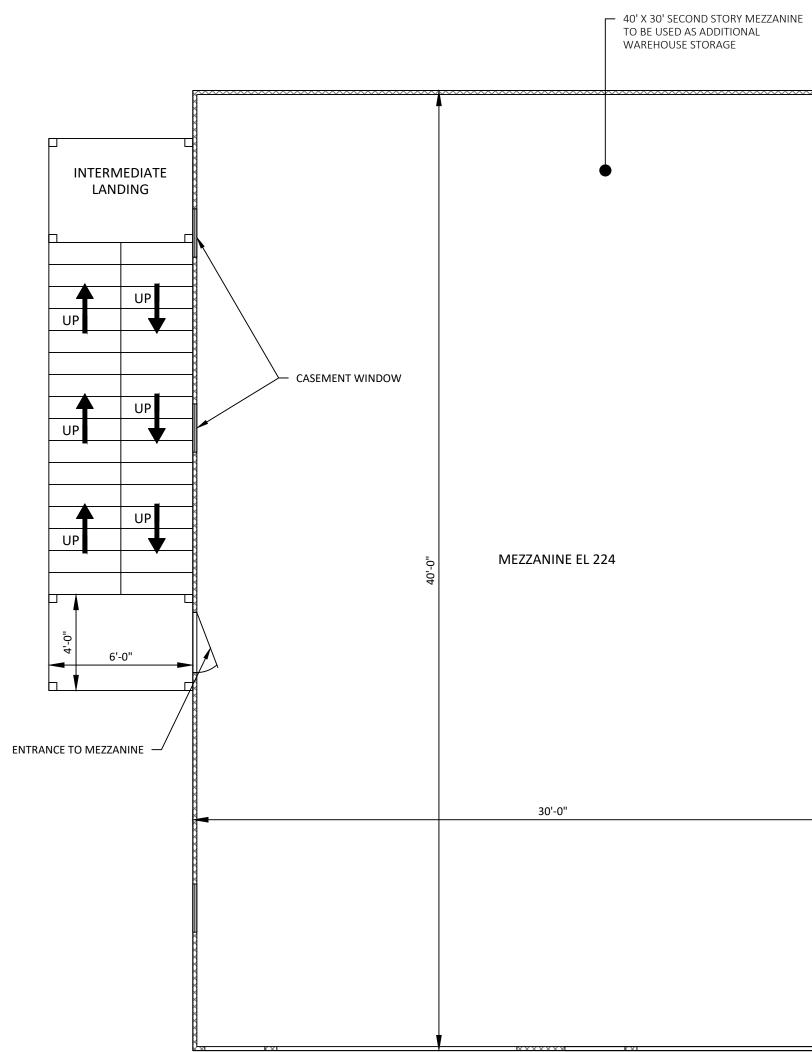








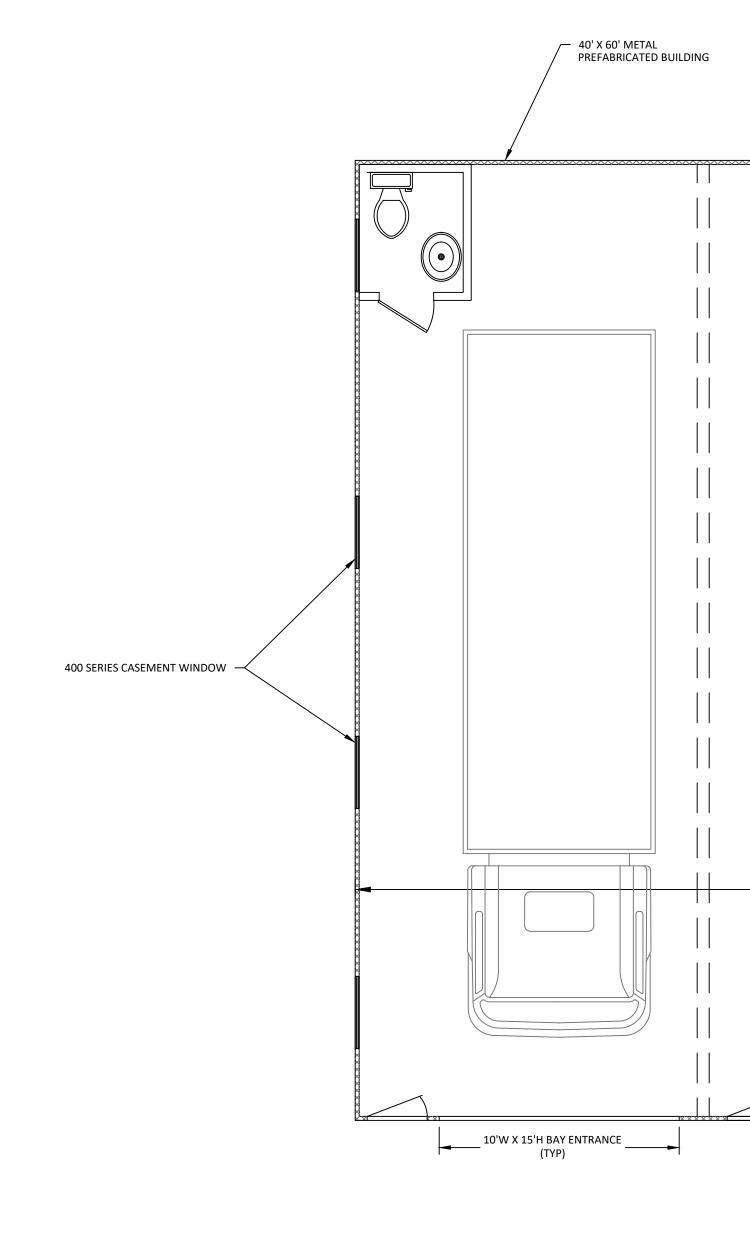




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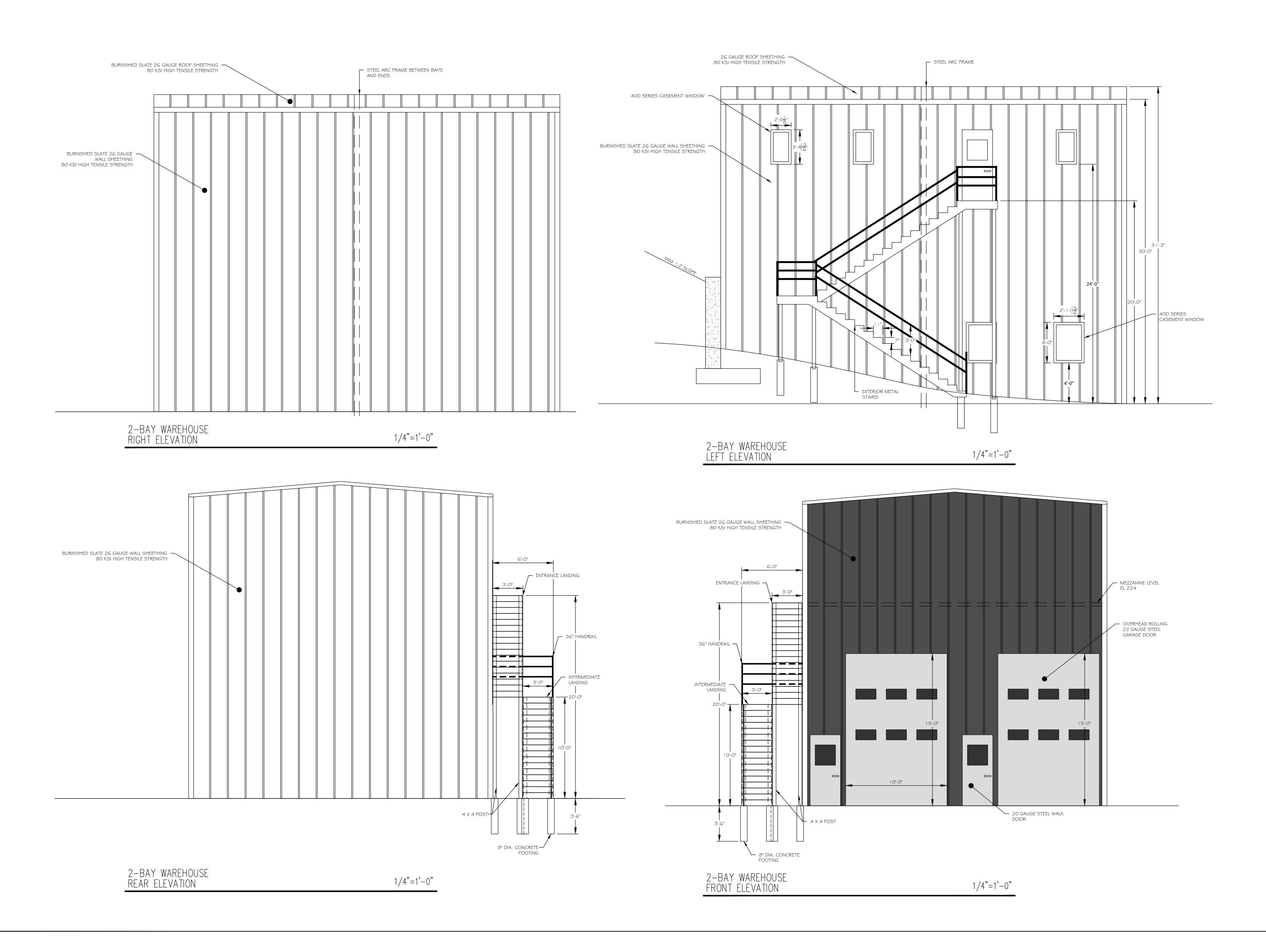


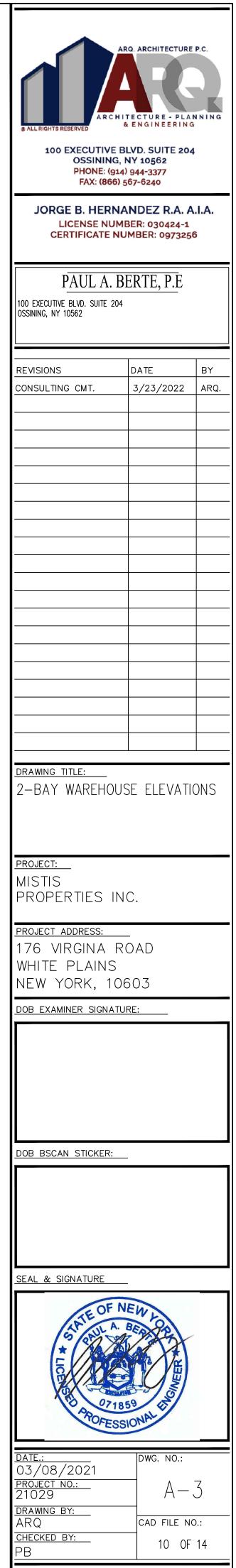
4–BAY WAREHOUSE FLOOR PLAN

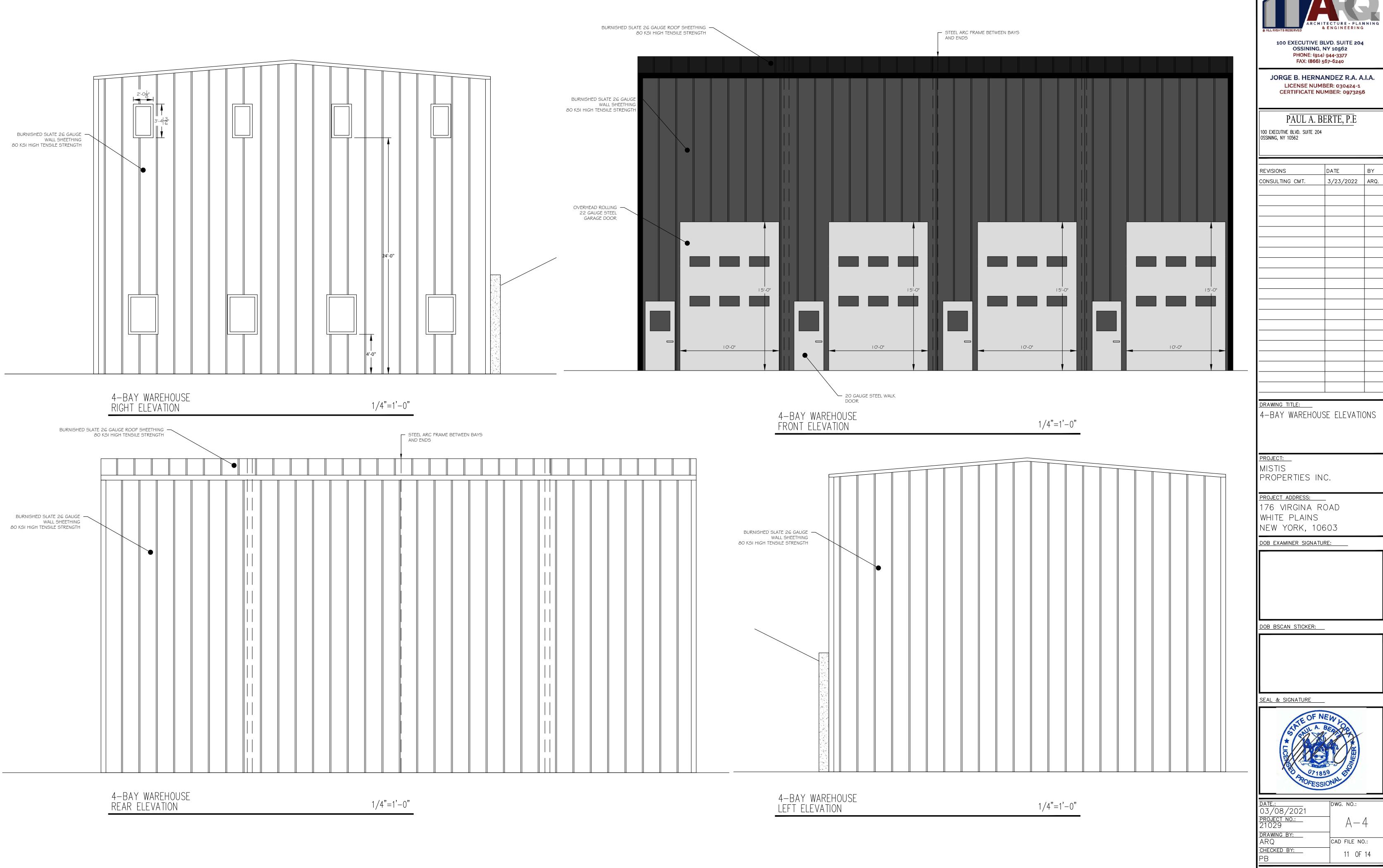
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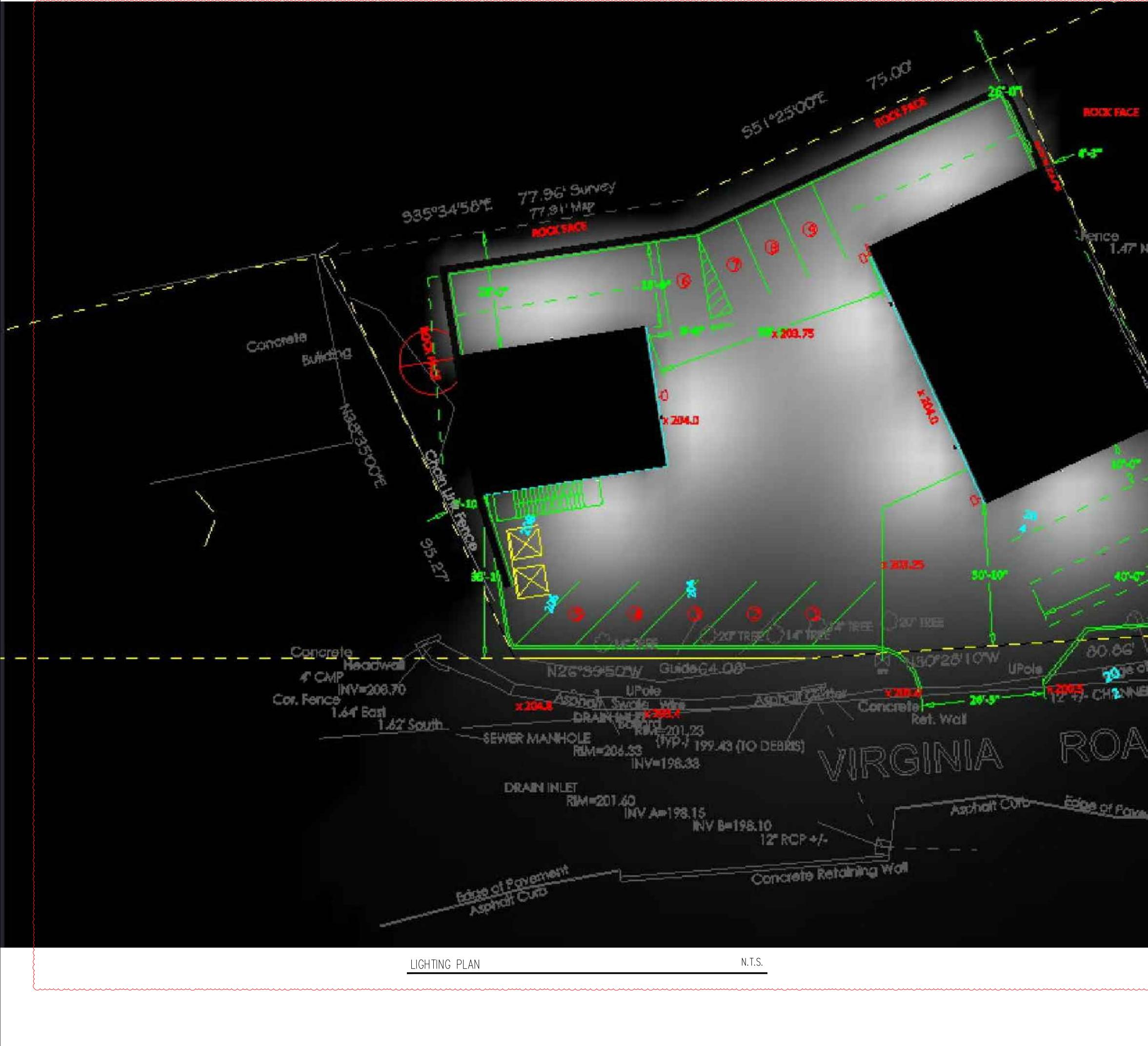
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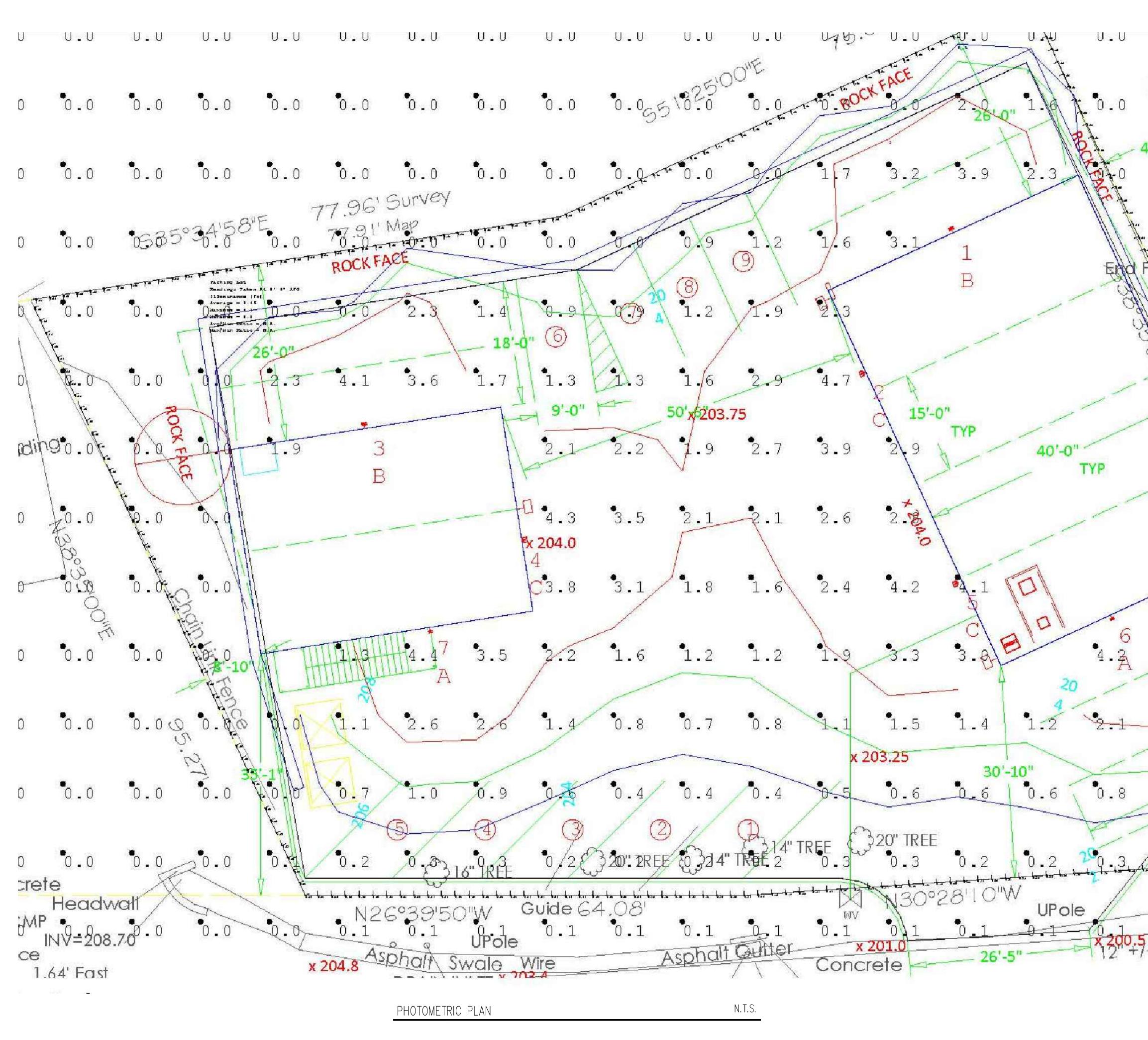




ARCHITECTURE P.C.



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| Labe | | | CalcType | Units | Avg | Max | Min | Avg/Min | Max/Min | Description | PtSpcLr | PtSpcTb | Meter Type |
| Prope | erty Line | | Illuminance | Fc | 0.11 | 0.49 | 0.00 | N.A. | N.A. | Readings Taken On Vertica | 0 2 | N.A. | Vert-PerpCCW |
| Site | | | Illuminance | Fc | 0.22 | 4.7 | 0.0 | N.A. | N.A. | Readings Taken At 0' 0" AF | G 10 | 10 | Horizontal |
| Parki | ng Lot | | Illuminance | Fc | 1.46 | 4.7 | 0.0 | N.A. | N.A. | Readings Taken At 0' 0" AF | G | | 0 |

WPLED26

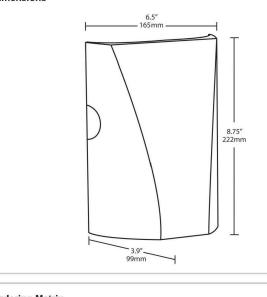


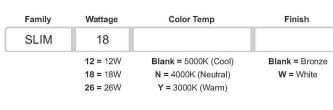
SLIM18

| | Project: | | Type: | |
|--|---|---|---|--|
| | Prepare | d By: | Date: | |
| 2, 18 and 26 Wall SLIM wall packs are ultra efficient an istribution with a compact low-profile design thats super plight. | | Constant Current 0.18A 0.11A 0.03A 0.09A | LED Into Walts Color Temp Color Accuracy L70 Lifespan Lumans Efficacy | 18W 5100K (Cool) 75 CRI 100,000 2,585 122.1 LFW |
| Color: Bronze | Weight: 4.3 lbs | | | |
| echnical Specifications | | | | 1 |
| istings | | | | |
| | Housing: | Green Technol | 1.172 | |
| JL Listing: | Precision die-cast aluminum housing | Mercury and UV h | rea. RoHS-complian | Loomponents. |
| JL Listing: Suilable for well locations. Suilable for mounting within | Precision die-cast aluminum housing Mounting: | Mercury and UV fi | rea. RoHS-complian | l components, |
| JL Listing: JL Listing: JL Listing: JL Listing: Solution State of the ground of the ground. | Precision die-cast aluminum housing | Mercury and UV h LED Character LED: | rea. RoHS-complian fistics | l components. |
| JL Listing: Suilable for well locations. Suilable for mounting within .2m (4ft) of the ground. ADA Compliant: | Precision dia-cast aluminum housing Mounting: Heavy-duly mounting bracket with hinged housing for easy installation | Marcury and UV fr LED Character LED: Multi-ship, long-life | rea. RoHS-complian fistics | l components, |
| UL Listing: Suilable for wet locations. Suilable for mounting within 1.2m (41) of the ground. ADA Compliant: SLIM TM is ADA Compliant | Precision dia-cast aluminum housing Mounting: Heavy-duly mounting bracket with hinged housing for | Mercury and UV fr LED Character LED: Nulli-chip, long-life Lifespan: | a LED | |
| Listings UL Listing: Suilable for well locations. Suilable for mounting within 1.2m (4R) of the ground. ADA Compliant: SLIM TM is ADA Compliant IESNA LM-79 & LM-80 Tosting: RAB LED luminaires and LED components have been | Precision dia-cast aluminum housing Mounting: Heavy-duly mounting bracket with hinged housing for easy installation Recommended Mounting Height: | Mercury and UV fr LED Character LED: Nulli-chip, long-life Lifespan: | rea. RoHS-complian fistics a LED D lifespan based on I | |
| UL Listing: Suilable for well locations. Suilable for mounting within 1.2m (4ft) of the ground. ADA Compliant: SLIM TM is ADA Compliant IESNA LM-79 & LM-80 Testing: RAB LED luminaities and LED components have been ested by an independent laboratory in accordance | Precision dia-cast aluminum housing Mounting: Heavy-duty mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft | Mercury and UV fr LED Character LED: Multi-chip, long-life Lifespan: 100,000-hour LED | rea. RoHS-complian Istics a LED D lifespan based on I calculations | |
| UL Listing: Suilable for well locations. Suilable for mounting within 1.2m (4R) of the ground. ADA Compliant: SLIM TM is ADA Compliant IESNA LM-79 & LM-80 Testing: RAB LED luminaires and LED components have been lested by an independent laboratory in accordance with IESNA LM-79 and LM-90. | Precision dia-cast aluminum housing Mounting: Heavy-duty mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft Lens: | Mercury and UV In LED Character LED: Multi-ship, long-life Lifespan: 100,000-hour LED results and TM-21 Color Consiste 7-step MacAdam | rea. RoHS-complian Istics a LED Dilfespan based on I calculations Ency: Ellipse binning to ac | ES LM-80 |
| UL Listing: Suilable for well locations. Suilable for mounting within 1.2m (4R) of the ground. ADA Compliant: SUIM TM is ADA Compliant ESNA LM-79 & LM-80 Tosting: RAB LED luminatives and LED components have been ested by an independent laboratory in accordance with IESNA LM-79 and LM-80. DLC Listod: | Precision dia-cast aluminum housing Mounting: Heavy-duly mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft Lens: Tempered glass lens | Mercury and UV fr LED Character LED: Multi-chip, long-life Lifespan: 100,000-hour LED results and TM-21 Color Consiste 7-stap MacAdam fixtura-to-fixtura co | rea. RoHS-complian Istics Dilfaspan based on I calculations Ency: Ellipse binning to ac obr | ES LM-80 |
| JL Listing: Suilable for well locations. Suilable for mounting within 1.2m (4R) of the ground. ADA Compliant: SLIM™ is ADA Compliant ESNA LM-79 & LM-80 Tosting: RAB LED luminaires and LED components have been ested by an independent laboratory in accordance with IESNA LM-79 and LM-90. DLC Listed: This product is listed by Design Lights Consortium DLC) as an ultra-efficient premium product that | Precision dia-cast aluminum housing Mounting: Heavy-duty mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft Lens: Tempared glass lens Reflector: | Mercury and UV In LED Character LED: Multi-chip, long-life Lifespan: 100,000-hour LEE results and TM-21 Color Consiste 7-step MacAdam fixture-to-fixture co Color Stability: | rea. RoHS-complian Istics a LED D lifespan based on I calculations ency: Ellipse binning to ac olor | ES LM-80 hieve consistent |
| JL Listing: Suilable for well locations. Suilable for mounting within t.2m (4ft) of the ground. ADA Compliant: SLIM™ is ADA Compliant ESNA LM-79 & LM-80 Tosting: R4B LED luminaties and LED components have been asked by an independent laboratory in accordance with IESNA LM-79 and LM-90. DLC Listed: This product is listed by Design Lights Consortium DLC) as an ultra-efficient premium product that qualifies for the highest lier of rebates from DLC Wamber Utilities. | Precision dia-cast aluminum housing Mounting: Heavy-duly mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft Lens: Tampared glass lens Reflector: Specular thermoplastic Gaskets: High-lamparature silicone | Mercury and UV In LED Character LED: Multi-chip, long-life Lifespan: 100,000-hour LED results and TM-21 Color Consiste 7-step MacAdam fixture-to-fixture co Color Stability: LED color tempera | rea. RoHS-complian Istics Dilfaspan based on I calculations Ency: Ellipse binning to ac obr | ES LM-80 hieve consistent o shift no more |
| JL Listing: Suilable for well locations. Suilable for mounting within 2m (4ft) of the ground. ADA Compliant: SUM™ is ADA Compliant ESNA LM-79 & LM-80 Tosting: AB LED luminaties and LED components have been asted by an independent laboratory in accordance with IESNA LM-79 and LM-90. DLC Listed: This product is listed by Design Lights Consortium DLC) as an ultra-efficient premium product that pualifies for the highest lier of rebates from DLC Amotor Utilities. DLC Product Code: PSPVC3C7 | Precision dia-cast aluminum housing Mounting: Heavy-duty mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft Lens: Tempered glass lens Reflector: Specular thermoplastic Gaskets: High-temperature silicone Finish: | Mercury and UV In LED Character LED: Multi-chip, long-life Lifespan: 100,000-hour LED results and TM-21 Color Consiste 7-step MacAdam fixture-to-fixture co Color Stability: LED color tempera | rea. RoHS-complian Istics a LED Dilfaspan based on I calculations ency: Ellipse binning to ac olor ature is warrantied to | ES LM-80 hieve consistent o shift no more |
| JL Listing: Suitable for well locations. Suitable for mounting within 1.2m (4II) of the ground. ADA Compliant: SLIM™ is ADA Compliant ESNA LM-79 & LM-80 Tosting: RAB LED luminaties and LED components have been asted by an independent laboratory in accordance with IESNA LM-79 and LM-80. DLC Listed: This product is listed by Design Lights Consortium DLC) as an ultra-efficient premium product that qualifies for the highest tier of rebates from DLC Member Utilities. DLC Product Code: PSPVC3C7 Construction | Precision dia-cast aluminum housing Mounting: Heavy-duly mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft Lens: Tampared glass lens Reflector: Specular thermoplastic Gaskets: High-lamparature silicone | Mercury and UV In LED Character LED: Multi-chip, long-life Lifespan: 100,000-hour LED results and TM-21 Color Consiste 7-step MacAdam fixture-to-fixture co Color Stability: LED color tempera | rea. RoHS-complian Istics a LED Dilfaspan based on I calculations ency: Ellipse binning to ac olor ature is warrantied to | ES LM-80 hieve consistent o shift no more |
| JL Listing: Suilable for well locations. Suilable for mounting within 1.2m (4ft) of the ground. ADA Compliant: SLIM™ is ADA Compliant ESNA LM-79 & LM-80 Tosting: RAB LED luminaties and LED components have been asted by an independent laboratory in accordance with IESNA LM-79 and LM-80. DLC Listod: This product is listed by Design Lights Consortium DLC) as an ultra-efficient premium product that qualifies for the highest lier of rebates from DLC Aimber Utilities. DLC Product Code: PSPVC3C7 Construction P Rating: | Precision dia-cast aluminum housing Mounting: Heavy-duty mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft Lens: Tempered glass lens Reflector: Specular thermoplastic Gaskets: High-temperature silicone Finish: | Mercury and UV In LED Character LED: Multi-chip, long-life Lifespan: 100,000-hour LED results and TM-21 Color Consiste 7-step MacAdam fixture-to-fixture co Color Stability: LED color tempera | rea. RoHS-complian Istics a LED Dilfaspan based on I calculations ency: Ellipse binning to ac olor ature is warrantied to | ES LM-80 hieve consistent o shift no more |
| JL Listing: Suitable for wet locations. Suitable for mounting within 1.2m (4ft) of the ground. ADA Compliant: SLIM™ is ADA Compliant ESNA LM-79 & LM-80 Tosting: RAB LED luminaties and LED components have been asked by an independent laboratory in accordance with IESNA LM-79 and LM-80. DLC Listed: This product is listed by Design Lights Consortium DLC) as an ultra-efficient premium product that qualifies for the highest lier of rebates from DLC damber Utilities. DLC Product Code: PSPVC3C7 Construction P Rating: ngress Protection rating of IP86 for dust and water | Precision dia-cast aluminum housing Mounting: Heavy-duty mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft Lens: Tempered glass lens Reflector: Specular thermoplastic Gaskets: High-temperature silicone Finish: | Mercury and UV In LED Character LED: Multi-chip, long-life Lifespan: 100,000-hour LED results and TM-21 Color Consiste 7-step MacAdam fixture-to-fixture co Color Stability: LED color tempera | rea. RoHS-complian Istics a LED Dilfaspan based on I calculations ency: Ellipse binning to ac olor ature is warrantied to | ES LM-80 hieve consistent o shift no more |
| UL Listing: Suilable for wat locations. Suilable for mounting within 1.2m (4R) of the ground. ADA Compliant: SLIM TM is ADA Compliant IESNA LM-79 & LM-80 Testing: | Precision dia-cast aluminum housing Mounting: Heavy-duty mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft Lens: Tempered glass lens Reflector: Specular thermoplastic Gaskets: High-temperature silicone Finish: | Mercury and UV In LED Character LED: Multi-chip, long-life Lifespan: 100,000-hour LED results and TM-21 Color Consiste 7-step MacAdam fixture-to-fixture co Color Stability: LED color tempera | rea. RoHS-complian Istics a LED Dilfaspan based on I calculations ency: Ellipse binning to ac olor ature is warrantied to | ES LM-80 hieve consistent o shift no more |
| UL Listing: Suitable for well locations. Suitable for mounting within 1.2m (4R) of the ground. ADA Compliant: SLIM™ is ADA Compliant IESNA LM-79 & LM-80 Tosting: RAB LED luminatives and LED components have been lested by an independent laboratory in accordance with IESNA LM-79 and LM-90. DLC Listed: This product is listed by Design Lights Consortium (DLC) as an ultra-efficient premium product that qualifies for the highest lier of rebates from DLC Member Utilities. DLC Product Code: PSPVC3C7 Construction IP Rating: Ingress Protection rating of IP86 for dust and water Cold Weather Starting: | Precision dia-cast aluminum housing Mounting: Heavy-duty mounting bracket with hinged housing for easy installation Recommended Mounting Height: Up to 14 ft Lens: Tempered glass lens Reflector: Specular thermoplastic Gaskets: High-temperature silicone Finish: | Mercury and UV In LED Character LED: Multi-chip, long-life Lifespan: 100,000-hour LED results and TM-21 Color Consiste 7-step MacAdam fixture-to-fixture co Color Stability: LED color tempera | rea. RoHS-complian Istics a LED Dilfaspan based on I calculations ency: Ellipse binning to ac olor ature is warrantied to | ES LM-80 hieve consistent o shift no more |

RAE Outdoor SLIM18

Technical Specifications (continued) LED Characteristics Electrical Warranty: Color Uniformity: RAB warrants that our LED products will be free from Driver: RAB's range of CCT (Correlated Color Temperature) defects in materials and workmanship for a period of Constant Current, Class 2, 100-277V, 50/60 Hz., 4KV five (5) years from the date of delivery to the end user, surge protection, 120V: 0.19A, 208V: 0.11A, 240V: follows the guidelines for the American National including coverage of light output, color stability, driver 0.10A, 277V: 0.08A Standard for Specifications for the Chromaticity of performance and fixture finish. RAB's warranty is Solid State Lighting (SSL) Products, ANSI C78.377-THD: subject to all terms and conditions found at www.rablighting.com/legal#warranty 11% at 120V, 21% at 277V Other Buy American Act Compliance: Power Factor: Patents: RAB values USA manufacturing! Upon request, RAB 99.2% at 120V, 91.5% at 277V The design of the SLIM™ is protected by patents in may be able to manufacture this product to be U.S. Pat D681,864, and pending patents in Canada, compliant with the Buy American Act (BAA). Please China, Taiwan and Mexico. contact customer service to request a quote for the product to be made BAA compliant. HID Replacement Range: Optical Replaces 100W Metal Halide BUG Rating: B1 U0 G0 Dimensions Features Full cutoff, fully shielded LED wall pack Can be used as a downlight or uplight Contractor friendly features for easy installation 100,000-hour LED Life 5-Year, No-Compromise Warranty **Ordering Matrix**





Luminaire Location Summary LumNo Label Mounting Height SLIMFC37 20 WPLED26 18 2 SLIMFC37 20 3 WPLED26 18 4 WPLED26 18 SLIM18 16 6 16 SLIM18

NOTES: * The light loss factor (LLF) is a product of many v has been applied to the calculated results unless of mean lumens / initial lumens per lamp manufac

* Illumination values shown (in footcandles) are th horizontal, vertical or inclined as designated in the to the plane of calculation.

* The calculated results of this lighting simulation r Actual measured results may vary from the anticip to means and methods which are beyond the cont

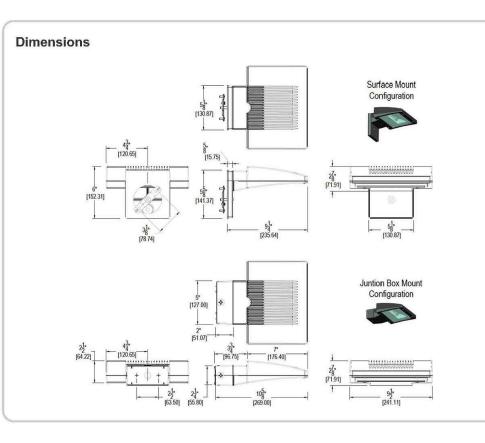
* Mounting height determination is job site specific (insertion point of the luminaire symbol) to be take

and at the bottom of the symbol for all other lumin * Damin Sales luminaire and product designs are Patents issued or pending apply.

PLAET Outdoor

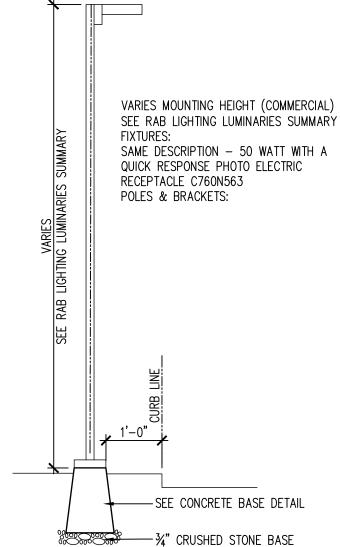
WPLED26

5000K (Cool) 100,000 3,483 119.7 LPW



Features

Maintains 70% of initial lumens at 100,000-hours Weatherproof high temperature silicone gaskets Superior heat sinking with die cast aluminum housing and external fins 100 up to 277 Volts 5-Year, No-Compromise Warranty



LIGHTING POLE DETAIL

RAE Outdoo

RAB Outdoor

Driver Options Blank = Standard (120-277V) Blank = No Option /D10 = Dimmable /PC = 120V Button /PC2 = 277V Button /LC = Lightcloud® Controller

SLIMFC37

Housing:

Precision dia-cast aluminum housing and door frame



Formulated for high durability and long-tasting color

| Prepared | · | Date: | | |
|-------------|------------------|----------------|--------------|--|
| Driver Info | (| LED Info | | |
| Тура | Constant Current | Watts | 37W | |
| 120V | 0.31A | Color Temp | 5000K (Cool) | |
| 2087 | 0.19A | Color Accuracy | 73 CR1 | |
| 240V | 0.16A | L70 Lifespan | 100,000 | |
| 277V | 0.14A | Lumens | 3,585 | |
| Input Watts | 35.3W | Efficacy | 101 LPW | |

| Technical Specifications | | | |
|--|---|---|--|
| Listings | Mounting: | Green Technology: | |
| UL Listing: | Dia-cast back box with four (4) conduit entry points | Mercury and UV free. RoHS-compliant components. | |
| Suitable for Wat Locations, Wall Mount Only. | and knockout pattern for junction box or direct wall | LED Characteristics | |
| ESNA LM-79 & LM-80 Testing: | mounting. Hinged housing and bubble level for easy installation. | LED: | |
| RAB LED luminaires and LED components have been | Full Cutoff: | Long-life, high-efficiency, micro-power, surface moun LEDs; binned and mixed for uniform light output and color | |
| lested by an independent laboratory in accordance with IESNA LN-79 and LN-80. | Full-cutoff meets dark-sky requirements | | |
| Construction | Recommended Mounting Height: | Lifespan: | |
| Footprint: | Up to 20 N | 100,000-hour LED lifespan based on IES LM-80 results and TM-21 calculations Color Stability: LED color temperature is warrantied to shift no more | |
| Designed to replace RAB HID WP1 wall packs, both in | Lens: | | |
| size and lootprint lamplale, so upgrading to LED is | Microprismatic diffusion glass lens reduces glare and | | |
| easy and seamless | has smooth and even light distribution | | |
| IP Rating: | Reflector | Ihan 200K in CCT over a 5-year period | |
| Ingress Protection rating of IP66 for dust and water | Specular thermoplastic | Color Consistency: | |
| Cold Weather Starting: | Gaskets: | 7-step MacAdam Ellipse binning to achieve consisten | |
| Minimum starting temperature is -40 C (-40 F) | The unique design of the light-lock gasket ensures no | fixtura-lo-lixtura color | |
| Maximum Ambient Temperature: | walar or environmental elements will ever get inside the SLIM | | |
| Suilable for use in 40 C (104 F) | Finish: | | |
| | | | |

| variables, only lamp lumen depreciation (LLD) otherwise noted. The LLD is the result (quotient) cturers' specifications. | | ARQ. ARCHITECTURE P.C. |
|--|--|---|
| he predicted results for planes of calculation either e calculation summary. Meter orientation is normal | Isoline Legend Illuminance (Fc) | |
| represent an anticipated prediction of system performance. pated performance and are subject | Color Value 0.5 | ARCHITECTURE - PLANNING & ALL RIGHTS RESERVED & ENGINEERING |
| ntrol of Damin Sales. ic, our lighting simulations assume a mounting height en at the top of the symbol for ceiling mounted luminaires | 2 | 100 EXECUTIVE BLVD. SUITE 204 OSSINING, NY 10562 |
| naire mounting configurations. protected under U.S. and International intellectual property laws. | | PHONE: (914) 944-3377 FAX: (866) 567-6240 |
| | | JORGE B. HERNANDEZ R.A. A.I.A. LICENSE NUMBER: 030424-1 CERTIFICATE NUMBER: 0973256 |
| | | |
| | | PAUL A. BERTE, P.E 100 EXECUTIVE BLVD. SUITE 204 OSSINING, NY 10562 |
| | | |
| | | REVISIONS DATE BY |
| | | CONSULTING CMT. 3/23/2022 ARQ. |
| | | |
| | T CIRCLE METER (9" – 10") | |
| 2" GALV. STEEL CONDUIT SLEEVE | E MANU. TEMPLATE) | |
| CONCRETE BASE PLAN | <u> </u> | |
| SQUARE → BEVE | LLED EDGE | |
| | IOR BOLTS 4 EACH, 1" | |
| | ADED 36" LONG ³ 4" DIAMETER CONDUIT FOR | |
| CAP GROU | ND WIRE. | |
| | GROUND WIRE- #4 AWG COPPER | |
| 4" MIN. CRUSHED STONE BASE 24" SQUARE | 5⁄8" COPPER CLAD ROD MIN. 10'−0" LONG | |
| CONCRETE BASE SECTION | | <u>drawing title:</u> SCHEDULES, SPECIFICATIONS & |
| | 1/4" = 1'-0" | DETAIL |
| | | PROJECT: |
| | | MISTIS PROPERTIES INC. |
| | | PROJECT ADDRESS: |
| | | 176 VIRGINA ROAD White plains |
| IMFC37 | FAE Outdoor | NEW YORK, 10603 |
| chnical Specifications (continued) ner Warranty: | Electrical | DOB EXAMINER SIGNATORE: |
| cessories: RAB warrants that our LED products w ilable accessories include polyshield and wire defects in materials and workmanship f rd. Click www.rablighting.com//filter/Wall filve (5) years from the date of delivery I kACCESSORIES to see all accessories. including coverage of light output, color performance and fixture finish. RAB's w performance and fixture finish. RAB's w | or a period of Constant Current, 100-277V. 50/60Hz, 100-277VAC to the end user, 0.6A, 4kV Surge Protection, 700mA, Power Factor 99.6%. | |
| ents: design of the SLIM™ is protected by patents ding in US, Canada, China, Taiwan and Mexico Buy American Act Compliance: | d at THD: 22.1% at 347V | |
| P Replacement Range: RAB values USA manufacturing! Upon laces 200W Metal Halide may be able to manufacture this produc compliant with the Buy American Act (E contact customer service to request a c product to be made BAA compliant. product to be made BAA compliant. | ct to be BAA). Please | |
| Detical BUG Rating: B1 U0 G0 | | DOB BSCAN STICKER: |
| nensions | Features | |
| | Covers footprint of most traditional wall packs Easy installation with hinged access, bubble level and multiple conduit entries | |
| 9.5" 241mm | Tight-lock gasket keeps elements out 100,000-hour LED lifespan 5-Year, No-Compromise Warranty | SEAL & SIGNATURE |
| | | STATE OF NEW LOOP |
| | | + CHUL A. BER PR |
| 9″5″ 128mm/ 127mm | | |
| lering Matrix | | POFESSIONAL |
| amily Cutoff Wattage Color Temp Finish | Driver Options Options Other Options | |
| Blank = Cutoff (10 37 = Blank = 5000K Blank = Blank = </td <td>ank = Standard (120- 277V) /LC = Lightcloud® Controller Blank = Standard Blank = No Option USA = BAA /BL = Bi-Level /PC = 120V Button Photocell Compliant</td> <td>03/08/2021 PROJECT NO.: 7</td> | ank = Standard (120- 277V) /LC = Lightcloud® Controller Blank = Standard Blank = No Option USA = BAA /BL = Bi-Level /PC = 120V Button Photocell Compliant | 03/08/2021 PROJECT NO.: 7 |
| degrees) 57W Y = 3000K (Warm) 62 = 62W | /D10 = Dimmable /PC2 = 277V Button Photocell /480 = 480V /PCS = 120V Swivel Photocell /PCS2 = 277V Swivel Photocell | 21029 L - J DRAWING BY: |

CHECKED BY:

14 OF 14



MEMORANDUM

| North Castle Planning Board |
|--|
| Adam Kaufman, AICP Paul Berte, P.E. Mistis Properties, Inc. |
| Joseph M. Cermele, P.E., CFM Kellard Sessions Consulting Consulting Town Engineers |
| May 7, 2021 |
| Mistis Properties 176 Virginia Road Section 122.16, Block 1, Lot 3 |
| |

As requested, Kellard Sessions Consulting has reviewed the site plans submitted in conjunction with the above-referenced project. The applicant is proposing the installation of two (2) prefabricated warehouse buildings and associated parking, stormwater mitigation and utilities. The plan requires the construction of retaining walls greater than six (6) feet in height. The property is 16,709 s.f. in size and located in the IND-A Zoning District.

Our comments are outlined below.

GENERAL COMMENTS

1. The plan requires a retaining wall along the rear and sides of the property with a maximum height of approximately 28 feet. For clarity, the proposed retaining wall shown on the grading plan shall include top of wall and bottom of wall elevations at each change in elevation to be coordinated with the elevation changes shown in section provided on Sheet S-7. The plan shall also illustrate the required grading of the slopes above the retaining wall necessary to maintain the proposed 1V:2H slope and include a detail of the armored slope referenced in the typical retaining wall sections.

CIVIL ENGINEERING | LANDSCAPE ARCHITECTURE | SITE & ENVIRONMENTAL PLANNING

North Castle Planning Board Mistis – 176 Virginia Road May 7, 2021 Page 2 of 3

- 2. Provide a site lighting and photometric plan for review and consideration by the Planning Board. Include specifications and details of all fixtures, poles and pole bases.
- 3. The proposed Frame and Cover Detail on Sheet S-5 shall require that manhole covers for sanitary sewers be stamped with "SEWER" on the casting.
- 4. The sanitary manhole detail included on Sheet S-2 shall be revised to require two (2) coats of bitumastic sealant to be applied to the exterior.
- 5. Please clarify the purpose and intent of the proposed pipe entering the proposed sewer manhole from the east. It is assumed that this is the sanitary connection for the bathroom in the building. Please specify the pipe size and material as necessary.
- 6. The hydrologic model demonstrates that the proposed stormwater management system will effectively mitigate the increased stormwater runoff generated by the project. However, it is suggested that the plan be revised to also connect the roof leaders of the proposed 2-bay warehouse (north building) and proposed Drain Inlet, DI-1. Doing so would allow the for the connection of Drain Inlet, DI-1, to the proposed Stormtrap System, rather than the current proposed 6"x 6" PVC TEE connection, (which would not be acceptable) and eliminate the multiple connections at Drain Inlet, DI-2. This would also provide mitigation and treatment for all impervious surfaces. The outlet structure shall be labeled on plan Sheet S-2.
- 7. The proposed outlet structure needs clarification. The detail on Sheet S-5 indicates a connection to the hydrodynamic separator; however, there is no indication of this on plan Sheet S-2. The detail also indicates outlet pipes of twelve (12) inch and four (4) inch diameter, while the plan illustrates a six (6) inch outlet pipe to Drain Inlet, DI-2. Please clarify. It appears that an additional structure will be required for the pipe transition or an alternate outlet structure design. The six (6) inch pipe connection should also be included in the hydrologic model. It currently appears to be undersized.
- 8. Drainage area maps shall be included in the Stormwater Pollution Prevention Plan (SWPPP). The areas and cover types tributary to each sub-catchment shall be clarified.

As additional information becomes available, we will continue our review. It is noted that an itemized response to all comments will facilitate completeness and efficiency of review.

PLANS REVIEWED, PREPARED BY PAUL BERTE, P.E., DATED MARCH 8, 2021:

- Site Plan, Map, & Notes (S-1)
- Drainage, Grading & Erosion Control (S-2)

North Castle Planning Board Mistis – 176 Virginia Road May 7, 2021 Page 3 of 3

- Turns & Sight Distance (S-3)
- Site Details (S-4 & S-5 & S-6)
- Section & Site Details (S-7)
- 2-Bay Warehouse First & Mezzanine Flood Plans (A-1)
- 4-Bay Warehouse Flood Plan (A-2)
- 2-Bay Warehouse Elevations (A-3)
- 4-Bay Warehouse Elevations (A-4)
- Engineer's Report

JMC/dc

https://kellardsessionsconsulti.sharepoint.com/sites/Kellard/Municipal/Northcastle/Corresp/018SitePlans/2021-05-07_NCPB_Mistis - 176 Virginia Rd_Review Memo.docx



Engineers Report

for

176 Virginia Road

White Plains, NY

Westchester County, New York

Calculations and Report By: Paul Berté

Date: <u>3/29/2022</u>

Last Revised: _____



NYS Professional Engineer Lic # 071859

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A. Introduction

This report has been prepared in accordance with Chapter 267 of the town of North Castle for the improvements on site on the subject property 176 Virginia Road.

This site is 0.383 acres as shown on a survey prepared by T.C. Merritts Surveyors, map of survey dated August 30, 2018 entitled "TOPOGRAPHY OF PROPERTY PREPARED FOR MISTIS PROPERTIES 100 INC".

This existing impervious surfaces on this lot is approximately 13,586 sf. 11,510 sf of the lot is covered in a heavily compacted subbase type material over asphalt pavement so it has been modeled as an impervious surface. The proposed impervious surfaces is estimated to be 13,340 sf.

An underground stormwater detention system is designed to reduce the post development peak rates of runoff to the pre-development rates for the 2, 10, 25, 100 24-hour storm events. Proposed grading has been designed to minimize the impact to adjacent properties and the Erosion and Sediment controls have been designed to manage stormwater and pollutants from the active site in accordance with the New York Standards and Specifications for Erosion and Sediment Control. Detailed flow control calculations (routings) are also included herewith.

B. Site Description

Existing Improvement

Existing improvements include a single-family home.

Existing Tress

Six (6) trees are to be removed from the property.

Soils

The site consists primarily of CuD Chatfield-Hollis-Rock described by the USDA NRCS as outcrop complex, 15 to 35 percent slopes. A soil report is included in Appendix B.

Flood Plain

The FEMA floodplain map (attached herewith in section 2) does not show potential floodplains in the immediate vicinity of the site.

Site Drainage Characteristics

Most of the site currently drains via sheet flow from north to south. Runoff drains into a concrete swale and catch basin in the street. Other than collecting and treating the runoff from the proposed impervious areas, no changes to the site drainage patterns are proposed with this application.

C. Project Description

Proposed Improvements

Proposed improvements include the installation of two (2) prefabricated warehouse buildings and parking areas.

D. Stormwater Management Methodology

Stormwater Runoff Rate

The on-site stormwater management design was analyzed using HydroCAD stormwater modeling software V10.00, which models Type III 24 hour stormwater flows using those methods contained in "Urban Hydrology for Small Watersheds Technical Release No. 55," prepared by the United States Department of Agriculture Soil Conservation Service. The stormwater management plan has been implemented to provide quality of runoff and flow control by means of the storage provided within the 4 Stormtrap ST1 Units. The system provides 666 cf of storage. The design was based on detention values for the 100, 25, 10, and 2-year storm events, to mitigate the post-development runoff flow to a rate that is less than the pre-development condition.

Grading and Drainage Design

This proposal includes a drainage design in accordance with chapter 9 of the 2015 Stormwater Management Design manual for redevelopment activity. It is anticipated that the subsurface stormwater treatment system will be installed in bedrock. Because infiltration will not be possible, the system has been designed as a detention system. The water quality volume has been achieved with a hydrodynamic separator which provides a total of 666 cf. Calculations can be seen in Appendix E. The runoff from roof leaders of the warehouse building 1 and 2 and the area Drain Inlet DI-1 will be sent to the Stormtrap ST1 units and then be directed to the hydrodynamic separator to be treated and finally out to the catch basin in the street. Runoff from the Trench Drain at the driveway entrance will be captured and directed to the hydrodynamic separator for treatment before going to the catch basin in Virginia Road. Runoff is captured and treated to mitigate the peak flows to the predevelopment rates.

Run off from the rain events will be detained in the Stormtrap ST1 single trap units and metered out to catch basin via a 8" orifice located at the base of the proposed underground detention system. The system is designed to reduce the peak rate of discharge from the existing condition.

Throughout the construction process, strict adherence to the **Site Plan** which specifies all erosion and sediment controls will be maintained to minimize sediment and pollutants from discharging off site.

The following table (Table 1) is a summary of the results of the hydrograph routings for the Pre-Development and Post Development stormwater flows for the entire property:

| Storm Event | Rainfall Depth | Pre Development | Post Development |
|-------------|----------------|-----------------|------------------|
| | | Rate (cfs) | Rate (cfs) |
| 2 year | 3.4″ | 1.03 | .94 |
| 10 year | 5.1″ | 1.61 | 1.44 |
| 25 year | 6.4″ | 2.06 | 1.79 |
| 100 year | 9.0″ | 2.93 | 2.47 |

Table 1: Summary of Stormwater Runoff Rates

E. Construction Phasing Plan and Stormwater Management Facilities Maintenance Program

Maintenance of Temporary and Permanent Structures and Practices

Temporary and permanent erosion controls measures will be maintained and inspected in accordance with the Site Plans and Details. All proposed soil erosion and sediment control practices are designed in accordance with the following publications:

- New York State Standards and Specifications for Erosion and Sediment Control, August 2005, latest edition.
- New York State Guidelines for Urban Erosion and Sediment Control, latest edition,
- New York State General Permit for Stormwater Discharges,
- "Reducing the Impacts of Stormwater Runoff from New Development", as published by the New York State Department of Environmental Conservation (NYSDEC).

The proposed soil erosion and sediment control devices include: protective earthmoving procedures and grading practices, soil stabilization, inlet protection, stabilized construction entrance and silt fencing. The approach of the plan is to control off-site sedimentation and re-establish vegetation as soon as practicable.

Additionally, contractor shall adhere to the recommended material stockpile location and construction entrance shown on the plans attached herewith. The plan will be implemented prior to commencement of earthmoving activities.

Construction Phasing Plan:

- 1. Erosion and sediment control (esc) measures and pollution prevention (pp) implementation,
 - a. Install silt fences along easterly project limits
 - b. Utilize existing driveway for construction access. If required, install stone (per detail) if access is required on and off exposed areas.
 - c. Install tree protection
 - d. Install temporary sanitary facilities (portable toilets) in a location that is at least 20 from any drainage facility or flow path. Recommend staking the facility to prevent accidental tipping by construction activity or wind.
 - e. Install waste container maintain rigorous site cleaning schedule to prevent debris from blowing off site. Construction waste shall be stored in a dumpster and carried off-site on a regular basis
 - f. Allocate concrete washout areas in advance of pour.
- 2. Phase 1 clearing, demo & excavation.
 - a. Clear all trees marked to be removed on the demolition plan. Clear & grub north half of the property, north of the existing 1 story residence. The existing residence is to remain until phase 2.
 - b. Excavate to proposed grade. It is anticipated that rock will be present on portions of the site. A minimum productivity rate of 55yards per day with a 2000ftlbf hammer is proposed to remove any rock encountered.
 - c. Due to the limited site area, all excavated soils & rock are to be immediately trucked off site.
 - d. Disturbed areas where construction will cease for more than 14 days will be stabilized with erosion controls, such hydro-seeding, hydro-mulch, or hay
 - e. Form and pour proposed retaining wall along rear property line up to the next phase section. If stable rock is encountered along the limits of the proposed cut, the retaining wall will not be necessary.
- 3. Construct retaining wall on west side of lot as shown in phase 1 phasing plan.

- 4. Cut in curb at proposed location and install temporary construction entrance.
- 5. Phase 2 clearing, demo & excavation
 - a. Disconnect and cap all utilities to existing 1 story residence. Restore pavement as specified.
 - b. Demolish all existing improvements remaining on the demolition plan.
 - c. Clear & grub south end of property.
 - d. Excavate to proposed grade. It is anticipated that rock will be present on portions of the site. A minimum productivity rate of 55cubicyards per day with a 2000ftlbf hammer is proposed to remove any rock encountered.
 - e. Due to the limited site area, all excavated soils & rock are to be immediately trucked off site.
 - f. Disturbed areas where construction will cease for more than 14 days will be stabilized with erosion controls, such hydro-seeding, hydro-mulch, or hay.
- 6. Form and pour proposed retaining wall along rear property line up to the next phase section. If stable rock is encountered along the limits of the proposed cut, the retaining wall will not be necessary.
- 7. Install subsurface storage system, site drainage and hydrodynamic separator to capture roof leader runoff. Protect inlets with sediment control inlet protection.
- 8. Vertical construction (install foundation and construct prefab metal warehouse structures)
- 9. Install underground utilities. Restore pavement as specified.
- 10. Install proposed curb and asphalt parking lot
- 11. Paint all striping for proposed lot.
- 12. Plant all proposed landscaping.
- 13. Final stabilization of disturbed areas
 - a. Remove all esc and pp measures upon approval of design engineer and/or esc inspector.
 - b. Awarded contractor shall be responsible for the proper implementation of the esc and pp practices. The following maintenance program is proposed in order to maintain the proper function of all drainage and erosion and sediment control facilities:
 - Inspect sediment control devices and construction access point routinely and if necessary remove accumulated sedimentation and debris; at no point should the filter bed be allowed to continue operations beyond 50% of its capacity being compromised by debris.
 - ii. All disturbed area will be stabilized and the sediment build-up in the filter removed. After the construction is completed, any areas disturbed shall be stabilized immediately after the required work is completed.
 - iii. Restore and re-seed any eroded areas as soon as possible
 - iv. The stormwater management facilities maintenance program will be managed by the home owner and shall include yearly inspection of the on-site catch basins and underground storage facilities and the removal of sediment, as necessary.

F. Narrative Report

The primary goal of the soil erosion and sediment control measures is to reduce soil erosion from areas stripped of vegetation during and after construction, and to prevent discharge of silt offsite. Erosion control barriers shall be placed around exposed areas during construction. The barriers shall consist of silt fence. Alternate practice may be implemented by the contractor after approval from the Design Engineer and the City Engineer.

Any areas stripped of vegetation during construction will be left bare for the shortest time possible. Any topsoil removed during construction will be temporarily stockpiled for future use in grading and landscaping. Stockpile locations have been provided on the Erosion and Sediment Control Plan and shall be contained within a silt fence/hay bale barrier.

Temporary vegetation will be established to protect exposed soil areas during construction. If growing conditions are not suitable for the temporary vegetation, mulch will be used. Materials that may be used for mulching include; straw, hay, salt hay, wood fiber, synthetic soil stabilizers, mulch netting, and sod. A permanent vegetative cover will be established upon completion of construction of those areas that have been brought to finish grade and to remain undisturbed.

A temporary stabilized construction entrance comprised of a stone anti-track pad shall be installed as necessary to minimize dirt tracking onto Virginia Road. The purpose of a stabilized entrance is to remove as much soil from the construction vehicle tires prior to exiting the site and traveling on the existing roadways. During construction, inlet protection (as applicable) will be installed at each storm sewer inlet to minimize the conveyance of silt and sediment through the storm sewer system.

For dewatering activities during excavation of the footings, a dewatering pump shall be in a perforated tub surrounded by filter fabric and stone (or approved alternative). Clean discharge should be directed to onsite drainage appurtenances to minimize erosion of soils. Discharge with suspended sediment shall be connected to a sediment bag on undisturbed ground in a location where the discharge will not cause erosion or flow over exposed soils.

Portable toilets shall be provided and located at least 20 feet from a drainage facility and shall be staked down to minimize overtopping from wind.

If the contractor encounters ground water during the excavation of the filtering system, he shall notify the design engineer immediately. The contractor shall store all excavated material at the designated location show on the Grading and Erosion Control Plan with the appropriate erosion control measures corresponding to the stockpile detail.

G. Material Handling and Waste Management

Contractor shall be responsible for all waste materials being collected and disposed of into one (1) metal trash dumpster. Dumpster shall have a secure watertight lid, be placed away from stormwater conveyances and drains, and meet all local and state solid-waste management regulations. Only trash and construction debris from the site will be deposited in the dumpster.

Contractor shall not store erodible or hazardous materials on any roadway. Oil and machinery fuels shall be kept to a necessary minimum and stored in structurally sound and sealed shipping containers or stored in the contractor's vehicles. Hazardous-material storage should be segregated from other non-waste materials. All hazardous materials will be disposed of in accordance with federal, state, and municipal regulations.

Contractor shall be responsible for maintaining the cleanliness of the streets (driveways/parking and adjacent areas) and storm drain inlet protection (as applicable) Best Management Practices (BMPs) throughout the construction project.

Contractor shall provide adequate designated concrete washout areas throughout the construction project and will be responsible for proper disposal of the concrete, mortar or grout collected there.

One (1) temporary sanitary facility (portable toilet) shall be provided at the site in the combined staging area. The toilet shall be away from a concentrated flow path and traffic flow and shall have collection pans underneath as secondary containment. The unit shall be staked down to prevent wind overtopping the unit.

Wood pallets, cardboard boxes, and other recyclable construction scraps will be disposed of in a designated dumpster for recycling. Construction equipment and maintenance materials shall be stored at the combined staging area.

All spills shall be cleaned up immediately upon discovery. Spent absorbent materials and rags will be hauled

off-site immediately after the spill is cleaned up for disposal. Spill large enough to discharge to surface water will be reported to the National Response Center at 1-800-424-8802. Material safety date sheets, a material inventory, and emergency contact information will be maintained on site.

H. Inspection and Maintenance

1. Stormtrap ST1 Singletrap:

Units shall be inspected once a year, sediment shall be removed. Owner shall be responsible for maintenance.

- Locate manholes connected to the system
- Remove grates or covers
- Vacuum pump the sediment. Do not flush sediment out inlet pipes.
- Replace grates and covers
- Record depth and date and schedule next inspection

2. Catch Basins/Inlets:

- Shall be inspected once a year, trash and debris shall be removed.
- Owner shall be responsible for maintenance.

3. Grease trap/oil separator

- Water generated from the truck wash will be captured by the floor trench drain and directed to the grease trap/oil separator, the baffle will separate the oil from water, then the water will continue towards the outlet pipe that is followed by the house trap and sewer line.
- Oil will rise and build up in the inlet chamber until it gets removed.
- Take a long stick that will reach the bottom of the chamber. Any resistance pushing to the bottom signifies sludge build up. Service the oil/water separator when build up is about 6 inches deep in the inlet chamber.
- Maintenance of oil/water separator must include through pump-out and cleaning a minimum of once a year. Cleaning shall be done before the oil accumulation inside the separator is within 2 inches from the top of the water level
- More frequent cleaning of oil/water separator may be required if deemed necessary by building department inspector.
- Owner shall be responsible for the proper removal and disposal. Owner may be required to maintain on-site records of dates and means of disposal which are subject to review by building department. Any removal of the waste material not performed by the owner's personnel must be performed by a licensed waste water disposal firm.

4. Aqua-Swirl: AS-4:

- During the first year post-construction, the unit should again be inspected every three months and cleaned as needed.
- It is also recommended that the system be inspected and cleaned once annually regardless of whether it has reached its sediment or floatable pollutant storage capacity.
- For the second and subsequent years post-construction, the Aqua-Swirl can be inspected and cleaned once annually if the system did not reach full sediment or floatable pollutant capacity in the first year post-construction.
- If the Aqua-Swirl[®] reached full sediment or floatable pollutant capacity in less than 12 months in the first year post-construction, the system should be inspected once every six months and cleaned as needed.

- AquaShield further recommends that external bypass (diversion) and convergence structures should be inspected and cleaned when feasible during inspection and maintenance events.
- Essential elements of a swirl chamber inspection include observing floating materials and measuring the accumulated sediment at the base of the swirl chamber. These two activities can be performed at the ground surface and there is no need to enter the device. A typical maintenance event includes the vacuuming and disposal of floatable pollutants and sediment from the swirl chamber. Proper health and safety protocols should be followed during all inspection and maintenance events. AquaShield™ recommends that all materials removed during the maintenance process be handled and disposed in accordance with all applicable federal, state and local guidelines. Depending on the influent pollutant characteristics of the system drainage area, it may be appropriate to perform Toxicity Characteristics Leaching Procedure (TCLP) analyses on representative samples of the removed material to ensure that the handling and disposition of materials complies with applicable environmental regulations.
- Owner shall be responsible for maintenance.

I. Final Stabilization

Permanent seeding shall be applied immediately after the final design grades are achieved as applicable throughout the site but no later than fourteen (14) days after construction activities have ceased. After stabilization, accumulated sediment shall be removed from site for disposal along with construction debris, trash and temporary BMPs e.g. silt fences, straw bales, material storage areas, sanitary toilets, etc.

Seedbed preparation/grass application

A minimum depth of 2 to 6 inches shall be applied on areas where disturbance results in subsoil being the final grade surface. The seedbed shall be free of large clods, rocks, woody debris and other intrusive materials; fertilizer shall be applied accordingly.

J. Conclusion:

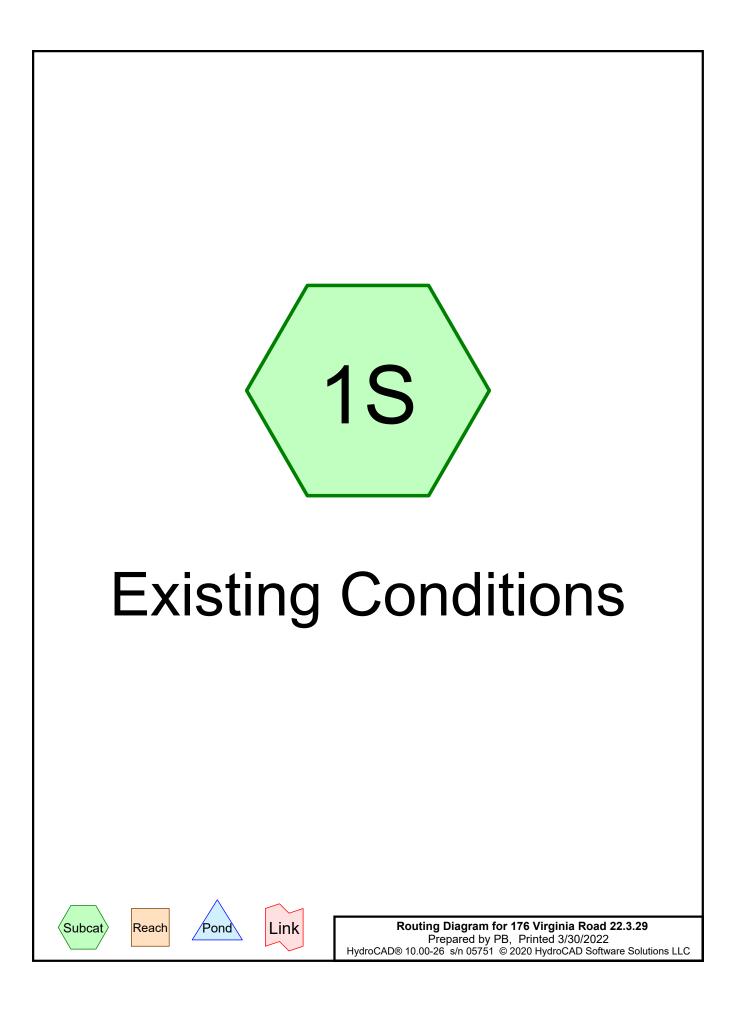
The implementation of this stormwater management plan will mitigate the post development stormwater flows to the predevelopment rates and not adversely affect the adjacent properties or the existing drainage system in Virginia Road.

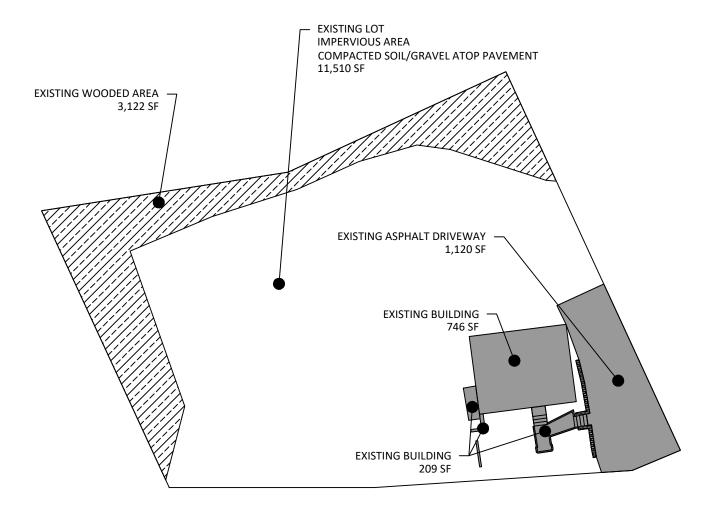
Respectfully Submitted,

ARQ PC

APPENDIX A

Stormwater Routings Pre Development Calculations





PRE DEVELOPMENT SCALE: 1" = 30'

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Area Listing (selected nodes)

| Area | CN | Description |
|---------|----|--|
| (sq-ft) | | (subcatchment-numbers) |
| 13,586 | 98 | Impervious (Building, Driveway & Lot) (1S) |
| 3,122 | 76 | Woods/grass comb., Fair, HSG C (1S) |

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Soil Listing (selected nodes)

| Area (sq-ft) | Soil Group | Subcatchment Numbers |
|-----------------|---------------|-------------------------|
| 0 | HSG A | |
| 0 | HSG B | |
| 3,122 | HSG C | 1S |
| 0 | HSG D | |
| 13,586 | Other | 1S |

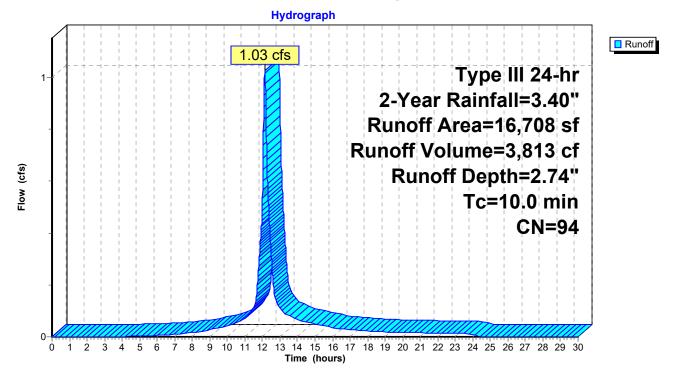
Summary for Subcatchment 1S: Existing Conditions

Runoff = 1.03 cfs @ 12.14 hrs, Volume= 3,813 cf, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.40"

| | Area (sf) | CN | Description | | | | | |
|-----|-----------|--------|--------------------------------|-----------------------------|-----------------------|--|--|--|
| * | 13,586 | 98 | Impervious | (Building, I | Driveway & Lot) | | | |
| | 3,122 | 76 | Woods/grass comb., Fair, HSG C | | | | | |
| | 0 | 98 | Unconnecte | Unconnected pavement, HSG C | | | | |
| | 16,708 | 94 | Weighted Average | | | | | |
| | 3,122 | | 18.69% Pe | rvious Area | | | | |
| | 13,586 | | 81.31% Imp | pervious Ar | ea | | | |
| - | Fo Longth | Slop | Volocity | Consoity | Description | | | |
| | Fc Length | Slope | , | Capacity | Description | | | |
| (mi | | (ft/ft |) (ft/sec) | (cfs) | | | | |
| 10 | .0 | | | | Direct Entry, Minimum | | | |

Subcatchment 1S: Existing Conditions



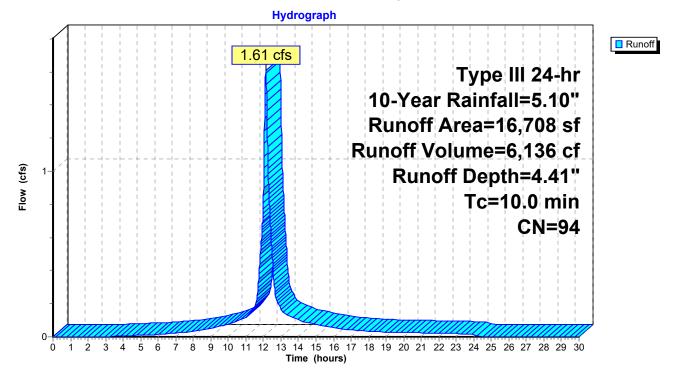
Summary for Subcatchment 1S: Existing Conditions

Runoff = 1.61 cfs @ 12.13 hrs, Volume= 6,136 cf, Depth= 4.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.10"

| | Area (sf) | CN | Description | | | | | | |
|-------|----------------------------|---------|---------------------------------------|-------------|-----------------------|--|--|--|--|
| * | 13,586 | 98 | Impervious (Building, Driveway & Lot) | | | | | | |
| | 3,122 | 76 | Woods/grass comb., Fair, HSG C | | | | | | |
| | 0 | 98 | Unconnected pavement, HSG C | | | | | | |
| | 16,708 | 94 | 94 Weighted Average | | | | | | |
| | 3,122 18.69% Pervious Area | | | | | | | | |
| | 13,586 | | 81.31% Imp | pervious Ar | ea | | | | |
| - | | ~ | | A | | | | | |
| To | 5 | Slope | | Capacity | Description | | | | |
| (min) | | (ft/ft) | (ft/sec) | (cfs) | | | | | |
| 10.0 | | | | | Direct Entry, Minimum | | | | |

Subcatchment 1S: Existing Conditions



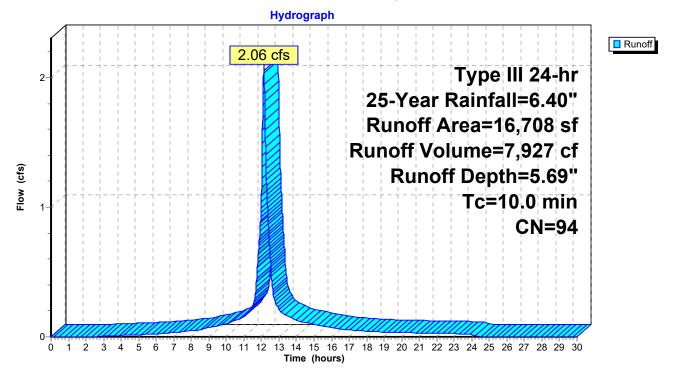
Summary for Subcatchment 1S: Existing Conditions

Runoff = 2.06 cfs @ 12.13 hrs, Volume= 7,927 cf, Depth= 5.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.40"

| | Ar | ea (sf) | CN | Description | | | | | | |
|----------|----------------------------|---------|--------|---------------------------------------|-------------|-------------------------|--|--|--|--|
| * | 1 | 13,586 | 98 | Impervious (Building, Driveway & Lot) | | | | | | |
| | | 3,122 | | Woods/grass comb., Fair, HSG C | | | | | | |
| | | 0 | 98 | Unconnected pavement, HSG C | | | | | | |
| | - | 16,708 | 94 | 4 Weighted Average | | | | | | |
| | 3,122 18.69% Pervious Area | | | | | | | | | |
| | | 13,586 | | 81.31% lmp | pervious Ar | ea | | | | |
| | Тс | Length | Slope | e Velocity | Capacity | Description | | | | |
| (m | nin) | (feet) | (ft/ft | , | (cfs) | Description | | | | |
| <u> </u> | | (ieet) | וועונ | (11/360) | (013) | Disc of Factory Minimum | | | | |
| 1 | 0.0 | | | | | Direct Entry, Minimum | | | | |

Subcatchment 1S: Existing Conditions



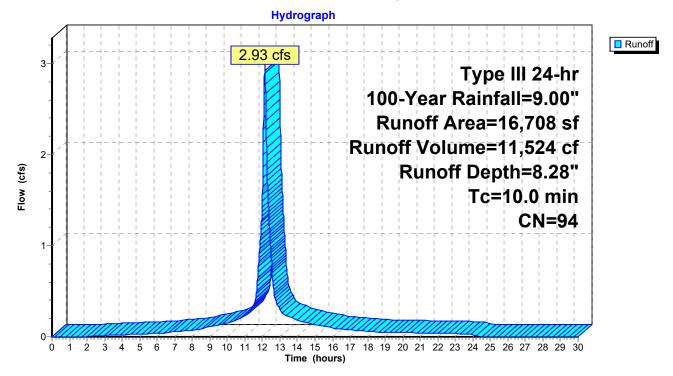
Summary for Subcatchment 1S: Existing Conditions

Runoff = 2.93 cfs @ 12.13 hrs, Volume= 11,524 cf, Depth= 8.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.00"

| | Area (sf) | CN | Description | | | | |
|----|-----------|-------|----------------------|--------------|-----------------------|--|--|
| * | 13,586 | 98 | Impervious | (Building, I | Driveway & Lot) | | |
| | 3,122 | 76 | Woods/gras | ss comb., F | Fair, HSG C | | |
| | 0 | 98 | Unconnecte | ed paveme | nt, HSG C | | |
| | 16,708 | 94 | 94 Weighted Average | | | | |
| | 3,122 | | 18.69% Pervious Area | | | | |
| | 13,586 | | 81.31% Im | pervious Ar | ea | | |
| | | | | _ | | | |
| | Tc Length | | | Capacity | Description | | |
| (m | n) (feet) | (ft/f | t) (ft/sec) | (cfs) | | | |
| 10 | 0.0 | | | | Direct Entry, Minimum | | |

Subcatchment 1S: Existing Conditions



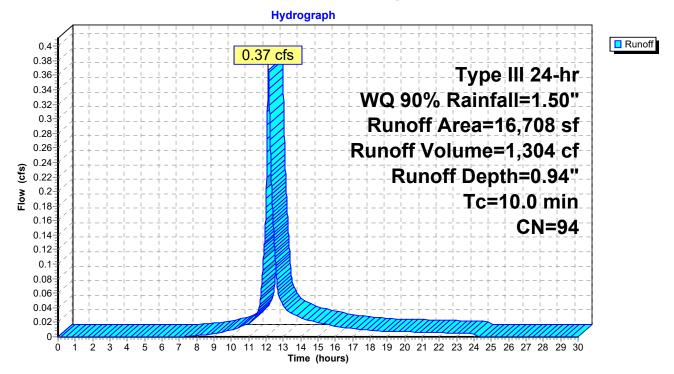
Summary for Subcatchment 1S: Existing Conditions

Runoff = 0.37 cfs @ 12.14 hrs, Volume= 1,304 cf, Depth= 0.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr WQ 90% Rainfall=1.50"

| | Area (sf) | CN | Description | | | | |
|----|-----------|-------|----------------------|--------------|-----------------------|--|--|
| * | 13,586 | 98 | Impervious | (Building, I | Driveway & Lot) | | |
| | 3,122 | 76 | Woods/gras | ss comb., F | Fair, HSG C | | |
| | 0 | 98 | Unconnecte | ed paveme | nt, HSG C | | |
| | 16,708 | 94 | 94 Weighted Average | | | | |
| | 3,122 | | 18.69% Pervious Area | | | | |
| | 13,586 | | 81.31% Im | pervious Ar | ea | | |
| | | | | _ | | | |
| | Tc Length | | | Capacity | Description | | |
| (m | n) (feet) | (ft/f | t) (ft/sec) | (cfs) | | | |
| 10 | 0.0 | | | | Direct Entry, Minimum | | |

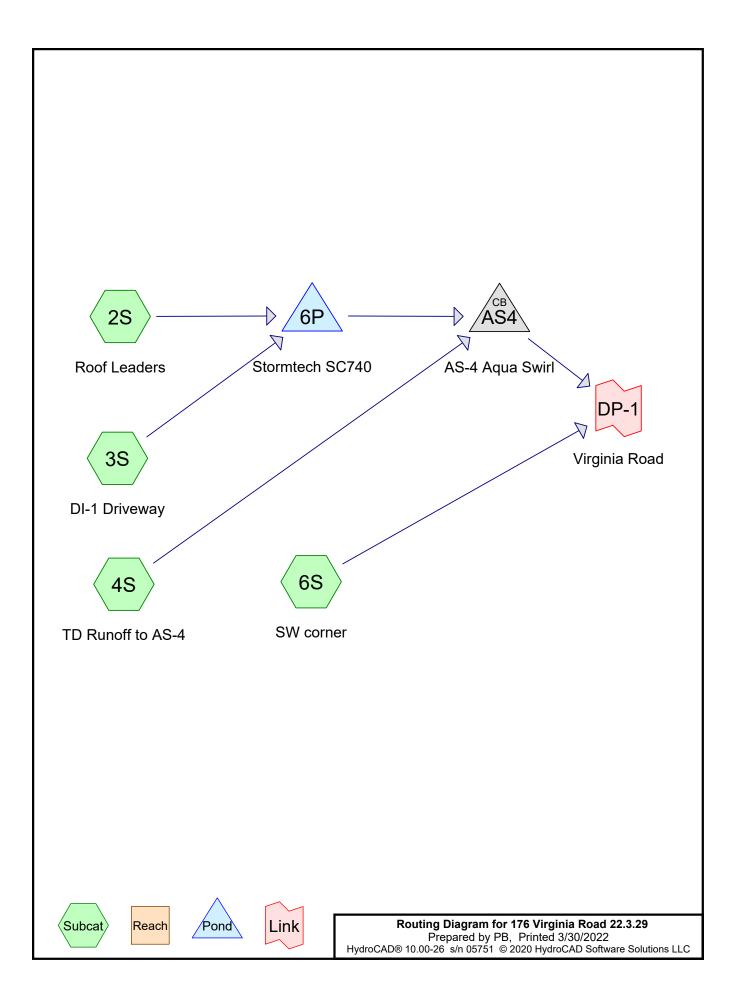
Subcatchment 1S: Existing Conditions

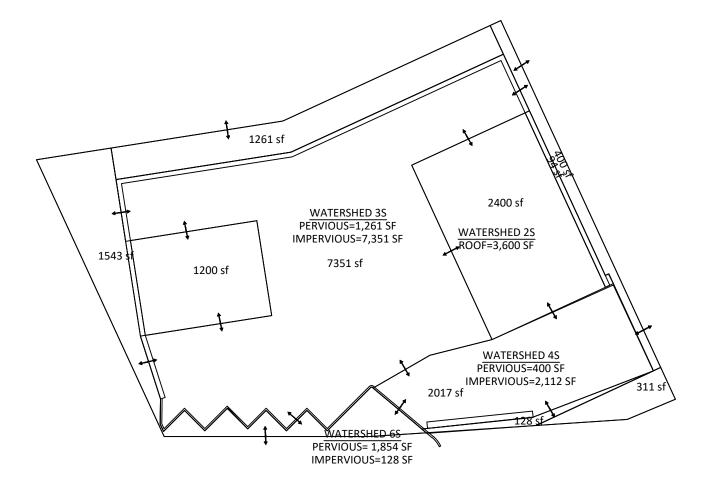


APPENDIX B

Stormwater Routings

Post Development Calculations





POST DEVELOPMENT

SCALE: 1" = 30'

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Area Listing (selected nodes)

| Area | CN | Description |
|---------|----|-------------------------------|
| (sq-ft) | | (subcatchment-numbers) |
| 1,261 | 76 | Area behind wall (3S) |
| 400 | 76 | Behind Bldg Bay 3-6 (4S) |
| 7,351 | 98 | Driveway (3S) |
| 2,112 | 98 | Dvwy to Trench Drain (4S) |
| 128 | 98 | Impervious (6S) |
| 311 | 76 | Landsacape area (6S) |
| 1,543 | 76 | Northerly landscape area (6S) |
| 3,600 | 98 | ROOF LEADERS (2S) |

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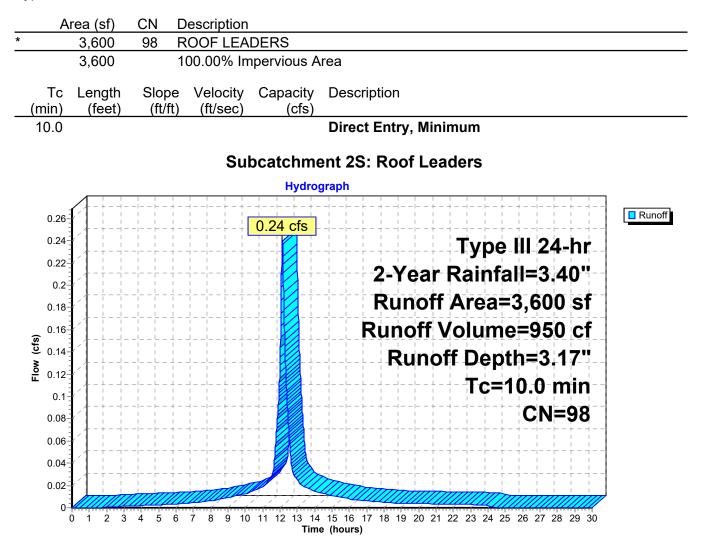
Soil Listing (selected nodes)

| Area (sq-ft) | Soil Group | Subcatchment Numbers |
|-----------------|---------------|-------------------------|
| 0 | HSG A | |
| 0 | HSG B | |
| 0 | HSG C | |
| 0 | HSG D | |
| 16,706 | Other | 2S, 3S, 4S, 6S |

Summary for Subcatchment 2S: Roof Leaders

Runoff = 0.24 cfs @ 12.13 hrs, Volume= 950 cf, Depth= 3.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.40"



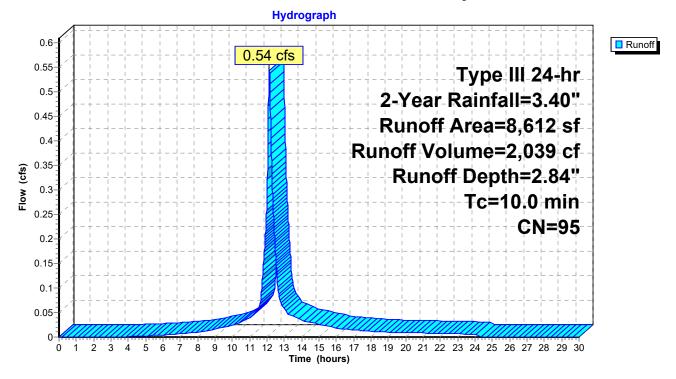
Summary for Subcatchment 3S: DI-1 Driveway

Runoff = 0.54 cfs @ 12.13 hrs, Volume= 2,039 cf, Depth= 2.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.40"

| | 10.0 | | | | | Direct Entry, Overland Flow to Inlet |
|---|-------|----------|--------|-------------|-------------|--------------------------------------|
| | (min) | (feet) | (ft/ft |) (ft/sec) | (cfs) | |
| | Тс | Length | Slop | e Velocity | Capacity | Description |
| | | 7,351 | | 85.36% Imp | pervious Ar | ea |
| | | 1,261 | | 14.64% Per | | |
| | | 8,612 | 95 | Weighted A | verage | |
| * | | 1,261 | 76 | Area behind | d wall | |
| * | | 7,351 | 98 | Driveway | | |
| | A | rea (sf) | CN | Description | | |

Subcatchment 3S: DI-1 Driveway



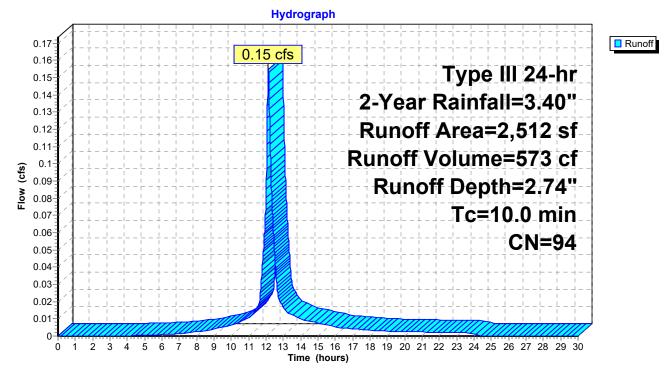
Summary for Subcatchment 4S: TD Runoff to AS-4

Runoff = 0.15 cfs @ 12.14 hrs, Volume= 573 cf, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.40"

| _ | A | rea (sf) | CN | Description | | |
|---|-------------|------------------|-----------------|-------------|-------------------|-----------------------|
| * | | 2,112 | 98 | Dvwy to Tre | ench Drain | |
| * | | 400 | 76 | Behind Bldg | g Bay 3-6 | |
| | | 2,512 | 94 | Weighted A | verage | |
| | | 400 | | 15.92% Per | vious Area | |
| | | 2,112 | | 84.08% Imp | pervious Ar | ea |
| | Tc (min) | Length (feet) | Slope (ft/ft | | Capacity (cfs) | Description |
| | 10.0 | | | | | Direct Entry, Minimum |

Subcatchment 4S: TD Runoff to AS-4

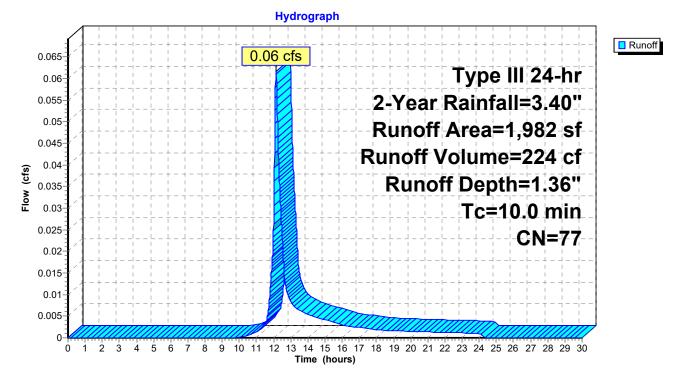


| Runoff | = | 0.06 cfs @ | 12.15 hrs. | Volume= | 224 cf, Depth= 1.36 | |
|------------|---|------------|------------|---------|---------------------|--|
| i toni o n | | | | | | |

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.40"

| | A | rea (sf) | CN | Description | | |
|----|------|----------------------------|-------|--------------|-------------|-----------------------|
| * | | 128 | 98 | Impervious | | |
| * | | 311 | 76 | Landsacap | e area | |
| * | | 1,543 | 76 | Northerly la | ndscape ar | rea |
| | | 1,982 | 77 | Weighted A | verage | |
| | | 1,854 93.54% Pervious Area | | | | |
| | | 128 | | 6.46% Impe | ervious Are | а |
| | Тс | Length | Slop | e Velocity | Capacity | Description |
| (n | nin) | (feet) | (ft/f | t) (ft/sec) | (cfs) | |
| 1 | 0.0 | | | | | Direct Entry, Minimum |

Subcatchment 6S: SW corner



Summary for Pond 6P: Stormtech SC740

Page 8

| Inflow Area = | 12,212 sf, 89.67% Impervious, | Inflow Depth = 2.94" for 2-Year event |
|---------------|-------------------------------|---------------------------------------|
| Inflow = | 0.78 cfs @ 12.13 hrs, Volume= | 2,989 cf |
| Outflow = | 0.73 cfs @ 12.17 hrs, Volume= | 2,989 cf, Atten= 7%, Lag= 2.4 min |
| Primary = | 0.73 cfs @ 12.17 hrs, Volume= | 2,989 cf |

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 201.43' @ 12.17 hrs Surf.Area= 461 sf Storage= 176 cf

Plug-Flow detention time= 12.1 min calculated for 2,989 cf (100% of inflow) Center-of-Mass det. time= 12.1 min (787.3 - 775.2)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|---|
| #1A | 200.40' | 0 cf | 15.29'W x 30.13'L x 3.67'H Field A |
| | | | 1,689 cf Overall - 1,071 cf Embedded = 618 cf x 0.0% Voids |
| #2A | 200.90' | 666 cf | StormTrap ST1 SingleTrap 2-0 x 4 Inside #1 |
| | | | Inside= 82.7"W x 24.0"H => 11.84 sf x 14.06'L = 166.5 cf |
| | | | Outside= 82.7"W x 32.0"H => 18.39 sf x 14.06'L = 258.6 cf |
| | | | 4 Chambers in 2 Rows |
| | | | <u>13.79' x 28.13' Core + 0.00' x 0.50' Border = 13.79' x 29.13' System</u> |
| | | 666 cf | Total Available Storage |

Storage Group A created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices | |
|---------|---------|----------------|--------------------------|------------------|
| #1 | Primary | 200.90' | 8.0" Vert. Orifice/Grate | C= 0.600 |
| Primary | OutFlow | Max=0.73 cfs @ |) 12.17 hrs HW=201.43' | (Free Discharge) |

1=Orifice/Grate (Orifice Controls 0.73 cfs @ 2.47 fps)

Pond 6P: Stormtech SC740 - Chamber Wizard Field A

Chamber Model = StormTrap ST1 SingleTrap 2-0 (StormTrap ST1 SingleTrap® Type VI) Inside= 82.7"W x 24.0"H => 11.84 sf x 14.06'L = 166.5 cf Outside= 82.7"W x 32.0"H => 18.39 sf x 14.06'L = 258.6 cf

82.7" Wide + 6.0" Spacing = 88.7" C-C Row Spacing

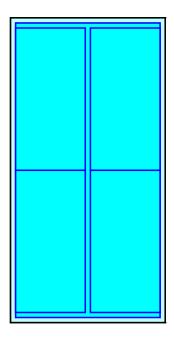
2 Chambers/Row x 14.06' Long = 28.13' Row Length +6.0" Border x 2 +6.0" End Stone x 2 = 30.13' Base Length 2 Rows x 82.7" Wide + 6.0" Spacing x 1 + 6.0" Side Stone x 2 = 15.29' Base Width 6.0" Base + 32.0" Chamber Height + 6.0" Cover = 3.67' Field Height

4 Chambers x 166.5 cf = 666.0 cf Chamber Storage 4 Chambers x 258.6 cf + 36.8 cf Border = 1,071.2 cf Displacement

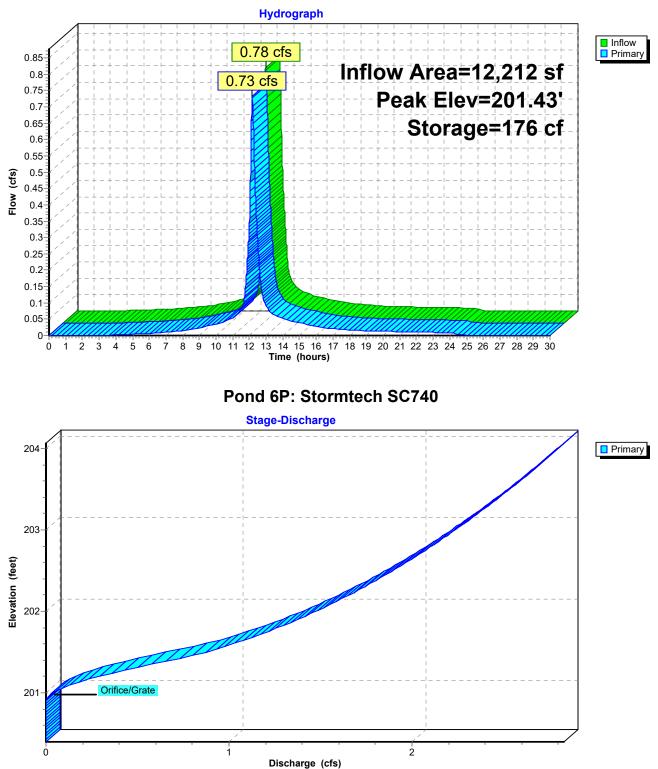
1,689.1 cf Field - 1,071.2 cf Chambers = 617.9 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

Chamber Storage = 666.0 cf = 0.015 af Overall Storage Efficiency = 39.4% Overall System Size = 30.13' x 15.29' x 3.67'

4 Chambers (plus border) 62.6 cy Field 22.9 cy Stone







Pond 6P: Stormtech SC740

Summary for Pond AS4: AS-4 Aqua Swirl

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14,724 sf, 88.72% Impervious, Inflow Depth = 2.90" for 2-Year event Inflow Area = Inflow 0.88 cfs @ 12.17 hrs, Volume= 3,562 cf = 0.88 cfs @ 12.17 hrs, Volume= 3,562 cf, Atten= 0%, Lag= 0.0 min Outflow = Primary 0.88 cfs @ 12.17 hrs, Volume= 3,562 cf =

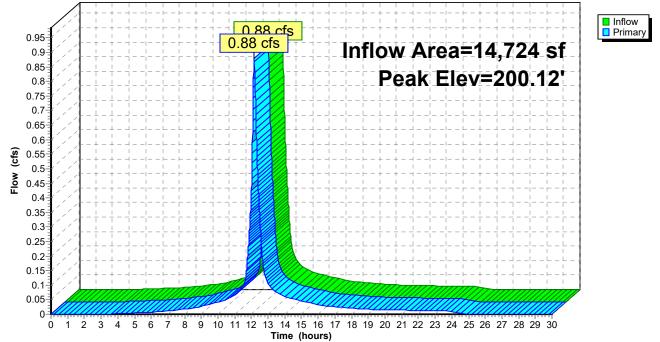
Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 200.12' @ 12.17 hrs

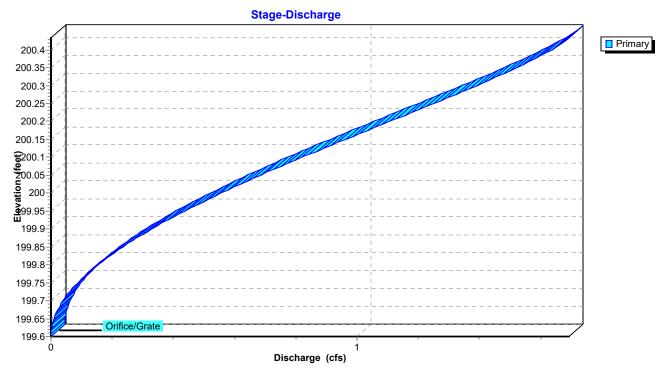
| Device | Routing | Invert | Outlet Devices | |
|--------|---------|---------|---------------------------|----------|
| #1 | Primary | 199.60' | 10.0" Vert. Orifice/Grate | C= 0.600 |

Primary OutFlow Max=0.88 cfs @ 12.17 hrs HW=200.12' (Free Discharge) -1=Orifice/Grate (Orifice Controls 0.88 cfs @ 2.45 fps)

Pond AS4: AS-4 Agua Swirl

Hydrograph



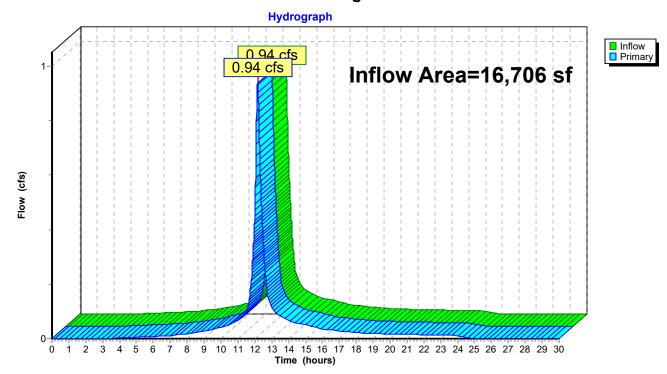


Pond AS4: AS-4 Aqua Swirl

Summary for Link DP-1: Virginia Road

| Inflow Are | a = | 16,706 sf, | 78.96% Impervious, | Inflow Depth = | 2.72" | for 2-Year event |
|------------|-----|------------|--------------------|----------------|----------|---------------------|
| Inflow | = | 0.94 cfs @ | 12.17 hrs, Volume= | 3,786 c | f | |
| Primary | = | 0.94 cfs @ | 12.17 hrs, Volume= | 3,786 c | f, Atter | n= 0%, Lag= 0.0 min |

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

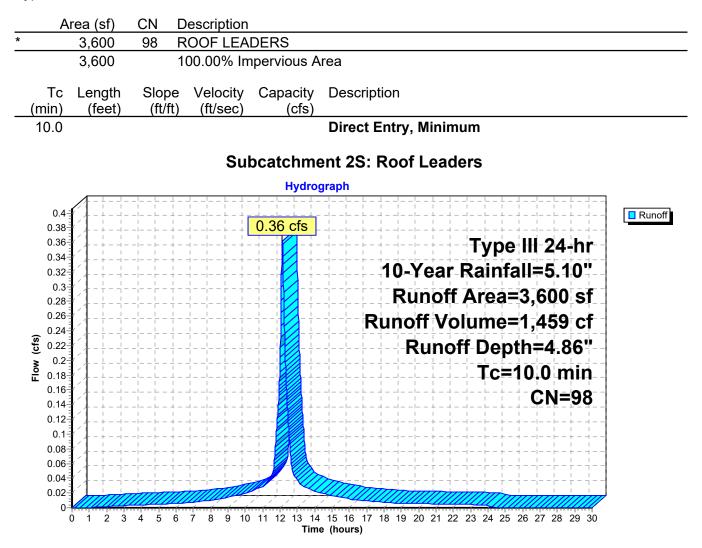


Link DP-1: Virginia Road

Summary for Subcatchment 2S: Roof Leaders

Runoff = 0.36 cfs @ 12.13 hrs, Volume= 1,459 cf, Depth= 4.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.10"



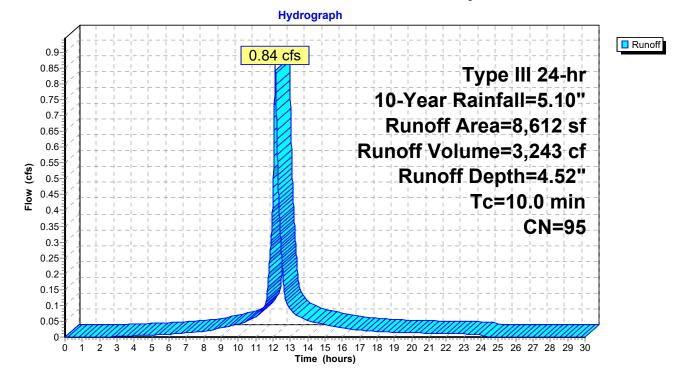
Summary for Subcatchment 3S: DI-1 Driveway

Runoff = 0.84 cfs @ 12.13 hrs, Volume= 3,243 cf, Depth= 4.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.10"

| _ | A | rea (sf) | CN | Description | | |
|---|-------------|-------------------------|-----------------|--|-------------------|--------------------------------------|
| * | | 7,351 | 98 | Driveway | | |
| * | | 1,261 | 76 | Area behind | d wall | |
| | | 8,612 1,261 7,351 | 95 | Weighted A 14.64% Per 85.36% Imp | vious Area | |
| | Tc (min) | Length (feet) | Slope (ft/ft | | Capacity (cfs) | Description |
| | 10.0 | | | | | Direct Entry, Overland Flow to Inlet |

Subcatchment 3S: DI-1 Driveway



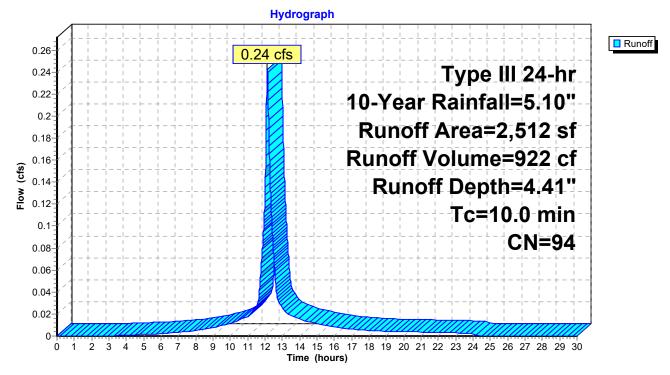
Summary for Subcatchment 4S: TD Runoff to AS-4

Runoff = 0.24 cfs @ 12.13 hrs, Volume= 922 cf, Depth= 4.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.10"

| _ | A | rea (sf) | CN | Description | | |
|---|-------------|-----------------------|----------------|--|-------------------|-----------------------|
| * | | 2,112 | 98 | Dvwy to Tre | ench Drain | |
| * | | 400 | 76 | Behind Bldg | g Bay 3-6 | |
| | | 2,512 400 2,112 | 94 | Weighted A 15.92% Per 84.08% Imp | vious Area | |
| | Tc (min) | Length (feet) | Slop (ft/ft | | Capacity (cfs) | Description |
| | 10.0 | | | | | Direct Entry, Minimum |

Subcatchment 4S: TD Runoff to AS-4



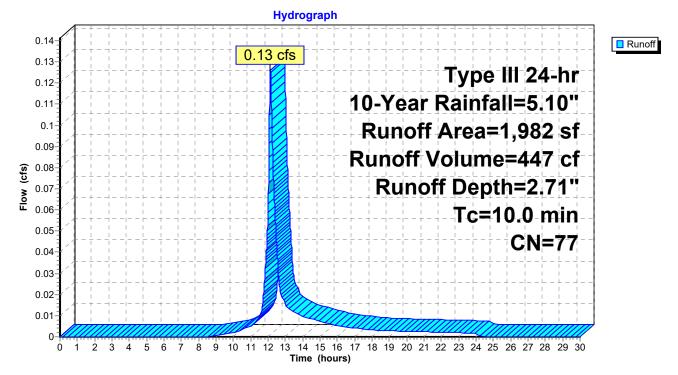
Summary for Subcatchment 6S: SW corner

Runoff = 0.13 cfs @ 12.14 hrs, Volume= 447 cf, Depth= 2.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.10"

| | A | rea (sf) | CN | Description | | | | |
|----------|------------|------------------|-----------------|----------------------|-------------------|-----------------------|--|--|
| * | | 128 | 98 | Impervious | | | | |
| * | | 311 | 76 | Landsacap | e area | | | |
| * | | 1,543 | 76 | Northerly la | ndscape a | rea | | |
| | | 1,982 | 77 | 7 Weighted Average | | | | |
| | | 1,854 | | 93.54% Pervious Area | | | | |
| | | 128 | | 6.46% Impe | ervious Are | а | | |
| (| Tc min) | Length (feet) | Slope (ft/ft | , | Capacity (cfs) | Description | | |
| <u> </u> | | (ieet) | וויוו |) (11/Sec) | (015) | | | |
| | 10.0 | | | | | Direct Entry, Minimum | | |

Subcatchment 6S: SW corner



Summary for Pond 6P: Stormtech SC740

| Inflow Area | ı = | 12,212 sf, 89.67% Impervious, Inflow Depth = 4.62" for 10-Ye | ear event |
|-------------|-----|--|-------------|
| Inflow | = | 1.20 cfs @ 12.13 hrs, Volume= 4,702 cf | |
| Outflow | = | 1.09 cfs @ 12.18 hrs, Volume= 4,701 cf, Atten= 10%, La | ag= 2.9 min |
| Primary | = | 1.09 cfs @ 12.18 hrs, Volume= 4,701 cf | |

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 201.65' @ 12.18 hrs Surf.Area= 461 sf Storage= 251 cf

Plug-Flow detention time= 10.1 min calculated for 4,701 cf (100% of inflow) Center-of-Mass det. time= 10.0 min (775.2 - 765.2)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1A | 200.40' | 0 cf | 15.29'W x 30.13'L x 3.67'H Field A |
| | | | 1,689 cf Overall - 1,071 cf Embedded = 618 cf x 0.0% Voids |
| #2A | 200.90' | 666 cf | StormTrap ST1 SingleTrap 2-0 x 4 Inside #1 |
| | | | Inside= 82.7"W x 24.0"H => 11.84 sf x 14.06'L = 166.5 cf |
| | | | Outside= 82.7"W x 32.0"H => 18.39 sf x 14.06'L = 258.6 cf |
| | | | 4 Chambers in 2 Rows |
| | | | 13.79' x 28.13' Core + 0.00' x 0.50' Border = 13.79' x 29.13' System |
| | | 666 cf | Total Available Storage |

Storage Group A created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices | |
|---------|---------|----------------|--------------------------|------------------|
| #1 | Primary | 200.90' | 8.0" Vert. Orifice/Grate | C= 0.600 |
| Primary | OutFlow | Max=1.09 cfs @ |) 12.18 hrs HW=201.65' | (Free Discharge) |

1=Orifice/Grate (Orifice Controls 1.09 cfs @ 3.12 fps)

Pond 6P: Stormtech SC740 - Chamber Wizard Field A

Chamber Model = StormTrap ST1 SingleTrap 2-0 (StormTrap ST1 SingleTrap® Type VI) Inside= 82.7"W x 24.0"H => 11.84 sf x 14.06'L = 166.5 cf Outside= 82.7"W x 32.0"H => 18.39 sf x 14.06'L = 258.6 cf

82.7" Wide + 6.0" Spacing = 88.7" C-C Row Spacing

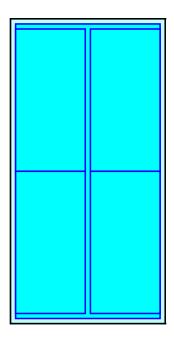
2 Chambers/Row x 14.06' Long = 28.13' Row Length +6.0" Border x 2 +6.0" End Stone x 2 = 30.13' Base Length 2 Rows x 82.7" Wide + 6.0" Spacing x 1 + 6.0" Side Stone x 2 = 15.29' Base Width 6.0" Base + 32.0" Chamber Height + 6.0" Cover = 3.67' Field Height

4 Chambers x 166.5 cf = 666.0 cf Chamber Storage 4 Chambers x 258.6 cf + 36.8 cf Border = 1,071.2 cf Displacement

1,689.1 cf Field - 1,071.2 cf Chambers = 617.9 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

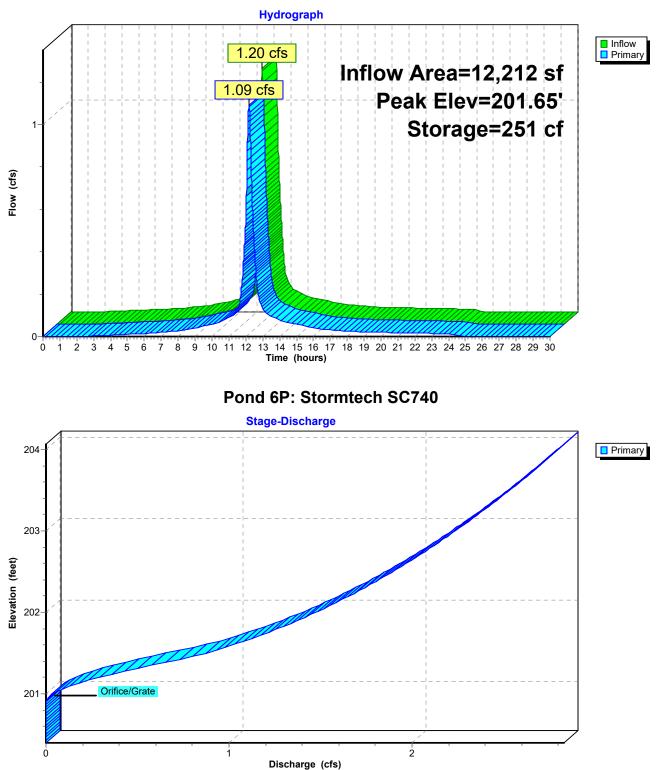
Chamber Storage = 666.0 cf = 0.015 af Overall Storage Efficiency = 39.4% Overall System Size = 30.13' x 15.29' x 3.67'

4 Chambers (plus border) 62.6 cy Field 22.9 cy Stone





176 Virginia Road 176 Virginia Road 22.3.29 Type III 24-hr 10-Year Rainfall=5.10" Prepared by PB Printed 3/30/2022 HydroCAD® 10.00-26 s/n 05751 © 2020 HydroCAD Software Solutions LLC



Pond 6P: Stormtech SC740

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Summary for Pond AS4: AS-4 Aqua Swirl

 Inflow Area =
 14,724 sf, 88.72% Impervious, Inflow Depth = 4.58" for 10-Year event

 Inflow =
 1.31 cfs @ 12.17 hrs, Volume=
 5,624 cf

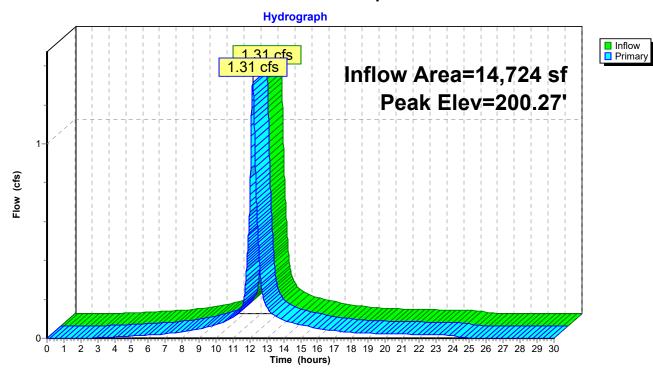
 Outflow =
 1.31 cfs @ 12.17 hrs, Volume=
 5,624 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 1.31 cfs @ 12.17 hrs, Volume=
 5,624 cf

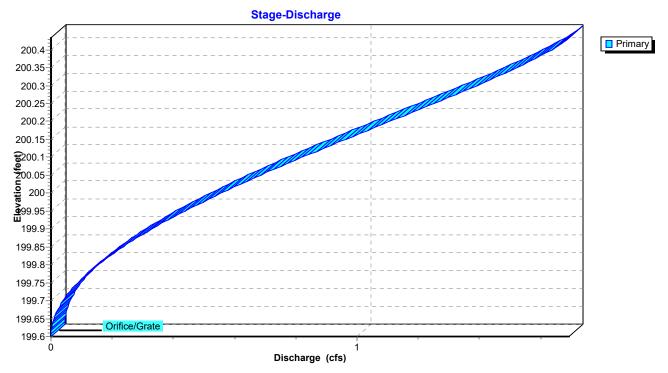
Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 200.27' @ 12.17 hrs

| Device | Routing | Invert | Outlet Devices | |
|--------|---------|---------|---------------------------|----------|
| #1 | Primary | 199.60' | 10.0" Vert. Orifice/Grate | C= 0.600 |

Primary OutFlow Max=1.31 cfs @ 12.17 hrs HW=200.27' (Free Discharge)



Pond AS4: AS-4 Aqua Swirl

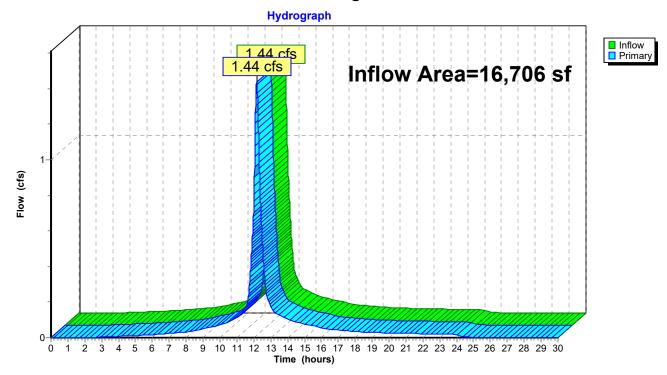


Pond AS4: AS-4 Aqua Swirl

Summary for Link DP-1: Virginia Road

| Inflow Are | a = | 16,706 sf, | 78.96% Impervious, | Inflow Depth = 4.36" | for 10-Year event |
|------------|-----|------------|--------------------|----------------------|---------------------|
| Inflow | = | 1.44 cfs @ | 12.17 hrs, Volume= | 6,071 cf | |
| Primary | = | 1.44 cfs @ | 12.17 hrs, Volume= | 6,071 cf, Atte | n= 0%, Lag= 0.0 min |

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

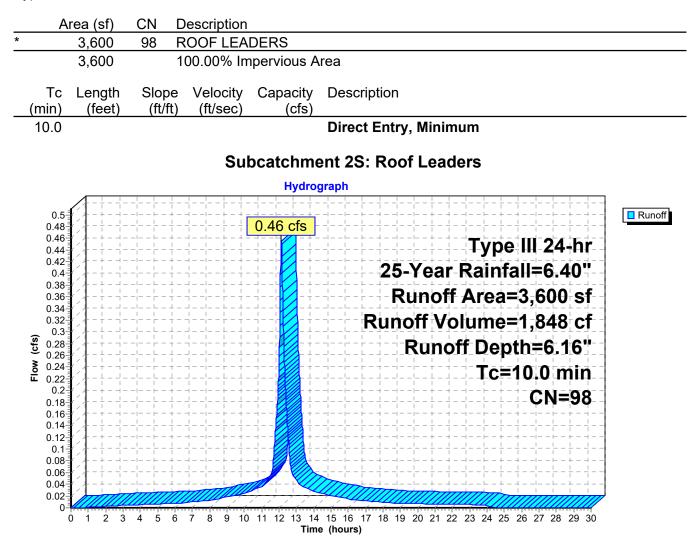


Link DP-1: Virginia Road

Summary for Subcatchment 2S: Roof Leaders

Runoff = 0.46 cfs @ 12.13 hrs, Volume= 1,848 cf, Depth= 6.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.40"



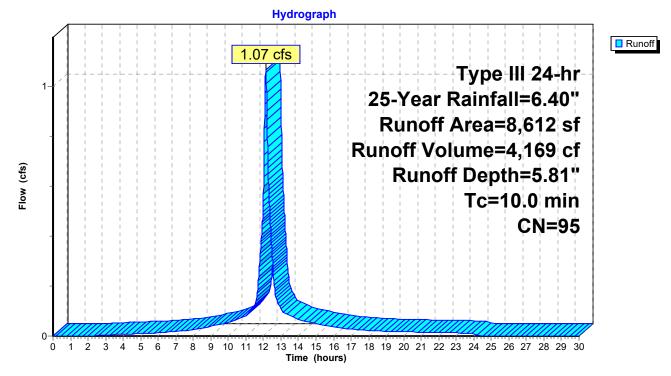
Summary for Subcatchment 3S: DI-1 Driveway

Runoff = 1.07 cfs @ 12.13 hrs, Volume= 4,169 cf, Depth= 5.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.40"

| _ | A | rea (sf) | CN | Description | | |
|---|-------|-----------------------------------|--------|--|------------|--------------------------------------|
| * | | 7,351 | 98 | Driveway | | |
| * | | 1,261 | 76 | Area behind | d wall | |
| | Тс | 8,612 1,261 7,351 Length | | Weighted A 14.64% Per 85.36% Imp • Velocity | vious Area | |
| | (min) | (feet) | (ft/ft | | (cfs) | Description |
| | 10.0 | , / | | // | | Direct Entry, Overland Flow to Inlet |

Subcatchment 3S: DI-1 Driveway



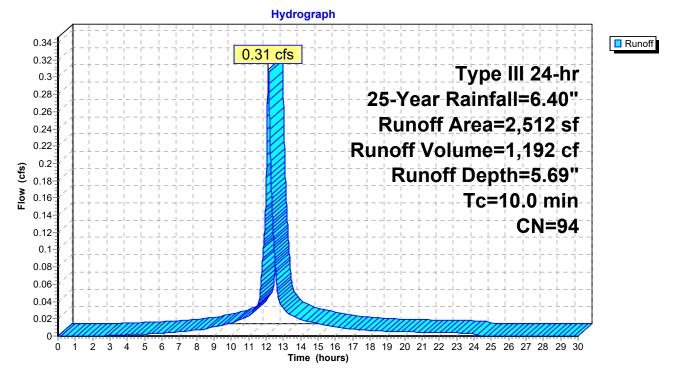
Summary for Subcatchment 4S: TD Runoff to AS-4

Runoff = 0.31 cfs @ 12.13 hrs, Volume= 1,192 cf, Depth= 5.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.40"

| _ | A | rea (sf) | CN | Description | | | |
|---|-------------|-----------------------|----------------|--|-------------------|-----------------------|--|
| * | | 2,112 | 98 | Dvwy to Tre | ench Drain | | |
| * | | 400 | 76 | Behind Bldg Bay 3-6 | | | |
| | | 2,512 400 2,112 | 94 | Weighted A 15.92% Per 84.08% Imp | vious Area | | |
| | Tc (min) | Length (feet) | Slop (ft/ft | , | Capacity (cfs) | Description | |
| | 10.0 | | | | | Direct Entry, Minimum | |

Subcatchment 4S: TD Runoff to AS-4



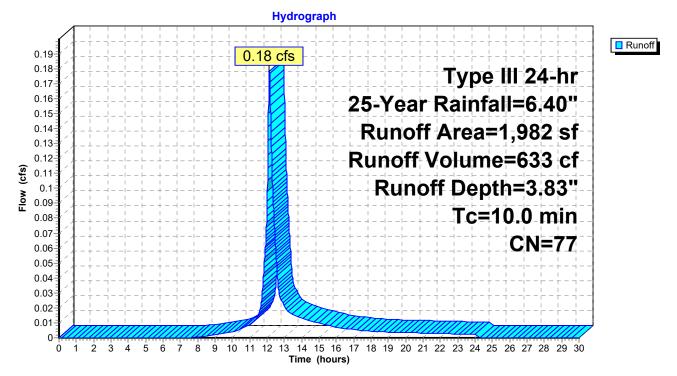
Summary for Subcatchment 6S: SW corner

Runoff = 0.18 cfs @ 12.14 hrs, Volume= 633 cf, Depth= 3.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.40"

| | A | rea (sf) | CN | Description | | | |
|---|-------|----------|----------------------------|--------------|-------------|-----------------------|--|
| * | | 128 | 98 | Impervious | | | |
| * | | 311 | 76 | Landsacap | e area | | |
| * | | 1,543 | 76 | Northerly la | indscape ai | rea | |
| | | 1,982 | 2 77 Weighted Average | | | | |
| | | 1,854 | 1,854 93.54% Pervious Area | | | | |
| | | 128 | | 6.46% Impe | ervious Are | a | |
| | _ | | | | | | |
| | Тс | Length | Slope | , | Capacity | Description | |
| (| (min) | (feet) | (ft/ft |) (ft/sec) | (cfs) | | |
| | 10.0 | | | | | Direct Entry, Minimum | |

Subcatchment 6S: SW corner



Summary for Pond 6P: Stormtech SC740

| Inflow Area | a = | 12,212 sf, 89.67% Impervious, Inflow Depth = 5.91" for 25-Y | ear event |
|-------------|-----|---|--------------|
| Inflow | = | 1.52 cfs @ 12.13 hrs, Volume= 6,017 cf | |
| Outflow | = | 1.34 cfs @ 12.19 hrs, Volume= 6,017 cf, Atten= 12%, L | .ag= 3.4 min |
| Primary | = | 1.34 cfs @ 12.19 hrs, Volume= 6,017 cf | |

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 201.87' @ 12.19 hrs Surf.Area= 461 sf Storage= 324 cf

Plug-Flow detention time= 9.2 min calculated for 6,017 cf (100% of inflow) Center-of-Mass det. time= 9.1 min (769.4 - 760.3)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1A | 200.40' | 0 cf | 15.29'W x 30.13'L x 3.67'H Field A |
| | | | 1,689 cf Overall - 1,071 cf Embedded = 618 cf x 0.0% Voids |
| #2A | 200.90' | 666 cf | StormTrap ST1 SingleTrap 2-0 x 4 Inside #1 |
| | | | Inside= 82.7"W x 24.0"H => 11.84 sf x 14.06'L = 166.5 cf |
| | | | Outside= 82.7"W x 32.0"H => 18.39 sf x 14.06'L = 258.6 cf |
| | | | 4 Chambers in 2 Rows |
| | | | 13.79' x 28.13' Core + 0.00' x 0.50' Border = 13.79' x 29.13' System |
| | | 666 cf | Total Available Storage |

Storage Group A created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices | | _ |
|---------|---------|----------------|--------------------------|------------------|---|
| #1 | Primary | 200.90' | 8.0" Vert. Orifice/Grate | C= 0.600 | |
| Primary | OutFlow | Max=1.34 cfs @ | 2 12.19 hrs HW=201.87 | (Free Discharge) | |

1=Orifice/Grate (Orifice Controls 1.34 cfs @ 3.85 fps)

Pond 6P: Stormtech SC740 - Chamber Wizard Field A

Chamber Model = StormTrap ST1 SingleTrap 2-0 (StormTrap ST1 SingleTrap® Type VI) Inside= 82.7"W x 24.0"H => 11.84 sf x 14.06'L = 166.5 cf Outside= 82.7"W x 32.0"H => 18.39 sf x 14.06'L = 258.6 cf

82.7" Wide + 6.0" Spacing = 88.7" C-C Row Spacing

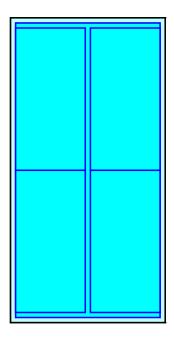
2 Chambers/Row x 14.06' Long = 28.13' Row Length +6.0" Border x 2 +6.0" End Stone x 2 = 30.13' Base Length 2 Rows x 82.7" Wide + 6.0" Spacing x 1 + 6.0" Side Stone x 2 = 15.29' Base Width 6.0" Base + 32.0" Chamber Height + 6.0" Cover = 3.67' Field Height

4 Chambers x 166.5 cf = 666.0 cf Chamber Storage 4 Chambers x 258.6 cf + 36.8 cf Border = 1,071.2 cf Displacement

1,689.1 cf Field - 1,071.2 cf Chambers = 617.9 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

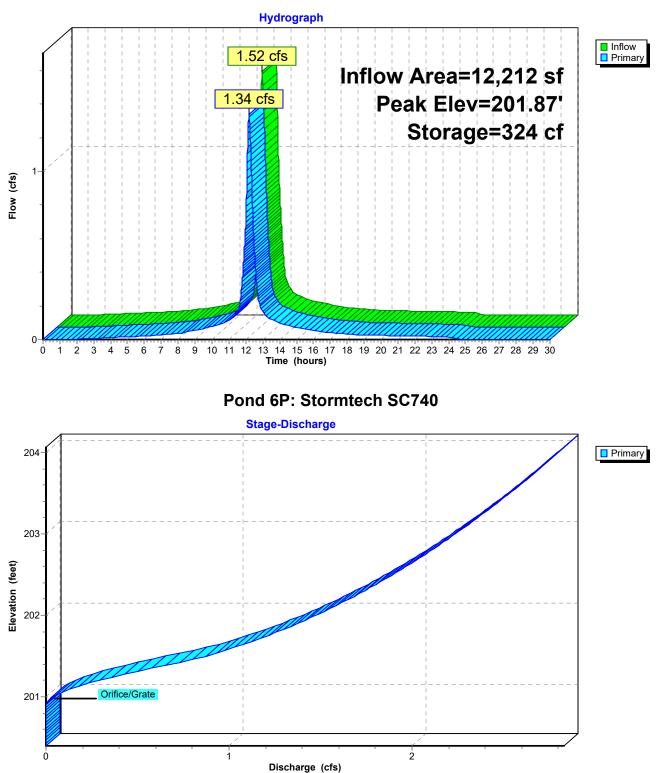
Chamber Storage = 666.0 cf = 0.015 af Overall Storage Efficiency = 39.4% Overall System Size = 30.13' x 15.29' x 3.67'

4 Chambers (plus border) 62.6 cy Field 22.9 cy Stone





176 Virginia Road Type III 24-hr 25-Year Rainfall=6.40" Prepared by PB Printed 3/30/2022 HydroCAD® 10.00-26 s/n 05751 © 2020 HydroCAD Software Solutions LLC Page 30



Pond 6P: Stormtech SC740

176 Virginia Road 22.3.29

Summary for Pond AS4: AS-4 Aqua Swirl

 Inflow Area =
 14,724 sf, 88.72% Impervious, Inflow Depth = 5.88" for 25-Year event

 Inflow =
 1.62 cfs @ 12.17 hrs, Volume=
 7,209 cf

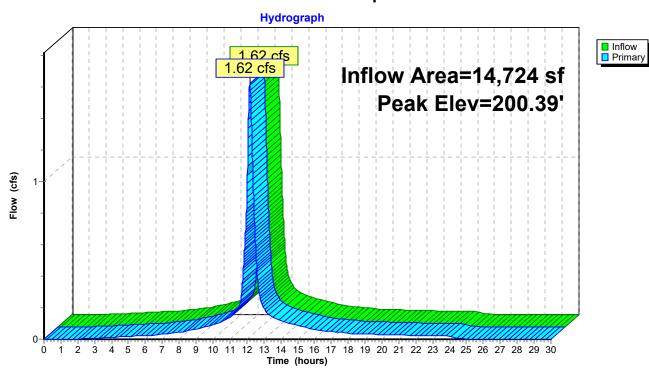
 Outflow =
 1.62 cfs @ 12.17 hrs, Volume=
 7,209 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 1.62 cfs @ 12.17 hrs, Volume=
 7,209 cf

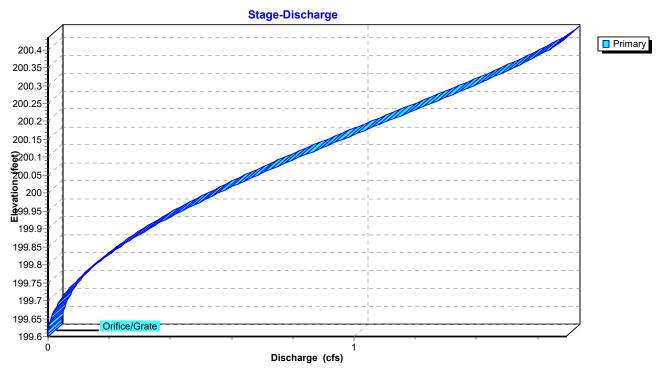
Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 200.39' @ 12.17 hrs

| Device | Routing | Invert | Outlet Devices | |
|--------|---------|---------|---------------------------|----------|
| #1 | Primary | 199.60' | 10.0" Vert. Orifice/Grate | C= 0.600 |

Primary OutFlow Max=1.62 cfs @ 12.17 hrs HW=200.39' (Free Discharge)



Pond AS4: AS-4 Aqua Swirl

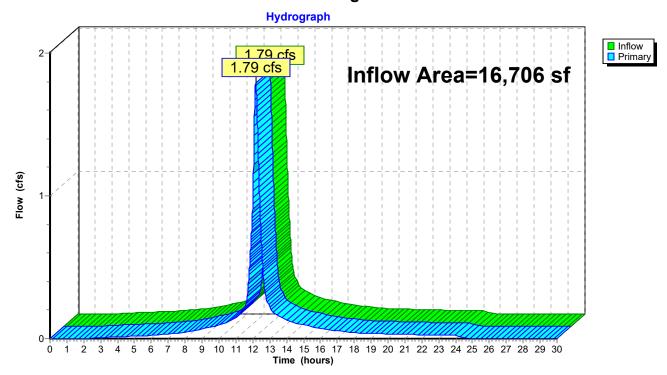


Pond AS4: AS-4 Aqua Swirl

Summary for Link DP-1: Virginia Road

| Inflow Area | a = | 16,706 sf | , 78.96% Impervious, | Inflow Depth = | 5.63" | for 25-Year event |
|-------------|-----|------------|----------------------|----------------|----------|---------------------|
| Inflow | = | 1.79 cfs @ | 12.17 hrs, Volume= | 7,841 c | f | |
| Primary | = | 1.79 cfs @ | 12.17 hrs, Volume= | 7,841 c | f, Atter | n= 0%, Lag= 0.0 min |

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

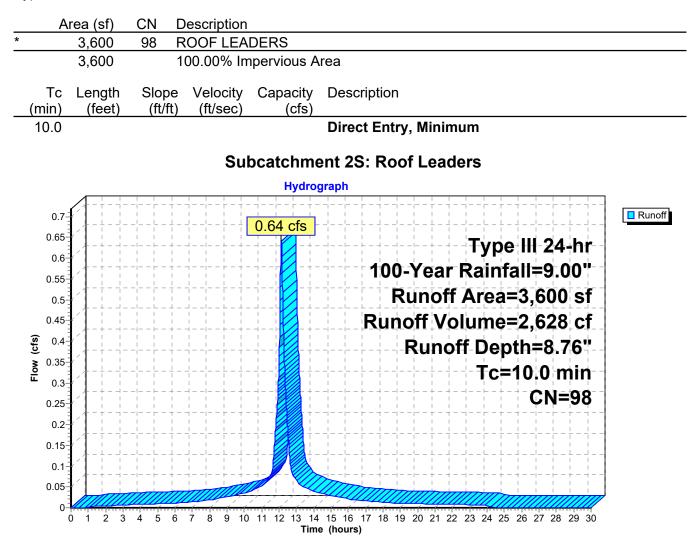


Link DP-1: Virginia Road

Summary for Subcatchment 2S: Roof Leaders

Runoff = 0.64 cfs @ 12.13 hrs, Volume= 2,628 cf, Depth= 8.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.00"



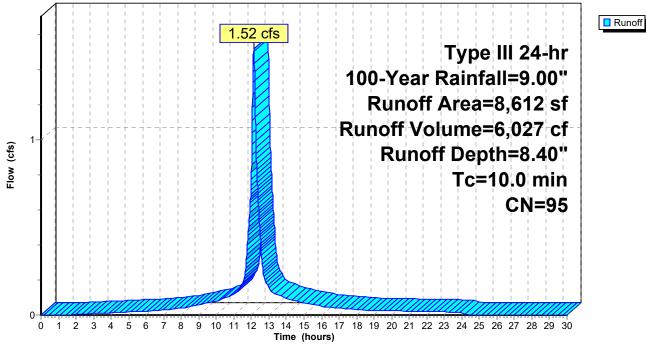
Summary for Subcatchment 3S: DI-1 Driveway

Runoff = 1.52 cfs @ 12.13 hrs, Volume= 6,027 cf, Depth= 8.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.00"

| | Ar | ea (sf) | CN | Description | | | | |
|--------------------------------|------------|-------------------------|------------------|--|-------------------|--------------------------------------|--|--|
| * | | 7,351 | 98 | Driveway | | | | |
| * | | 1,261 | 76 | Area behino | d wall | | | |
| | | 8,612 1,261 7,351 | | Weighted A 14.64% Pei 85.36% Imp | vious Area | | | |
| (r | Tc nin) | Length (feet) | Slope (ft/ft) | | Capacity (cfs) | Description | | |
| 1 | 0.0 | | | | | Direct Entry, Overland Flow to Inlet | | |
| Subcatchment 3S: DI-1 Driveway | | | | | | | | |





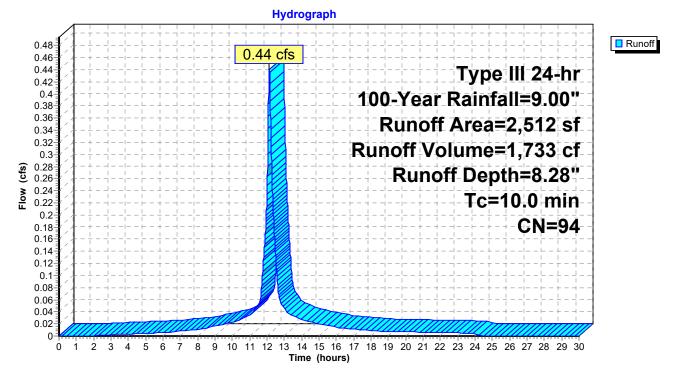
Summary for Subcatchment 4S: TD Runoff to AS-4

Runoff = 0.44 cfs @ 12.13 hrs, Volume= 1,733 cf, Depth= 8.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.00"

| | A | rea (sf) | CN | Description | | | | |
|---|--------------|----------|--------|---------------------|-------------|-----------------------|--|--|
| * | | 2,112 | 98 | Dvwy to Tre | ench Drain | | | |
| * | | 400 | 76 | Behind Bldg Bay 3-6 | | | | |
| | | 2,512 | 94 | Weighted A | verage | | | |
| | | 400 | | 15.92% Pe | vious Area | | | |
| | | 2,112 | | 84.08% Imp | pervious Ar | ea | | |
| | Тс | Length | Slop | , | Capacity | Description | | |
| | <u>(min)</u> | (feet) | (ft/ft |) (ft/sec) | (cfs) | | | |
| | 10.0 | | | | | Direct Entry, Minimum | | |

Subcatchment 4S: TD Runoff to AS-4



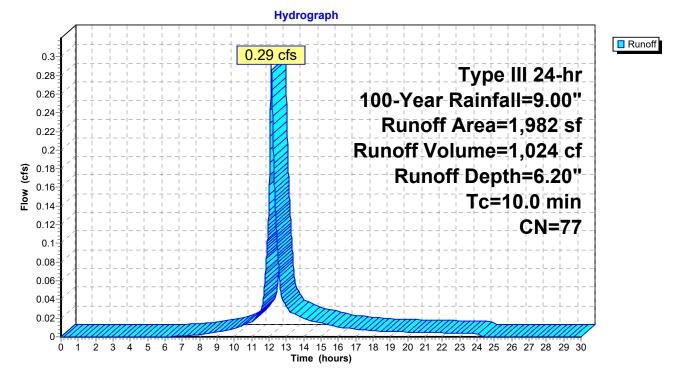
Summary for Subcatchment 6S: SW corner

Runoff = 0.29 cfs @ 12.14 hrs, Volume= 1,024 cf, Depth= 6.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=9.00"

| _ | A | rea (sf) | CN | Description | | | | |
|---|-------|--------------|---------|----------------------|-------------|-----------------------|--|--|
| * | | 128 | 98 | Impervious | | | | |
| * | | 311 | 76 | Landsacap | e area | | | |
| * | | 1,543 | 76 | Northerly la | ndscape ar | rea | | |
| | | 1,982 | 77 | Weighted A | verage | | | |
| | | 1,854 | | 93.54% Pervious Area | | | | |
| | | 128 | | 6.46% Impe | ervious Are | а | | |
| | та | l a sa artha | Clana | Valacity | Conseitu | Description | | |
| | Tc | Length | Slope | , | Capacity | Description | | |
| | (min) | (feet) | (ft/ft) | (ft/sec) | (cfs) | | | |
| | 10.0 | | | | | Direct Entry, Minimum | | |

Subcatchment 6S: SW corner



Summary for Pond 6P: Stormtech SC740

| Inflow Area | a = | 12,212 sf, 89.67% Imper | vious, Inflow Depth = 8.50 | " for 100-Year event |
|-------------|-----|----------------------------|----------------------------|-----------------------|
| Inflow | = | 2.16 cfs @ 12.13 hrs, Volu | ume= 8,655 cf | |
| Outflow | = | 1.81 cfs @ 12.20 hrs, Volu | ume= 8,654 cf, At | en= 16%, Lag= 4.1 min |
| Primary | = | 1.81 cfs @ 12.20 hrs, Volu | ume= 8,654 cf | |

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 202.40' @ 12.20 hrs Surf.Area= 461 sf Storage= 499 cf

Plug-Flow detention time= 8.1 min calculated for 8,654 cf (100% of inflow) Center-of-Mass det. time= 8.1 min (761.9 - 753.8)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1A | 200.40' | 0 cf | 15.29'W x 30.13'L x 3.67'H Field A |
| | | | 1,689 cf Overall - 1,071 cf Embedded = 618 cf x 0.0% Voids |
| #2A | 200.90' | 666 cf | StormTrap ST1 SingleTrap 2-0 x 4 Inside #1 |
| | | | Inside= 82.7"W x 24.0"H => 11.84 sf x 14.06'L = 166.5 cf |
| | | | Outside= 82.7"W x 32.0"H => 18.39 sf x 14.06'L = 258.6 cf |
| | | | 4 Chambers in 2 Rows |
| | | | 13.79' x 28.13' Core + 0.00' x 0.50' Border = 13.79' x 29.13' System |
| | | 666 cf | Total Available Storage |

Storage Group A created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices | |
|---------|---------|----------------|--------------------------|------------------|
| #1 | Primary | 200.90' | 8.0" Vert. Orifice/Grate | C= 0.600 |
| Primary | OutFlow | Max=1.81 cfs @ |) 12.20 hrs HW=202.40' | (Free Discharge) |

1=Orifice/Grate (Orifice Controls 1.81 cfs @ 5.19 fps)

Pond 6P: Stormtech SC740 - Chamber Wizard Field A

Chamber Model = StormTrap ST1 SingleTrap 2-0 (StormTrap ST1 SingleTrap® Type VI) Inside= 82.7"W x 24.0"H => 11.84 sf x 14.06'L = 166.5 cf Outside= 82.7"W x 32.0"H => 18.39 sf x 14.06'L = 258.6 cf

82.7" Wide + 6.0" Spacing = 88.7" C-C Row Spacing

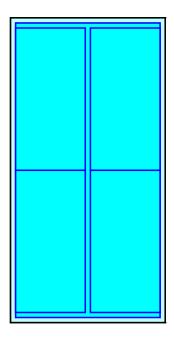
2 Chambers/Row x 14.06' Long = 28.13' Row Length +6.0" Border x 2 +6.0" End Stone x 2 = 30.13' Base Length 2 Rows x 82.7" Wide + 6.0" Spacing x 1 + 6.0" Side Stone x 2 = 15.29' Base Width 6.0" Base + 32.0" Chamber Height + 6.0" Cover = 3.67' Field Height

4 Chambers x 166.5 cf = 666.0 cf Chamber Storage 4 Chambers x 258.6 cf + 36.8 cf Border = 1,071.2 cf Displacement

1,689.1 cf Field - 1,071.2 cf Chambers = 617.9 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

Chamber Storage = 666.0 cf = 0.015 af Overall Storage Efficiency = 39.4% Overall System Size = 30.13' x 15.29' x 3.67'

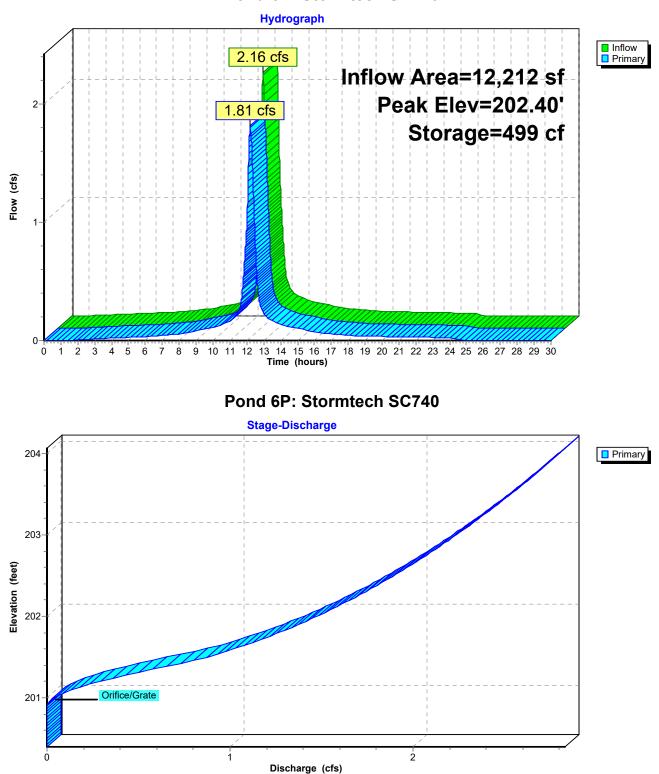
4 Chambers (plus border) 62.6 cy Field 22.9 cy Stone





176 Virginia Road 22.3.29 Prepared by PB

176 Virginia Road Type III 24-hr 100-Year Rainfall=9.00" Printed 3/30/2022 HydroCAD® 10.00-26 s/n 05751 © 2020 HydroCAD Software Solutions LLC Page 40



Pond 6P: Stormtech SC740

Summary for Pond AS4: AS-4 Aqua Swirl

 Inflow Area =
 14,724 sf, 88.72% Impervious, Inflow Depth = 8.47" for 100-Year event

 Inflow =
 2.20 cfs @ 12.18 hrs, Volume=
 10,387 cf

 Outflow =
 2.20 cfs @ 12.18 hrs, Volume=
 10,387 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 2.20 cfs @ 12.18 hrs, Volume=
 10,387 cf

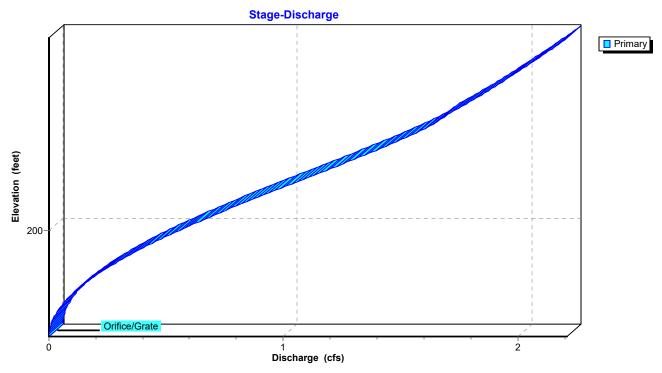
Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 200.72' @ 12.18 hrs

| Device | Routing | Invert | Outlet Devices | |
|--------|---------|---------|---------------------------|----------|
| #1 | Primary | 199.60' | 10.0" Vert. Orifice/Grate | C= 0.600 |

Primary OutFlow Max=2.20 cfs @ 12.18 hrs HW=200.72' (Free Discharge)

Hydrograph (9) 09 (9) 09 (9) 00 (9

Pond AS4: AS-4 Aqua Swirl

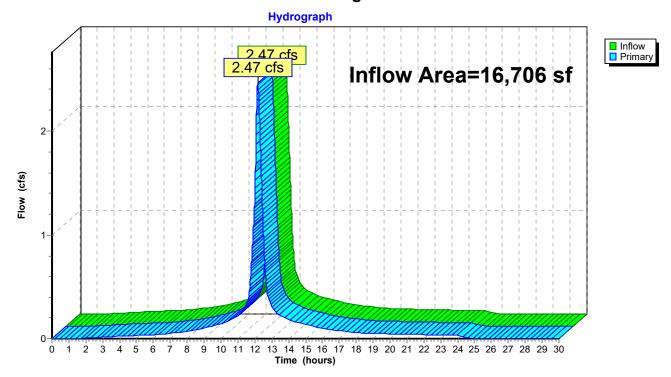




Summary for Link DP-1: Virginia Road

| Inflow Are | a = | 16,706 sf, 78.96% Impervious, Inflow Depth | = 8.20" for 100-Year event |
|------------|-----|--|-------------------------------|
| Inflow | = | 2.47 cfs @ 12.17 hrs, Volume= 11,41 | 1 cf |
| Primary | = | 2.47 cfs @ 12.17 hrs, Volume= 11,41 | 1 cf, Atten= 0%, Lag= 0.0 min |

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

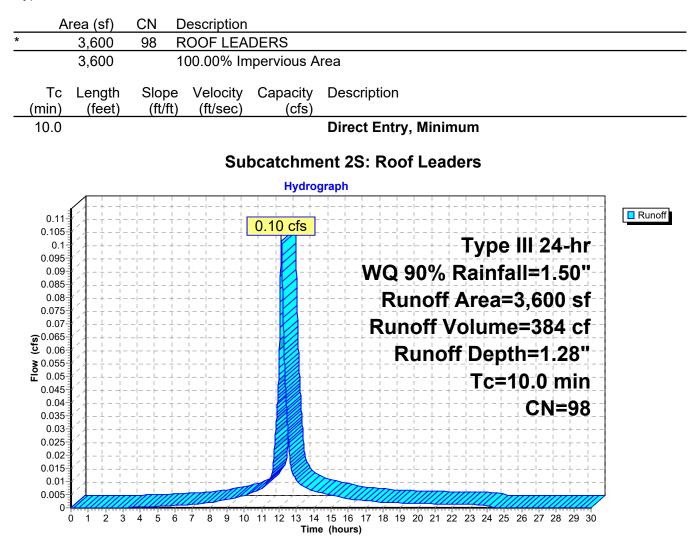


Link DP-1: Virginia Road

Summary for Subcatchment 2S: Roof Leaders

Runoff = 0.10 cfs @ 12.13 hrs, Volume= 384 cf, Depth= 1.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr WQ 90% Rainfall=1.50"



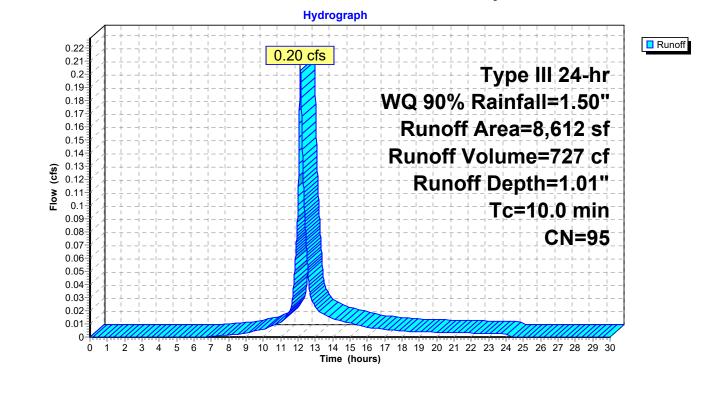
Summary for Subcatchment 3S: DI-1 Driveway

Runoff = 0.20 cfs @ 12.14 hrs, Volume= 727 cf, Depth= 1.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr WQ 90% Rainfall=1.50"

| _ | A | rea (sf) | CN | Description | | |
|---|-------------|-------------------------|----------------|--|-------------------|--------------------------------------|
| * | | 7,351 | 98 | Driveway | | |
| * | | 1,261 | 76 | Area behind | d wall | |
| | | 8,612 1,261 7,351 | 95 | Weighted A 14.64% Per 85.36% Imp | vious Area | |
| _ | Tc (min) | Length (feet) | Slop (ft/ft | | Capacity (cfs) | Description |
| | 10.0 | | | | | Direct Entry, Overland Flow to Inlet |

Subcatchment 3S: DI-1 Driveway



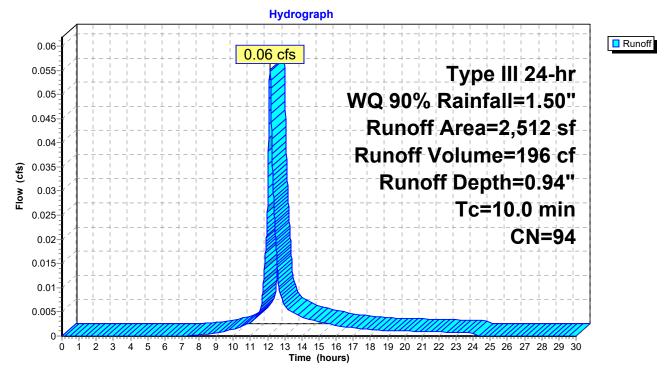
Summary for Subcatchment 4S: TD Runoff to AS-4

Runoff = 0.06 cfs @ 12.14 hrs, Volume= 196 cf, Depth= 0.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr WQ 90% Rainfall=1.50"

| | A | rea (sf) | CN | Description | | | | |
|---|-------------|------------------|-----------------|---------------------|-------------------|-----------------------|--|--|
| * | | 2,112 | 98 | Dvwy to Tre | ench Drain | | | |
| * | | 400 | 76 | Behind Bldg Bay 3-6 | | | | |
| | | 2,512 | 94 | Weighted A | verage | | | |
| | | 400 | | 15.92% Per | vious Area | | | |
| | | 2,112 | | 84.08% Imp | pervious Ar | ea | | |
| | Tc (min) | Length (feet) | Slope (ft/ft | | Capacity (cfs) | Description | | |
| | | (leet) | (11/11 |) (II/SeC) | (CIS) | | | |
| | 10.0 | | | | | Direct Entry, Minimum | | |

Subcatchment 4S: TD Runoff to AS-4



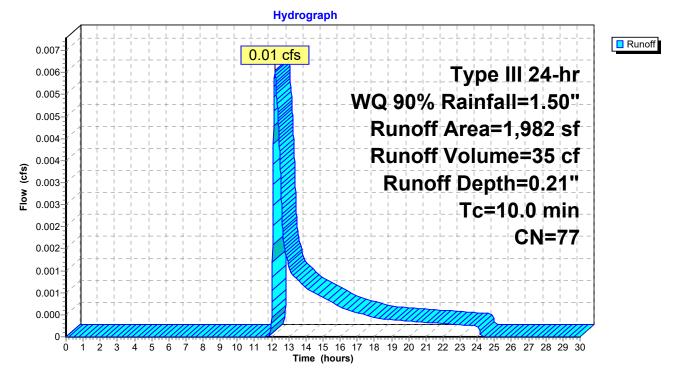
Summary for Subcatchment 6S: SW corner

Runoff = 0.01 cfs @ 12.19 hrs, Volume= 35 cf, Depth= 0.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr WQ 90% Rainfall=1.50"

| | Area (sf) | CN | Description | | | |
|----------|------------------------|-------|----------------------|-------------------|-----------------------|--|
| * | 128 | 98 | Impervious | | | |
| * | 311 | 76 | Landsacap | e area | | |
| * | 1,543 | 76 | Northerly la | indscape ai | rea | |
| | 1,982 | 77 | Weighted A | verage | | |
| | 1,854 | | 93.54% Pervious Area | | | |
| | 128 | | 6.46% Impe | ervious Are | а | |
| (mi | Tc Length n) (feet) | | | Capacity (cfs) | Description | |
| <u> </u> | | (11/1 | | (015) | | |
| 10 | .0 | | | | Direct Entry, Minimum | |

Subcatchment 6S: SW corner



Summary for Pond 6P: Stormtech SC740

| Inflow Area = | 12,212 sf, 89.67% Impervious, | Inflow Depth = 1.09" for WQ 90% event |
|---------------|-------------------------------|---------------------------------------|
| Inflow = | 0.30 cfs @ 12.14 hrs, Volume= | 1,111 cf |
| Outflow = | 0.28 cfs @ 12.19 hrs, Volume= | 1,110 cf, Atten= 10%, Lag= 3.0 min |
| Primary = | 0.28 cfs @ 12.19 hrs, Volume= | 1,110 cf |

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 201.19' @ 12.19 hrs Surf.Area= 461 sf Storage= 98 cf

Plug-Flow detention time= 17.9 min calculated for 1,110 cf (100% of inflow) Center-of-Mass det. time= 17.8 min (817.8 - 800.0)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1A | 200.40' | 0 cf | 15.29'W x 30.13'L x 3.67'H Field A |
| | | | 1,689 cf Overall - 1,071 cf Embedded = 618 cf x 0.0% Voids |
| #2A | 200.90' | 666 cf | StormTrap ST1 SingleTrap 2-0 x 4 Inside #1 |
| | | | Inside= 82.7"W x 24.0"H => 11.84 sf x 14.06'L = 166.5 cf |
| | | | Outside= 82.7"W x 32.0"H => 18.39 sf x 14.06'L = 258.6 cf |
| | | | 4 Chambers in 2 Rows |
| | | | 13.79' x 28.13' Core + 0.00' x 0.50' Border = 13.79' x 29.13' System |
| | | 666 cf | Total Available Storage |

Storage Group A created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices | |
|---------|---------------------|-------------|--------------------------|------------------|
| #1 | Primary | 200.90' | 8.0" Vert. Orifice/Grate | C= 0.600 |
| Primary | OutFlow Max= | =0.27 cfs @ |) 12.19 hrs HW=201.19 | (Free Discharge) |

1=Orifice/Grate (Orifice Controls 0.27 cfs @ 1.85 fps)

Pond 6P: Stormtech SC740 - Chamber Wizard Field A

Chamber Model = StormTrap ST1 SingleTrap 2-0 (StormTrap ST1 SingleTrap® Type VI) Inside= 82.7"W x 24.0"H => 11.84 sf x 14.06'L = 166.5 cf Outside= 82.7"W x 32.0"H => 18.39 sf x 14.06'L = 258.6 cf

82.7" Wide + 6.0" Spacing = 88.7" C-C Row Spacing

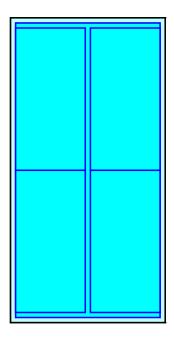
2 Chambers/Row x 14.06' Long = 28.13' Row Length +6.0" Border x 2 +6.0" End Stone x 2 = 30.13' Base Length 2 Rows x 82.7" Wide + 6.0" Spacing x 1 + 6.0" Side Stone x 2 = 15.29' Base Width 6.0" Base + 32.0" Chamber Height + 6.0" Cover = 3.67' Field Height

4 Chambers x 166.5 cf = 666.0 cf Chamber Storage 4 Chambers x 258.6 cf + 36.8 cf Border = 1,071.2 cf Displacement

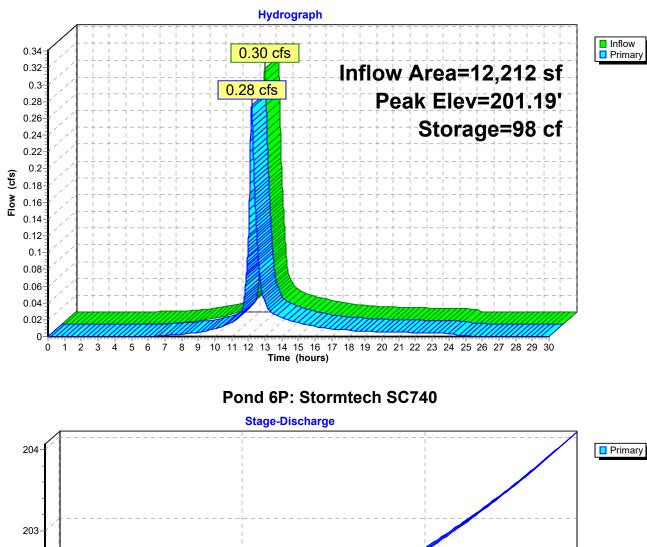
1,689.1 cf Field - 1,071.2 cf Chambers = 617.9 cf Stone x 0.0% Voids = 0.0 cf Stone Storage

Chamber Storage = 666.0 cf = 0.015 af Overall Storage Efficiency = 39.4% Overall System Size = 30.13' x 15.29' x 3.67'

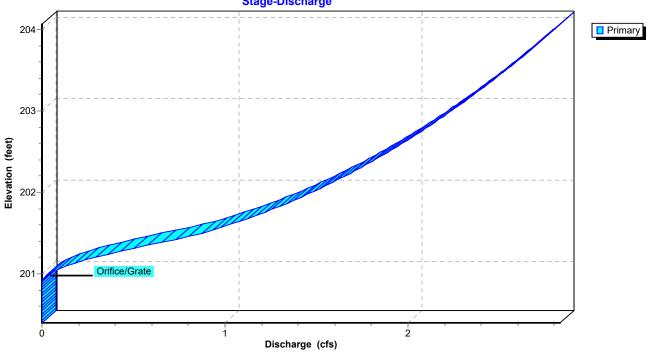
4 Chambers (plus border) 62.6 cy Field 22.9 cy Stone







Pond 6P: Stormtech SC740



Summary for Pond AS4: AS-4 Agua Swirl

14,724 sf, 88.72% Impervious, Inflow Depth > 1.06" for WQ 90% event Inflow Area = 0.33 cfs @ 12.18 hrs, Volume= Inflow 1,307 cf = 0.33 cfs @ 12.18 hrs, Volume= Outflow 1,307 cf, Atten= 0%, Lag= 0.0 min = 0.33 cfs @ 12.18 hrs, Volume= Primary 1,307 cf =

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 199.90' @ 12.18 hrs

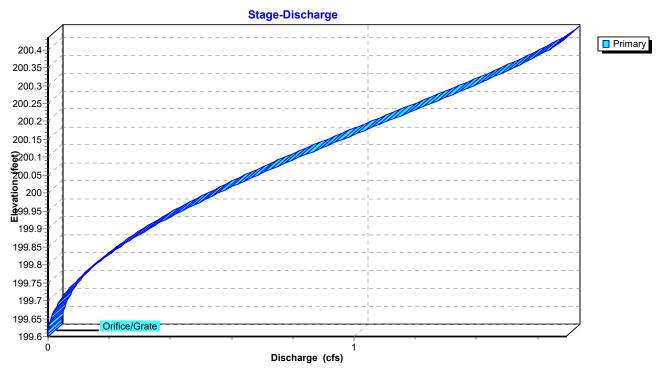
| Device | Routing | Invert | Outlet Devices | |
|--------|---------|---------|---------------------------|----------|
| #1 | Primary | 199.60' | 10.0" Vert. Orifice/Grate | C= 0.600 |

Primary OutFlow Max=0.33 cfs @ 12.18 hrs HW=199.90' (Free Discharge) -1=Orifice/Grate (Orifice Controls 0.33 cfs @ 1.86 fps)

Hydrograph Inflow Primary 0.33 cfs 0.36 0.33 cfs Inflow Area=14,724 sf 0.34 0.32 Peak Elev=199.90' 0.3 0.28 0.26 0.24 0.22 (cfs) 0.2 Flow 0.18 0.16 0.14 0.12 0.1 0.08 0.06 0.04 0.02 0 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 Time (hours)

Pond AS4: AS-4 Aqua Swirl

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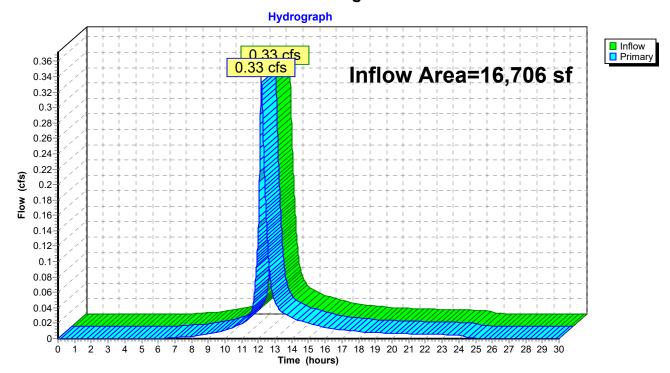


Pond AS4: AS-4 Aqua Swirl

Summary for Link DP-1: Virginia Road

| Inflow Are | a = | 16,706 sf | , 78.96% Impervious, | Inflow Depth > | 0.96" | for WQ 90% event |
|------------|-----|------------|----------------------|----------------|----------|---------------------|
| Inflow | = | 0.33 cfs @ | 12.18 hrs, Volume= | 1,341 c | f | |
| Primary | = | 0.33 cfs @ | 12.18 hrs, Volume= | 1,341 c | f, Atter | n= 0%, Lag= 0.0 min |

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



Link DP-1: Virginia Road

APPENDIX C

USDA Soils Report



United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Westchester County, New York



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



| | MAP L | EGEND |) | MAP INFORMATION | | | |
|--------------|---|---|---------------------|--|--|--|--|
| Area of Int | terest (AOI) Area of Interest (AOI) | 300 | Spoil Area | The soil surveys that comprise your AOI were mapped at 1:12,000. | | | |
| Soils | | ۵ | Stony Spot | · | | | |
| 30115 | Soil Map Unit Polygons | 0 | Very Stony Spot | Warning: Soil Map may not be valid at this scale. | | | |
| ~ | Soil Map Unit Lines | 8 | Wet Spot | Enlargement of maps beyond the scale of mapping can cause | | | |
| | Soil Map Unit Points | \triangle | Other | misunderstanding of the detail of mapping and accuracy of soil | | | |
| — Special | Point Features | Special Line Features | | line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed | | | |
| అ | Blowout | Water Fea | | scale. | | | |
| | Borrow Pit | \sim | Streams and Canals | | | | |
| * | Clay Spot | Transport | tation Rails | Please rely on the bar scale on each map sheet for map measurements. | | | |
| \diamond | Closed Depression | | Interstate Highways | | | | |
| X | Gravel Pit | ~ | US Routes | Source of Map: Natural Resources Conservation Service Web Soil Survey URL: | | | |
| 0 00 | Gravelly Spot | ~ | Major Roads | Coordinate System: Web Mercator (EPSG:3857) | | | |
| 0 | Landfill | ~ | Local Roads | Maps from the Web Soil Survey are based on the Web Mercator | | | |
| ٨. | Lava Flow | Backgrou | ind | projection, which preserves direction and shape but distorts | | | |
| عليه | Marsh or swamp | | Aerial Photography | distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more | | | |
| 衆 | Mine or Quarry | | | accurate calculations of distance or area are required. | | | |
| 0 | Miscellaneous Water | | | This product is generated from the USDA-NRCS certified data as | | | |
| 0 | Perennial Water | | | of the version date(s) listed below. | | | |
| \vee | Rock Outcrop | | | Soil Survey Area: Westchester County, New York | | | |
| + | Saline Spot | | | Survey Area Data: Version 14, Sep 3, 2018 | | | |
| °*° | Sandy Spot | | | Soil map units are labeled (as space allows) for map scales | | | |
| - | Severely Eroded Spot | | | 1:50,000 or larger. | | | |
| \diamond | Sinkhole | | | Date(s) aerial images were photographed: Jul 21, 2014—Aug | | | |
| ≫ | Slide or Slip | | | 27, 2014 | | | |
| ø | Sodic Spot | | | The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. | | | |

Map Unit Legend

| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI |
|-----------------------------|--|--------------|----------------|
| CuD | Chatfield-Hollis-Rock outcrop complex, 15 to 35 percent slopes | 0.2 | 77.0% |
| Uf | Urban land | 0.0 | 23.0% |
| Totals for Area of Interest | | 0.2 | 100.0% |

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Westchester County, New York

CuD—Chatfield-Hollis-Rock outcrop complex, 15 to 35 percent slopes

Map Unit Setting

National map unit symbol: 2w69h Elevation: 0 to 1,540 feet Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F Frost-free period: 140 to 240 days Farmland classification: Not prime farmland

Map Unit Composition

Chatfield, extremely stony, and similar soils: 35 percent Hollis, extremely stony, and similar soils: 30 percent Rock outcrop: 20 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Chatfield, Extremely Stony

Setting

Landform: Ridges, hills Landform position (two-dimensional): Backslope, shoulder, summit Landform position (three-dimensional): Crest, side slope, nose slope Down-slope shape: Convex Across-slope shape: Convex, linear Parent material: Coarse-loamy melt-out till derived from granite, gneiss, and/or schist

Typical profile

Oi - 0 to 1 inches: slightly decomposed plant material

A - 1 to 2 inches: fine sandy loam

Bw - 2 to 30 inches: gravelly fine sandy loam

2R - 30 to 40 inches: bedrock

Properties and qualities

Slope: 15 to 35 percent
Percent of area covered with surface fragments: 9.0 percent
Depth to restrictive feature: 20 to 41 inches to lithic bedrock
Natural drainage class: Well drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Very low (0.00 to 0.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water storage in profile: Low (about 4.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: B Hydric soil rating: No

Description of Hollis, Extremely Stony

Setting

Landform: Hills, ridges

Landform position (two-dimensional): Backslope, shoulder, summit Landform position (three-dimensional): Side slope, nose slope, crest Down-slope shape: Convex Across-slope shape: Linear, convex Parent material: Coarse-loamy melt-out till derived from granite, gneiss, and/or schist

Typical profile

Oi - 0 to 2 inches: slightly decomposed plant material *A - 2 to 7 inches:* gravelly fine sandy loam *Bw - 7 to 16 inches:* gravelly fine sandy loam *2R - 16 to 26 inches:* bedrock

Properties and qualities

Slope: 15 to 35 percent
Percent of area covered with surface fragments: 9.0 percent
Depth to restrictive feature: 8 to 23 inches to lithic bedrock
Natural drainage class: Somewhat excessively drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Very low (0.00 to 0.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water storage in profile: Very low (about 2.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: D Hydric soil rating: No

Description of Rock Outcrop

Setting

Landform: Hills, ridges *Parent material:* Igneous and metamorphic rock

Typical profile

R - 0 to 79 inches: bedrock

Properties and qualities

Slope: 15 to 35 percent Depth to restrictive feature: 0 inches to lithic bedrock Runoff class: Very high Capacity of the most limiting layer to transmit water (Ksat): Very low (0.00 to 0.00 in/hr)

Available water storage in profile: Very low (about 0.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8

Hydrologic Soil Group: D *Hydric soil rating:* No

Minor Components

Charlton, extremely stony

Percent of map unit: 7 percent Landform: Hills, ridges Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Linear, convex Across-slope shape: Convex Hydric soil rating: No

Leicester, extremely stony

Percent of map unit: 4 percent Landform: Drainageways, hills, ground moraines, depressions Landform position (two-dimensional): Toeslope, footslope Landform position (three-dimensional): Base slope Down-slope shape: Linear, concave Across-slope shape: Concave Hydric soil rating: Yes

Paxton, extremely stony

Percent of map unit: 2 percent Landform: Hills, ground moraines, drumlins Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Linear, convex Across-slope shape: Convex, linear Hydric soil rating: No

Sutton, extremely stony

Percent of map unit: 2 percent Landform: Ground moraines, hills Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Uf—Urban land

Map Unit Setting

National map unit symbol: bd7j Elevation: 50 to 2,400 feet Mean annual precipitation: 46 to 50 inches Mean annual air temperature: 46 to 52 degrees F *Frost-free period:* 115 to 215 days *Farmland classification:* Not prime farmland

Map Unit Composition

Urban land: 85 percent *Minor components:* 15 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Minor Components

Udorthents

Percent of map unit: 5 percent Hydric soil rating: No

Riverhead

Percent of map unit: 2 percent Hydric soil rating: No

Udorthents, wet substratum

Percent of map unit: 2 percent Hydric soil rating: No

Unadilla

Percent of map unit: 2 percent Hydric soil rating: No

Chatfield

Percent of map unit: 2 percent Hydric soil rating: No

Sutton

Percent of map unit: 2 percent Hydric soil rating: No

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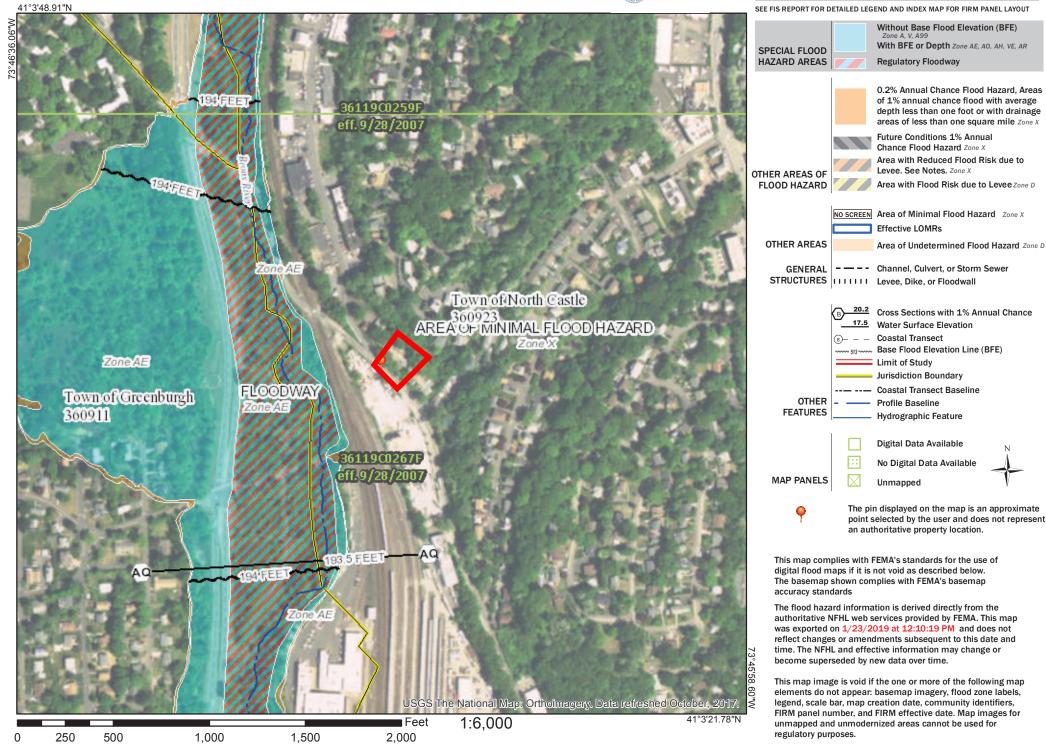
APPENDIX D

FEMA Flood Plain Map

National Flood Hazard Layer FIRMette



Legend



APPENDIX E

Water Quality Calculations

Project Mistis Properties 176 Virginia Road Revised 11-Jun-19

WATER QUALITY COMPUTATIONS, WQv

| A = Total Site Area (Limit of Disturbance) | | | = | 0.386 a | с = | 16,813 s.f | | | | | |
|---|------|---------|---------------|------------------------------------|--------------------------|--|---------------------------------|-------------------------------------|-------------------------|--------------------------|----------------------|
| Ai = Impervious Area at Post Development | Cond | lition | = | 0.048 a | c = | 2,076 s.f | . T | otal area | recoi | nstruced within pr | oject limits |
| | | | | | | | | | | | |
| I = percent Impervious Area = | | | [(| Ai |)(| 100 |)]/(| A |)= | | |
| I = percent Impervious Area = | | | [(| 0.048 ac |)(| 100 |)]/(| 0.386 ac |)= | 12 % | |
| Du - Valumatria Dun off Cooff - | | | , | | | 0.000 | V | 10.05 | 11_ | | |
| Rv = Volumetric Runoff Coeff. = | | | (| se | +[(| 0.009 |)(| 12.35 |)]= | | |
| Rv = Volumetric Runoff Coeff. = | | | (| 0.05 | +[(| 0.009 |)(| 12.35 |)]= | 0.161 | |
| P = Precipitation Depth = | | | | | | | | | | 1.5 in. | |
| P – Precipitation Depth – | | | | | | | | | | 1.3 III. | |
| WQv = Water Quality Volume = | [(| Р |)(| Rv |)(| А |)]/ | 12 | | | |
| WQv (required) | [(| 1.5 in. |)(| 0.161 |)(| 0.386 ac | - | 12 | = | 0.008 ac.ft = | 0,339 cf |
| WQv | | | | | | | | | = | 0.008 ac.ft = | 0,339 cf |
| 25% WQv (required per re-development |) | | | | | | | | = | 0.002 ac.ft = | 0,085 cf |
| | | | | | | | | | | | |
| A = Total Site Area (Limit of Disturbance) | | | = | 0.386 a | c = | 16,813 s.f | | | | | |
| | | | | | | , | - | | | | |
| Ai = Impervious Area at Post Development | Cond | lition | = | 0.259 a | c = | , 11,264 s.f | | Expanded | l Imp | ervious Cover | |
| Ai = Impervious Area at Post Development | Conc | lition | = | 0.259 a | c = | | | Expandec | l Imp | ervious Cover | |
| Ai = Impervious Area at Post Development I = percent Impervious Area = | Conc | lition | = [(| 0.259 a Ai | c =)(| | | | l Imp)= | ervious Cover | |
| | Conc | lition | | | | 11,264 s.f | | A |)= | ervious Cover 67 % | |
| I = percent Impervious Area = | Conc | lition | [(| Ai |)(| 11,264 s.f 100 |)]/(| A |)= | | |
| I = percent Impervious Area = | Conc | lition | [(| Ai |)(| 11,264 s.f 100 |)]/()]/(| A |)=)= | | |
| l = percent Impervious Area = l = percent Impervious Area = | Conc | lition | [(| Ai 0.259 ac |)()(| 11,264 s.f 100 100 |)]/()]/(| A 0.386 ac equired W |)=)= | | |
| I = percent Impervious Area = I = percent Impervious Area = Rv = Volumetric Runoff Coeff. = Rv = Volumetric Runoff Coeff. = | Conc | lition | [(| Ai 0.259 ac se |)()(+=[(| 11,264 s.f 100 100 0.009 |)]/()]/()(| A 0.386 ac equired W |)=)= ()]= | 67 % 0.653 | |
| I = percent Impervious Area = I = percent Impervious Area = Rv = Volumetric Runoff Coeff. = | Conc | lition | [(| Ai 0.259 ac se |)()(+=[(| 11,264 s.f 100 100 0.009 |)]/()]/()(| A 0.386 ac equired W |)=)= ()]= | 67 % | |
| I = percent Impervious Area = I = percent Impervious Area = Rv = Volumetric Runoff Coeff. = Rv = Volumetric Runoff Coeff. = P = Precipitation Depth = | | | [([((| Ai 0.259 ac se 0.05 |)()(++[(++[(| 11,264 s.f 100 100 0.009 0.009 |)]/()]/()()(| A 0.386 ac equired W 67.00 |)=)= ()]= | 67 % 0.653 | |
| I = percent Impervious Area = I = percent Impervious Area = Rv = Volumetric Runoff Coeff. = Rv = Volumetric Runoff Coeff. = P = Precipitation Depth = WQv = Water Quality Volume = | [(| Ρ | [([((| Ai 0.259 ac se 0.05 Rv |)()(++[(++[(| 11,264 s.f 100 100 0.009 0.009 |)]/()]/()()()]/ | A 0.386 ac equired W 67.00 |)=)= ()]=)]= | 67 % 0.653 1.5 in. | 1.372 cf |
| I = percent Impervious Area = I = percent Impervious Area = Rv = Volumetric Runoff Coeff. = Rv = Volumetric Runoff Coeff. = P = Precipitation Depth = | | | [([((| Ai 0.259 ac se 0.05 |)()(++[(++[(| 11,264 s.f 100 100 0.009 0.009 |)]/()]/()()()]/ | A 0.386 ac equired W 67.00 |)=)= ()]= | 67 % 0.653 | 1,372 cf 1,372 cf |

Total WQv 90% Rainfall Event = 0.033 ac.ft = 1,457 cf

AquaSwirl AS-4

Note: See HydroCAD for storm routings

= 0.030 ac.ft 1,304 cf

Total WQv (Provided) = 0.033 ac.ft = 1,457 cf